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Compatibility Conditions of Structural Mechanics

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SYMBOLS

A	internal energy
a, b	plate radii
a_{vx}, a_{vy}, a_{vz}	direction cosines
B	complementary energy
$\bar{B}_x, \bar{B}_y, \bar{B}_z$	body force components
$[B]$	equilibrium matrix
$[C]$	compatibility matrix
E	Young's modulus
e_x, e_y	eccentricities
$\{F\}$	internal force
$[G]$	flexibility matrix
$[J]$	deformation coefficient matrix
K	plate rigidity
$[K]$	stiffness matrix
ℓ	table height
M_r, M_ϕ	moments
P	load
P_x, P_y, P_z	reactions where displacements \bar{u} , \bar{v} , and \bar{w} are prescribed
$\bar{P}_x, \bar{P}_y, \bar{P}_z$	prescribed tractions
$\{P\}$	external load
$\{P^*\}$	equivalent load
q	uniformly distributed load
R_1, \dots, R_4	reactions along table legs
\mathcal{R}	redundant force
\Re	residue function
r	radius

[S]	IFM matrix
Δt	temperature differential
u, v, w	displacement components
V	volume
W	potential of work done
x, y, z	Cartesian coordinates
β	deformation
β^0	initial deformation
$\gamma_{xy}, \gamma_{yz}, \gamma_{zx}$	shear strain components
$\epsilon_x, \epsilon_y, \epsilon_z$	normal strain components
θ_x, θ_y	rotations
ν	Poisson's ratio
π_s	variational functional
$\sigma_x, \sigma_y, \sigma_z$	normal stress components
$\tau_{xy}, \tau_{yz}, \tau_{zx}$	shear stress components
ϕ_1, ϕ_2, ϕ_3	stress functions
BCC	boundary compatibility conditions
BMF	Beltrami-Michell formulation
CBMF	completed Beltrami-Michell formulation
CC	compatibility conditions
DDR	deformation displacement relations
EE	equilibrium equations
IFM	integrated force method
IFMD	dual integrated force method

SUMMARY

The theory of elasticity has camouflaged a deficiency in the compatibility formulation since 1860. In structures the ad hoc compatibility conditions through virtual “cuts” and closing “gaps” are not parallel to the strain formulation in elasticity. This deficiency in the compatibility conditions has prevented the development of a direct stress determination method in structures and in elasticity. We have addressed this deficiency and attempted to unify the theory of compatibility. This work has led to the development of the integrated force method for structures and the completed Beltrami-Michell formulation for elasticity. The improved accuracy observed in the solution of numerical examples by the integrated force method can be attributed to the compliance of the compatibility conditions. Using the compatibility conditions allows mapping of variables and facile movement among different structural analysis formulations. This paper reviews and illustrates the requirement of compatibility in structures and in elasticity. It also describes the generation of the conditions and quantifies the benefits of their use. The traditional analysis methods and available solutions (which have been obtained bypassing the missed conditions) should be verified for compliance of the compatibility conditions.

INTRODUCTION

The equilibrium equations and the compatibility conditions required for the analysis of stress in an elastic continuum were formulated by Cauchy and Saint-Venant during the first and third quarters of the nineteenth century, respectively. Cauchy's stress formulation was complete, with field equations and boundary conditions. Saint-Venant formulated the field equations in 1860 but did not provide the compatibility conditions on the boundary. Traditional compatibility theory in structures has been developed through the concept of redundant forces, with virtual "cuts" and subsequent closing of the "gaps." In elasticity the field equations generated through Saint-Venant's strain formulation are not parallel to the concept of cuts and closed gaps in structures. The compatibility formulation differed in elasticity and in structures. In other words, compatibility theory has concealed a weak spot since the strain formulation in 1860. The deficiency has prevented the development of a direct structural analysis method, even though the origin of the science can be traced back to the cantilever experiment of Galileo, conducted four centuries ago (ref. 1)

If compatibility is not required (as in the analysis of determinate structures), the method of forces represents the most efficient formulation. This method heretofore could not be extended to the analysis of indeterminate structures because of the weakness in compatibility theory. The compatibility barrier blocked the natural growth of the method, causing it to split into the stiffness method and the redundant force method as depicted in Fig. 1. The stiffness method has made magnificent progress, but the ad hoc redundant force formulation has disappeared for all practical purposes because of its limited scope (refs. 2 and 3).

Our research has attempted to extend the method of forces to the analysis of indeterminate structures. The basic concept of the method is expressed in the following symbolic equation (devised through discussions with Richard H. Gallagher):

$$\left[\begin{array}{c} \text{Equilibrium equation} \\ \text{Compatibility condition} \end{array} \right] \{\text{Stress}\} = \left[\begin{array}{c} \text{Mechanical load} \\ \text{Initial deformation} \end{array} \right] \quad (1)$$

For stress analysis the equilibrium equation (EE) represents the necessary condition. Sufficiency is achieved through the compatibility conditions (CC). Even though a method corresponding to equation (1) could not be developed, such a formulation was envisioned by Michell and is described by Love (ref. 4) in the following quotation:

It is possible by taking account of these relations [compatibility conditions] to obtain a complete system of equations [eq. (1)] which must be satisfied by stress components, and thus the way is open for a direct determination of stress without the intermediate steps of forming and solving differential equations to determine the components of displacements.

At the turn of the twentieth century Beltrami and Michell attempted to formulate a direct method in elasticity with stress $\{\sigma\}$ as the unknown, referred to as the Beltrami-Michell formulation (BMF). Its field equation ($\mathcal{L}\{\sigma\} = \{\mathcal{P}\}$), was obtained by coupling the EE and the CC. The BMF had limited scope because the compatibility conditions on the boundary were not treated. For discrete structural analysis a method that can be considered parallel to the BMF with internal force $\{F\}$ as the unknown and the governing equation ($\{S\}\{F\} = \{P\}$) was not available during the formative 1960's.

We have addressed and attempted to unify compatibility theory in elasticity and in structures. In elasticity the compatibility conditions on the boundary have been identified (refs. 5 and 6). In structures our compatibility conditions are analogous to the strain formulation (ref. 7). Coupling the compatibility conditions to the equilibrium equations has led to the formulation of the integrated force method (IFM, ref. 8) for finite element structural analysis and the completed Beltrami-Michell formulation (CBMF) in elasticity (ref. 6). Both IFM and CBMF are versatile formulations applicable to static and dynamic analysis of linear and nonlinear structures. These methods and other analysis formulations are summarized in Table I. The IFM equations can be specialized to obtain the stiffness method (refs. 9 and 10) and the classical force method (ref. 11). Through the IFM matrices the hybrid and total formulations can be defined. The variational functional of the integrated force method can be specialized to obtain the functionals of the other methods given in Table I. The IFM equations allow free movement from stress to displacement and vice versa.

This paper is an invited paper in the International Journal of Numerical Methods in Engineering commemorating Richard H. Gallagher (1927–1997), who was a co-contributor to the theory of the integrated force method (refs. 7, 8, and 12). The paper reviews and illustrates the requirement of compatibility in structures and in elasticity.

It discusses the role of compatibility in structures by considering Navier's table problem as an example. It also illustrates the necessity of the boundary compatibility conditions in elasticity through the analysis of a composite plate. We generated the three-dimensional compatibility conditions on the boundary from the stationary conditions of the IFM variational functional. We also quantified the benefits from their use in structural analysis by solving three examples. The paper is completed with discussions and conclusions.

REQUIREMENT OF COMPATIBILITY CONDITIONS

A direct determination of the six stress components requires six equations—three equilibrium equations and three compatibility conditions. However, indirect methods can be obtained by transforming stress into displacement or scalar function. The indirect methods emphasize either equilibrium equations or compatibility conditions. However, confirming the accuracy of the response generated by the direct or indirect methods requires satisfying the EE and the CC. Likewise, analysis of indeterminate structures requires both equilibrium and compatibility. The requirement of compatibility is illustrated by considering both Navier's table problem for structures and a composite circular plate in elasticity.

Navier's Table Problem

Navier attempted to determine the four reactions (R_1 , R_2 , R_3 , and R_4) along the legs of the table shown in Fig. 2. The symmetrical table is made of wood, rests on a level stone floor, and is subjected to a load P with eccentricities e_x and e_y . The table height is ℓ , and the distances between the legs along the x and y directions are $2a$ and $2b$, respectively. Young's modulus of the table leg material is E , and its cross-sectional area is A . The table top is considered rigid.

The IFM solution for the four reactions is obtained by augmenting Navier's three equilibrium equations with one compatibility condition:

$$\begin{bmatrix} -1 & -1 & -1 & -1 \\ a & a & -a & -a \\ -b & b & b & -b \\ 1 & -1 & 1 & -1 \end{bmatrix} \begin{Bmatrix} R_1 \\ R_2 \\ R_3 \\ R_4 \end{Bmatrix} = \begin{Bmatrix} -P \\ -e_y P \\ e_x P \\ 0 \end{Bmatrix} \quad (2)$$

Solving the IFM equation yields the four reactions as follows:

$$\begin{Bmatrix} R_1 \\ R_2 \\ R_3 \\ R_4 \end{Bmatrix} = \frac{P}{4} \begin{bmatrix} 1 - r_x - r_y \\ 1 + r_x - r_y \\ 1 + r_x + r_y \\ 1 - r_x + r_y \end{bmatrix} = \frac{P}{4} \begin{Bmatrix} 1 \\ 1 \\ 1 \\ 1 \end{Bmatrix}_{r_x=r_y=0} \quad (3)$$

where $r_x = e_x/a$ and $r_y = e_y/b$. Each leg carries one-quarter of the load when it has no eccentricity ($r_x = r_y = 0.0$).

Three displacement components are backcalculated as

$$\begin{Bmatrix} w \\ \theta_x \\ \theta_y \end{Bmatrix} = \left(\frac{\ell}{4AE} \right) \begin{bmatrix} -1 & -1 & -1 & -1 \\ \frac{1}{a} & \frac{1}{a} & -\frac{1}{a} & -\frac{1}{a} \\ -\frac{1}{b} & \frac{1}{b} & \frac{1}{b} & -\frac{1}{b} \end{bmatrix} \begin{Bmatrix} R_1 \\ R_2 \\ R_3 \\ R_4 \end{Bmatrix} \quad (4)$$

If the table had three legs, its three reactions could have been calculated from the three equilibrium equations. Its fourth leg requires an additional compatibility condition. The IFM through its compatibility formulation provides the fourth equation in the matrix (see eq. (2)).

Instead of developing the compatibility condition Navier rewrote the three equilibrium equations in terms of the displacements (w , θ_x , and θ_y) to obtain the three stiffness equations. Solution yielded the three displacements, from which the four reactions could be backcalculated. Generalization of this procedure, which is credited to Navier, became the popular stiffness method (refs. 13 and 14).

In the redundant force method one leg of the table was “cut” to obtain a three-legged determinate table. This determinate problem was solved for reactions and displacement Δ^P at the cut. The solution process was repeated for a load \mathcal{R} , called a redundant force, in place of the reaction for the leg that had been cut. The resulting displacement $\Delta^{\mathcal{R}}$ at the cut was obtained. Because the physical table had no real cut, the gap was closed ($\Delta^P + \Delta^{\mathcal{R}} = 0$), yielding the value of the redundant reaction. Solving the determinate structure subjected to both the redundant reaction and the external load provided the result for the indeterminate problem. The generalization of this procedure became the redundant force method (refs. 2, 3, and 15).

Composite Circular Plate

The compatibility requirement in elasticity is illustrated through the example of a composite circular plate as shown in Fig. 3. The structure is made of an aluminum inner plate (I) of radius $a = 6$ in. and a steel outer sector (II) of radius $b = 12$ in. The inner plate (I) is subjected to a uniformly distributed load of intensity $q = 100$ psi, whereas the outer plate (II) is uniformly heated with a temperature differential of $\Delta t = 50$ °F. The thicknesses of the inner and outer plates are $h^I = 0.2$ in. and $h^{II} = 0.15$ in., Young’s moduli are $E^I = 10.6 \times 10^6$ and $E^{II} = 30.0 \times 10^6$ psi, Poisson’s ratios are $\nu^I = 0.33$ and $\nu^{II} = 0.3$, respectively, the coefficient of expansion of the outer plate is α_t .

In the completed Beltrami-Michell formulation the moments M_r and M_ϕ are considered as the unknowns. The plate has one field equilibrium equation and one compatibility condition as follows:

$$\frac{d^2}{dr^2}(rM_r) - \frac{dM_\phi}{dr} + rq = 0 \quad (5)$$

$$r \frac{d}{dr}(M_\phi - \nu M_r) + (1 + \nu)(M_\phi - M_r) + Kr \frac{\alpha_t}{h} \left(\frac{d\Delta t}{dr} \right) = 0 \quad (6)$$

where the plate rigidity is defined as $K = Eh^3/[12(1 - \nu^2)]$. Solving the composite problem requires solving the two field equations for the six boundary conditions. One boundary compatibility condition occurs at the clamped boundary ($r = b$):

$$\left[\frac{1}{K}(M_\phi - \nu M_r) + \alpha_t \left(\frac{\Delta t}{h} \right) \right]^{II} = 0 \quad (7)$$

Three boundary conditions occur at the interface ($r = a$):

$$M_r^I = M_r^{II} \quad (8a)$$

$$\left[\frac{d}{dr}(rM_r) - M_\phi \right]^I = \left[\frac{d}{dr}(rM_r) - M_\phi \right]^{II} \quad (8b)$$

$$\left[\frac{1}{K}(M_\phi - \nu M_r) + \alpha_t \frac{\Delta t}{h} \right]^I = \left[\frac{1}{K}(M_\phi - \nu M_r) + \alpha_t \frac{\Delta t}{h} \right]^{II} \quad (8c)$$

where the superscripts I and II refer to the two plate sectors at the interface. At the origin ($r = 0$)

$$\text{Moments are finite: } (M_{\phi}^I, M_r^I) \quad (8d)$$

Solving the field equations yields moments for the inner plate as follows:

$$M_r^I(r) = -\frac{B_1}{r^2} + \frac{1}{2} C_1(1 + \nu^I) \log r + \frac{1}{4} C_1(1 - \nu^I) + \frac{1}{2} D_1 - \frac{1}{16} (3 + \nu^I) q r^2 \quad (9a)$$

$$M_{\phi}^I(r) = -\frac{B_1}{r^2} + \frac{1}{2} C_1(1 + \nu^I) \log r - \frac{1}{4} C_1(1 - \nu^I) + \frac{1}{2} D_1 - \frac{1}{16} (1 + 3\nu^I) q r^2 \quad (9b)$$

Likewise, solving for the outer plate with no distributed load gives

$$M_r^II(r) = -\frac{B_2}{r^2} + \frac{1}{2} C_2(1 + \nu^{II}) \log r + \frac{1}{4} C_2(1 - \nu^{II}) + \frac{1}{2} D_2 \quad (10a)$$

$$M_{\phi}^II(r) = -\frac{B_2}{r^2} + \frac{1}{2} C_2(1 + \nu^{II}) \log r - \frac{1}{4} C_2(1 - \nu^{II}) + \frac{1}{2} D_2 \quad (10b)$$

The six constants ($B_1, B_2, C_1, C_2, D_1,$ and D_2) are determined from the six boundary conditions. Substituting the numerical values for the parameters yields the following moment solutions:

For the inner plate ($0 \leq r \leq a$)

$$M_r^I(r) = 844.05 - 20.81r^2 \quad (11a)$$

$$M_{\phi}^I(r) = 844.05 - 12.44r^2 \quad (11b)$$

For the outer plate ($a \leq r \leq b$)

$$M_r^II(r) = 2046.63 - (5203.06/r^2) - 1170 \log r \quad (11c)$$

$$M_{\phi}^II(r) = 2676.63 + (5203.06/r^2) - 1170 \log r \quad (11d)$$

The displacement, if required, is obtained by integrating the deformation displacement relations. Calculating the integration for the constraints required using the kinematic boundary equations (refs. 16 and 17). The transverse displacements for the plate are as follows (ref. 16):

For the inner plate ($0 \leq r \leq a$)

$$w^I(r) = 3.1209 - 0.0356r^2 + 0.1757 \times 10^{-3} r^4 \quad (12a)$$

For the outer plate ($a \leq r \leq b$)

$$w^II(r) = 5.3614 - 0.1344r^2 - 0.7296 \log r + 0.04417r^2 \log r \quad (12b)$$

The CBMF required the boundary compatibility conditions given by equations (7) and (8c). Because these conditions were not available, the CBMF could not be developed earlier. The CBMF can be transformed to obtain Navier's displacement method (ref. 18).

GENERATION OF COMPATIBILITY CONDITIONS

Strain, or deformation balance, represents the physical concept behind the compatibility conditions, which are controller type of relations. In elasticity the strains ϵ are controlled, $f(\epsilon_x, \epsilon_y, \dots, \gamma_{zx}) = 0$; or in structures the deformations β are balanced, $f(\beta_1, \beta_2, \dots, \beta_n) = 0$. The compatibility conditions are generated here first for elasticity and then for structures.

Generation of Compatibility Conditions in Elasticity

The field compatibility conditions can be derived by eliminating the displacements from the strain displacement relations (ref. 19). This technique has not yet been successfully extended to deriving the boundary compatibility conditions (BCC). At present the BCC have been generated from the stationary condition of the IFM variational functional. This functional π_s has three terms: A , B , and W . For three-dimensional elasticity π_s has the following explicit form:

$$\pi_s = A + B - W \quad (13)$$

Term A represents the internal energy expressed in terms of stresses and displacements.

$$A = \int_{\text{volume}} \left\{ \sigma_x \frac{\partial u}{\partial x} + \sigma_y \frac{\partial v}{\partial y} + \sigma_z \frac{\partial w}{\partial z} + \tau_{xy} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) + \tau_{yz} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) + \tau_{zx} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) \right\} dV \quad (14a)$$

Term B represents the complementary energy. This term, expressed in terms of strain and stress function, has the following form:

$$B = \int_{\text{volume}} \left\{ \epsilon_x \left(\frac{\partial^2 \phi_3}{\partial y^2} + \frac{\partial^2 \phi_2}{\partial z^2} \right) + \epsilon_y \left(\frac{\partial^2 \phi_1}{\partial z^2} + \frac{\partial^2 \phi_3}{\partial x^2} \right) + \epsilon_z \left(\frac{\partial^2 \phi_2}{\partial x^2} + \frac{\partial^2 \phi_1}{\partial y^2} \right) + \tau_{xy} \left(-\frac{\partial^2 \phi_3}{\partial x \partial y} \right) + \gamma_{xy} \left(-\frac{\partial^2 \phi_1}{\partial y \partial z} \right) + \gamma_{zx} \left(-\frac{\partial^2 \phi_2}{\partial z \partial x} \right) \right\} dV \quad (14b)$$

Term W represents the potential of the work done due to the prescribed traction, displacement, and specified body force:

$$W = \int_{\text{surface } S_1} (\bar{P}_x u + \bar{P}_y v + \bar{P}_z w) ds + \int_{\text{surface } S_2} (P_x \bar{u} + P_y \bar{v} + P_z \bar{w}) ds + \int_{\text{volume}} (\bar{B}_x u + \bar{B}_y v + \bar{B}_z w) dV \quad (14c)$$

where $\sigma_x, \sigma_y, \sigma_z, \tau_{xy}, \tau_{yz},$ and τ_{zx} are the six stress components; $\epsilon_x, \epsilon_y, \epsilon_z, \gamma_{xy}, \gamma_{yz},$ and γ_{zx} are the six strain components; $u, v,$ and w are the three displacement components; $\phi_1, \phi_2,$ and ϕ_3 are the three stress functions; $\bar{P}_x, \bar{P}_y, \bar{P}_z$ are the three prescribed tractions; $P_x, P_y,$ and P_z are reactions where displacements $\bar{u}, \bar{v},$ and \bar{w} are prescribed; and $\bar{B}_x, \bar{B}_y, \bar{B}_z$ are the three body force components. The stress functions are defined as

$$\begin{aligned}
\sigma_x &= \frac{\partial^2 \phi_3}{\partial y^2} + \frac{\partial^2 \phi_2}{\partial z^2} & \sigma_y &= \frac{\partial^2 \phi_1}{\partial z^2} + \frac{\partial^2 \phi_3}{\partial x^2} & \sigma_z &= \frac{\partial^2 \phi_2}{\partial x^2} + \frac{\partial^2 \phi_1}{\partial y^2} \\
\tau_{xy} &= -\frac{\partial^2 \phi_3}{\partial x \partial y} & \tau_{yz} &= -\frac{\partial^2 \phi_1}{\partial y \partial z} & \tau_{xz} &= -\frac{\partial^2 \phi_2}{\partial z \partial x}
\end{aligned} \tag{15}$$

The selection of stress functions and their derivation from the field equations are given in reference 20. The stationary condition of π_s with respect to displacements u , v , and w and stress functions ϕ_1 , ϕ_2 , and ϕ_3 yields all the equations of the completed Beltrami-Michell formulation. The set includes the known elasticity equations (three equilibrium equations and three compatibility conditions in the field; three boundary, equilibrium, or traction conditions; and three displacement continuity conditions on the boundary) available in standard textbooks (ref. 19). The stationary condition also yields the three additional boundary compatibility conditions. These conditions obtained in terms of strains, when expressed in terms of stresses for an isotropic material, are as follows:

$$\begin{aligned}
\frac{\partial}{\partial z} \left\{ a_{vz} (\sigma_y - \nu \sigma_z - \nu \sigma_x) - a_{vy} (1 + \nu) \tau_{yz} \right\} + \frac{\partial}{\partial y} \left\{ a_{vy} (\sigma_z - \nu \sigma_x - \nu \sigma_y) - a_{vz} (1 + \nu) \tau_{yz} \right\} &= 0 \\
\frac{\partial}{\partial x} \left\{ a_{vx} (\sigma_z - \nu \sigma_x - \nu \sigma_y) - a_{vz} (1 + \nu) \tau_{zx} \right\} + \frac{\partial}{\partial z} \left\{ a_{vz} (\sigma_x - \nu \sigma_y - \nu \sigma_z) - a_{vx} (1 + \nu) \tau_{zx} \right\} &= 0 \\
\frac{\partial}{\partial y} \left\{ a_{vy} (\sigma_x - \nu \sigma_y - \nu \sigma_z) - a_{vx} (1 + \nu) \tau_{xy} \right\} + \frac{\partial}{\partial x} \left\{ a_{vx} (\sigma_y - \nu \sigma_z - \nu \sigma_x) - a_{vy} (1 + \nu) \tau_{xy} \right\} &= 0
\end{aligned} \tag{16}$$

where a_{vx} , a_{vy} , and a_{vz} are the direction cosines of the outward normal vector to the boundary surface. The three BCC given by equation (16), missed since 1860, have been derived for the first time for a three-dimensional elastic continuum. These conditions, when added to the Beltrami-Michell formulation, complete the method of stress in elasticity. CBMF solutions for plates and cylindrical shells are reported in references 6 and 16.

Compatibility Conditions for Finite Element Structural Analysis

The compatibility conditions for finite element analysis are obtained as an extension of Saint-Venant's strain formulation in elasticity. The two steps of strain formulation are

1. Formulation of the strain displacement relations
2. Elimination of the displacements to obtain the compatibility conditions

For an illustration of the strain formulation consider the plane stress problem with its strain displacement relations:

$$\epsilon_x = \frac{\partial u}{\partial x} \quad \epsilon_y = \frac{\partial v}{\partial y} \quad \gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \tag{17a}$$

The field CC are obtained by eliminating the displacements:

$$\frac{\partial^2 \epsilon_x}{\partial y^2} + \frac{\partial^2 \epsilon_y}{\partial x^2} - \frac{\partial^2 \gamma_{xy}}{\partial x \partial y} = 0 \tag{17b}$$

For discrete structures the deformation displacement relations (DDR) are equivalent to the strain displacement relations in elasticity. The DDR can be obtained by using the equality relation between internal strain energy and external work (ref. 7) as follows:

$$\{\beta\} = [\mathbf{B}]^T \{X\} \quad (18)$$

where $[\mathbf{B}]$ is the $m \times n$ rectangular equilibrium matrix of the EE, $[\mathbf{B}]\{F\} = \{P\}$; $\{\beta\}$ is the n -component deformation vector; and $\{X\}$ and $\{P\}$ are m -component displacement and load vectors, respectively. The DDR contain $r = n - m$ constraints on deformations, which represent the r compatibility conditions. The CC are obtained by eliminating the m displacements from the n DDR as follows (ref. 8):

$$[\mathbf{C}]\{\beta\} = \{0\} \quad (19)$$

The deformation $\{\beta\}$ of structures is analogous to strain in elasticity. The rectangular and banded compatibility matrix $[\mathbf{C}]$ of dimension $r \times n$, has a full-row rank of r . It is independent of sizing design parameters (such as the area of the bars and the moments of inertia of the beams), material properties, and external loads (ref. 7).

Compliance of Compatibility Conditions

Compliance of the compatibility in the finite element stiffness method is illustrated by considering the example of a plane stress elasticity problem. The field compatibility condition $\left(\frac{\partial^2 \epsilon_x}{\partial y^2} + \frac{\partial^2 \epsilon_y}{\partial x^2} - \frac{\partial^2 \gamma_{xy}}{\partial x \partial y} = 0 \right)$, when expressed in terms of continuous displacement variables u and v produces a trivial condition (or an identity $\{f(u,v) - f(u,v) = 0\}$). The field compliance is well recognized in the stiffness method. The boundary compatibility condition expressed in terms of strains has the following form:

$$a_{vx} \frac{\partial}{\partial x} \epsilon_y + a_{vy} \frac{\partial}{\partial y} \epsilon_x - \frac{1}{2} \left[a_{vx} \frac{\partial}{\partial x} \gamma_{xy} + a_{vy} \frac{\partial}{\partial y} \gamma_{xy} \right] = 0 \quad (20a)$$

When expressed in terms of continuous displacements u and v the boundary compatibility condition becomes a non-trivial function as follows:

$$\Re = a_{vx} \left[\frac{\partial^2 v}{\partial x \partial y} - \frac{1}{2} \left(\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 u}{\partial x \partial y} \right) \right] + a_{vy} \left[\frac{\partial^2 u}{\partial x \partial y} - \frac{1}{2} \left(\frac{\partial^2 u}{\partial y^2} + \frac{\partial^2 v}{\partial x \partial y} \right) \right] \neq 0 \quad (20b)$$

The boundary compatibility condition expressed in terms of the displacement is not automatically satisfied. Consider a simple finite element model with a four-node rectangular element and a three-node triangular element, as shown in Fig. 4. Along the interface connecting nodes 2 and 4 the compliance of the boundary compatibility can be imposed by the following residue function:

$$\Re^{\text{interface}} = \Re^{\text{rectangular element}} + \Re^{\text{triangular element}} = 0 \quad (21)$$

Consider displacement functions for the rectangular and triangular membrane elements (ref. 21) as follows:

For a rectangular element

$$u^{\text{rectangular}}(x, y) = c_1 x + c_2 xy + c_3 y + c_4 \quad (22)$$

Likewise the $v^{\text{rectangular}}(x, y)$ can be written. The eight constants (c_1, c_2, \dots, c_8) can be linked to the eight nodal displacements of the rectangular element.

For a triangular element

$$\text{triangular } u(x, y) = d_1x + d_2y + d_3 \quad (23)$$

Likewise the $\text{triangular } v(x, y)$ can be written. The six constants (d_1, d_2, \dots, d_6) can be linked to the six nodal displacements of the triangular element.

The contribution to the residue function \mathfrak{R} for each of the two elements can be obtained as follows:

$$\mathfrak{R}^{\text{element-rectangular-(2-4)}} = 0.5\{a_{vx}c_6 + a_{vy}c_2\} \quad (24)$$

$$\mathfrak{R}^{\text{element-triangular-(2-4)}} = 0 \quad (25)$$

Thus, the boundary compliance ($\mathfrak{R}^{\text{rectangular element}} + \mathfrak{R}^{\text{triangular element}} \neq 0$) at the interface of the finite element model shown in Fig. 4 is not satisfied.

In the finite element analysis the traditional assumption that the stiffness method satisfies the compatibility condition a priori needs to be reviewed with respect to compliance of the boundary compatibility conditions. In the stiffness method continuity is imposed on the nodal displacements (such as, for example, at nodes 2 and 4 in Fig. 4). The displacement continuity condition can not be sufficient for the compliance of the boundary compatibility condition in the interface (generated by nodes 2 and 4 in Fig. 4).

BENEFITS FROM COMPATIBILITY CONDITIONS

In this paper we discuss the benefits gained from using the compatibility conditions in finite element analysis. Similar benefits in continuum analysis (refs. 16, 17, and 22), design optimization, and structural testing are described in references 12, 23, and 24. To quantify the benefit, we have developed a research-level finite element code entitled IFM/Analyzers. This code provides for three analysis methods: integrated force method, dual integrated force method, and stiffness method. The IFM and IFMD equations, programmed in IFM/Analyzers code, are summarized next.

Equations of Integrated Force Method for Static Analysis

The IFM equations for a finite element model with n force and m displacement unknowns are obtained by coupling the m equilibrium equations ($[\mathbf{B}]\{F\} = \{P\}$) to the $r = n - m$ compatibility conditions ($[\mathbf{C}][\mathbf{G}]\{F\} = \{\delta R\}$):

$$\begin{bmatrix} [\mathbf{B}] \\ [\mathbf{C}][\mathbf{G}] \end{bmatrix} \{F\} = \begin{Bmatrix} \{P\} \\ \{\delta R\} \end{Bmatrix} \quad \text{or} \quad [\mathbf{S}]\{F\} = \{P^*\} \quad (26)$$

From the internal forces $\{F\}$ the displacements $\{X\}$ are backcalculated as

$$\{X\} = [\mathbf{J}]\{[\mathbf{G}]\{F\} + \{\beta^0\}\} \quad (27)$$

where $[\mathbf{J}] = m$ rows of $\{[\mathbf{S}]^{-1}\}^T$

In equations (26 and 27) $[\mathbf{B}]$ is the $m \times n$ rectangular equilibrium matrix, $[\mathbf{G}]$ is the $n \times n$ flexibility matrix, $[\mathbf{C}]$ is the $r \times n$ compatibility matrix, $\{\delta R\} = -[\mathbf{C}]\{\beta^0\}$ is the r -component effective initial deformation vector, $\{\beta^0\}$ is the initial deformation vector of dimension n , $[\mathbf{S}]$ is the IFM governing matrix, and $[\mathbf{J}]$ is the $m \times n$ deformation coefficient matrix. The IFM matrices and vectors are generated in references 25 and 26.

Equations of Dual Integrated Force Method for Static Analysis

A dual formulation of the primal IFM has been developed. The dual integrated force method has been obtained by mapping forces into displacements at the element level. Like the IFM the dual method has two sets of equations. The primary set is used to calculate the displacements. Forces are backcalculated from the secondary set of equations. IFM and IFMD are analytically equivalent—producing identical solutions for stress, displacement, frequency, and buckling load. The IFMD governing equations are

$$[\mathbf{K}]_{\text{IFMD}} \begin{Bmatrix} X \\ m \times 1 \end{Bmatrix} = \begin{Bmatrix} P \\ m \times 1 \end{Bmatrix}_{\text{IFMD}} \quad (28)$$

$$\{F\} = [\mathbf{G}]^{-1}[\mathbf{B}]^T \{X\} - [\mathbf{G}]^{-1}\{\beta\}^0 \quad (29)$$

where

$$[\mathbf{K}]_{\text{IFMD}} = [\mathbf{B}][\mathbf{G}]^{-1}[\mathbf{B}]^T$$

$$\{P\}_{\text{IFMD}} = \left\{ \{P\} + ([\mathbf{B}][\mathbf{G}]^{-1}\{\beta\}^0) \right\}$$

The IFMD primary equations and the stiffness method equations appear to be similar, but the coefficients of the stiffness matrix and that of the dual matrix $[\mathbf{K}]_{\text{IFMD}}$ differ in magnitude (ref. 10).

Numerical Examples

The numerical examples presented use three elements of the IFM/Analyzers code: (1) a quadrilateral membrane element, QUAD0405; (2) a hexagonal solid element, HEX2090; and (3) a plate flexure element, PLB0409. The generation of the elemental equilibrium matrix $[\mathbf{B}^e]$ in IFM uses both the force and displacement fields, even though the final equation is expressed in terms of elemental forces ($[\mathbf{B}^e]\{F^e\} = \{P^e\}$). The role of the displacement field in $[\mathbf{B}^e]$ resembles the concept of virtual displacement. The generation of the elemental matrices uses numerical integration but does not use reduced integration or bubble function techniques. In the numerical examples the performance of the simple IFM elements is compared with that of the popular stiffness elements. The basic attributes of the IFM elements are

1. QUAD0405: This is a four-node element with two displacements (u and v) and three stresses (σ_x , σ_y , and τ_{xy}). The element has eight displacement degrees of freedom. Five force unknowns represent the discretization of the three stresses (σ_x , σ_y , and τ_{xy}). Normal stresses σ_x and σ_y vary linearly while the shear stress τ_{xy} is constant.
2. HEX2090: This is a 20-node element with three displacements (u , v , and w) and six stresses (σ_x , σ_y , σ_z , τ_{xy} , τ_{xz} , and τ_{yz}). Displacements are discretized to obtain 60 degrees of freedom. All six stress components are represented by 90 force unknowns.
3. PLB0409: This is a four-node plate bending element with transverse displacement w and three moments (M_x , M_y , and M_{xy}). The displacement is discretized to obtain 12 degrees of freedom. All three moments are represented by nine force unknowns.

Numerical example 1: cantilever beam modeled by membrane elements.—The cantilever beam shown in Fig. 5(a) is 24 in. long, 2 in. deep, and 0.25 in. thick. It is made of steel with Young's modulus of 30×10^6 psi and Poisson's ratio of 0.30 and is subjected to 200-lb load at its free end. The beam was modeled with the QUAD0405 element of IFM/Analyzers and the CQUAD4 element of a popular commercial code. The stiffness elements (CQUAD4 of the commercial code and QUAD0405 of IFM/Analyzers) supported the linear stress field along the

beam depth. Thus, only discretization along the beam length was considered, which, however, is the standard practice for comparison (ref. 27). The solution for the tip displacement normalized with respect to material strength is depicted in Fig. 5(b). The error in the displacement for a four-element model obtained by IFM/Analyzers was 1.1 percent versus 10 percent for the commercial code. For an eight-element model the error was reduced to zero by IFM. The converged commercial code solution exhibited an error of about 9 percent even for a dense model with 48 elements. For this example a converged solution by the commercial stiffness code was found to be incorrect.

Numerical example 2: cantilever beam modeled by solid elements.—The cantilever beam shown in Fig. 6(a) is 12 in. long, 1 in. deep, and 1 in. thick. It is made of steel with Young's modulus of 30×10^6 psi and Poisson's ratio of 0.30 and is subjected to 10-lb load at its free end. The beam was modeled with hexahedral elements. For IFM/Analyzers the HEX2090 element was used. A 20-node CHEXA element was used for the commercial code. The displacement, frequency, and stress solutions for different finite element models are given in Figs. 6(b) and (c). The maximum displacement and fundamental frequency generated by IFM and the commercial code converged at about the same rate. Difficulty was encountered for stress convergence. The stress patterns generated by IFM and the stiffness method looked similar. However, against a 720-psi beam solution, IFM converged to 630 psi, but the stiffness method could produce only 356 psi for a six-element model.

Numerical example 3: a plate flexure problem.—A rectangular clamped plate, shown in Fig. 7(a), subjected to a concentrated load of 1000 lb, was solved by using the PLB0409 element of IFM/Analyzers and by the CQUAD4 element of a commercial code. The plate is made of steel with Young's modulus of 30×10^6 psi and Poisson's ratio of 0.30. Its dimensions (a , b , and thickness t , see Fig. 7) are 24, 12, and 0.25 in., respectively. Results normalized with respect to Timoshenko's solutions (ref. 18) are depicted in Figs. 7(b) and (c). The central displacement obtained by IFM achieved an accuracy of 98.7 percent for a model with eight elements. For a coarser model with only four elements the error was about 17.5 percent. The solution obtained by the commercial code required 64 elements to achieve an accuracy of 98 percent. The performance of a second commercial code followed the pattern of the first code with some variation. For the bending moment convergence occurred with the PLB0409 element, using a model with four elements per quarter plate. For a model with one element per quarter plate the IFM solution exhibited a 20 percent error. For the commercial code the bending moment exhibited an error of about 7 percent even for a fine model with 16 elements per quarter plate.

For the three examples the integrated force method outperformed the stiffness method, overshadowing the simplicity at the element level.

DISCUSSION

The discussion is separated into the completeness of the theory, the quality of analytical predictions, and the near-term research.

Completeness of Theory

The theory of structures began before the birth of Newton in the notebooks of Leonardo da Vinci and through Galileo Galilei's studies on structures (ref. 1). The growth accelerated during the nineteenth century as the eminent scientists and engineers Cauchy, Navier, Saint-Venant, Castigliano, and others worked on different applications (see Fig. 8). It was commonly believed that all fundamental concepts of this four-century-old science were fully understood before the turn of the twentieth century. This belief was not correct. The traditional theory of compatibility camouflaged a deficiency described in this paper.

If the compatibility conditions were not required, the theory of (determinate) structures could be covered in a few undergraduate courses. Satisfying the compatibility conditions makes the theory a research topic practiced at doctoral and postdoctoral levels around the world. Here we digress to speculate on the possible reasons for the tardy incorporation of the compatibility conditions into the theory of structures.

The compatibility conditions are more complex relationships than the equilibrium equations. However, it is not likely that their complexity would have been an insurmountable obstacle for Cauchy, Saint-Venant, Navier, or Maxwell (see Fig. 8).

Complacency could be another possible reason, but observed inconsistencies between different approaches to the solution of indeterminate problems suggest that this could not have been the primary reason. For example, displacements are the unknowns in the displacement method, but redundants, instead of forces, became the unknowns in the traditional force method. Also, the displacement method could solve all three types of boundary value problems in elasticity, but the Beltrami-Michell formulation could handle only stress boundary value problems. The redundant force method in structures did not parallel the Beltrami-Michell formulation in elasticity. Even novice researchers would surely have noticed and questioned the lack of uniformity between the methods in elasticity and structures.

Often an approximate solution was adequate to fulfill the need of an industry that may have had little interest in the particular method used. Take, for example, the building and bridge industries, which require analysis of indeterminate trusses, continuous beams, frames, and other structures. The redundant force method solved such problems. For plates and shells solutions obtained through the superposition technique served the industry well. Solving engineering problems not only became central to the work but also occupied most of the available time of competent researchers, leaving little or no time for them to ponder or address the deficiencies and completion of the theory of compatibility. The engineering problem-solving aspect of structural mechanics was considered to be the most important. The dividends associated with solving industrial problems are, in our opinion, the primary reason behind the slow progress of the compatibility conditions. Complexity and complacency may be considered but secondary reasons.

It is important to evaluate the completeness of the theory of structures as a science, as opposed to merely solving industrial problems. The current acceptance of the stiffness solution by industry should not lead to neglect of the discipline, which may have entered a developmental plateau. Research on compatibility can reinvigorate the structures discipline and eventually provide a robust alternative formulation for solving industrial structures problems.

Quality of Analytical Prediction

The quality of analytical predictions can be directly controlled by imposing the requirement that the equilibrium equations and the compatibility conditions be simultaneously satisfied. The discretization error can be reduced through mesh refinement. The integrated force method adopts this approach and produces accurate solutions even when the structure is modeled with a modest size of finite element mesh. Even though emphasis on and clever manipulation of one of the two equation sets may appear to be adequate for the analysis of some problems, the reliability of the solution thus obtained cannot always be guaranteed. For example, the monotonically converged stiffness solution in Fig. 5 was erroneous.

Consider next a deficiency observed by Thodhunter (ref. 28) while he was scrutinizing Astronomer Royal Airy's attempt to analyze the stress in cantilever and simply supported beams. Airy formulated a potential function to satisfy the equilibrium equations. He manipulated the equilibrium equations and the boundary conditions to generate the solution for the examples. However, his results were incorrect because he neglected the compatibility compliance. Thodhunter (ref. 28) describes the situation:

Important Addition and Correction. The solution of the problems suggested in the last two Articles were given—as has already been stated—on the authority of a paper by the late Astronomer Royal, published in a report of the British Association. I now observe, however—when the printing of the articles and engraving of the Figures is already completed—that they cannot be accepted as true solutions, inasmuch as they do not satisfy the general equations (164) of § 303 [note that the equations in question are the compatibility conditions]. It is perhaps as well that they should be preserved as a warning to the students against the insidious and comparatively rare error of choosing a solution which satisfies completely all the boundary conditions, without satisfying the fundamental condition of strain [note that the condition in question is the compatibility condition], and which is therefore of course not a solution at all.

The advice of Thodhunter should be followed to generate solutions by simultaneously satisfying all the equilibrium equations and all the compatibility conditions.

Near-Term Research

Equilibrium and compatibility constitute the two fundamental equations of the theory of structures, as depicted in the two halves of the pie diagram in Fig. 9. The immaturity in the compatibility conditions is represented by the shaded quarter. The theory of structures has been developed by using the information contained in three-quarters of the pie diagram. Using the additional quarter should improve the theory and the performance of the analysis methods. Near-term research should address at least two issues: unification of the theory, and compliance of the boundary compatibility conditions.

Unification of theory.—Using the compatibility conditions can lead to the unification of the theory of structures, allowing free movement between different analysis variables as well as the various methods of structural analysis. In discrete element analysis we can move from displacement to stress through a set of algebraic equations and vice versa. Likewise we should be able to move from force method to displacement method. For example, we can move from the integrated force method to the dual integrated force method, both methods yielding identical solutions. Even though the IFM and IFMD produce identical solutions, each method has unique attributes. The primal IFM is more suited to design optimization because this method generates closed-form sensitivities for stress, displacement, and frequency constraints (ref. 12). An analytical initial design to begin optimization iteration can be obtained by IFM (ref. 29). Singularity in optimization and behavior of constraints are elegantly treated by IFM (refs. 30 to 32). The dual IFMD is not as suitable as the primal IFM for design calculations. The governing matrix of IFMD is symmetrical. Therefore, IFMD can use solvers that have been obtained for the stiffness method. This approach should be extended for continuum analysis, reducing any barrier between elasticity and structures. For example, the force method concept should be consistent between structures and elasticity (like the integrated force method and the completed Beltrami-Michell formulation).

Satisfaction of boundary compatibility conditions.—The traditional solutions and other analysis methods must be checked for the compliance of the boundary compatibility conditions. Compliance of the BCC at the element interface in the stiffness-method-based finite element analysis should be reviewed. The stiffness elements, if necessary, should be modified for the compliance of the BCC.

CONCLUSIONS

The theory of structures has concealed a deficiency that has been present in the compatibility conditions since Saint-Venant's strain formulation in 1860. We have researched and addressed these important conditions in structures and elasticity.

Because of the compatibility deficiency the direct methods (integrated force method in structures and completed Beltrami-Michell formulation in elasticity) that bestow simultaneous emphasis on stress equilibrium and strain compatibility were not available during the formative 1960's. Despite the deficiency structural analysis has made magnificent progress through the stiffness method. However, in academia research into the stiffness method has entered a developmental plateau. Bringing back the method of forces can reinvigorate the discipline and provide a robust alternative formulation for solving industrial structural problems.

We can more precisely quantify the accuracy of predictions by comparing the force and stiffness method solutions and balance the monopoly that is currently biased toward the stiffness method. The new methods can bring value-added benefits in structural analysis, structural design, and structural testing.

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TABLE I.—METHODS OF STRUCTURAL MECHANICS AND ASSOCIATED VARIATIONAL FUNCTIONALS

Method number	Method		Primary variable		Variational functional
	Elasticity	Structures	Elasticity	Structure	
1	Completed Beltrami-Michell formulation	Integrated force method	Stress	Force	IFM variational functional
2	Airy formulation	Redundant force method	Stress function	Redundant	Complementary energy
3	Navier formulation	Stiffness method	Displacement	Deflection	Potential energy
4	Hybrid method	Reissner method	Stress and displacement	Force and deflection	Reissner functional
5	Total formulation	Washizu method	Stress, strain, and displacement	Force, deformation, and deflection	Washizu functional

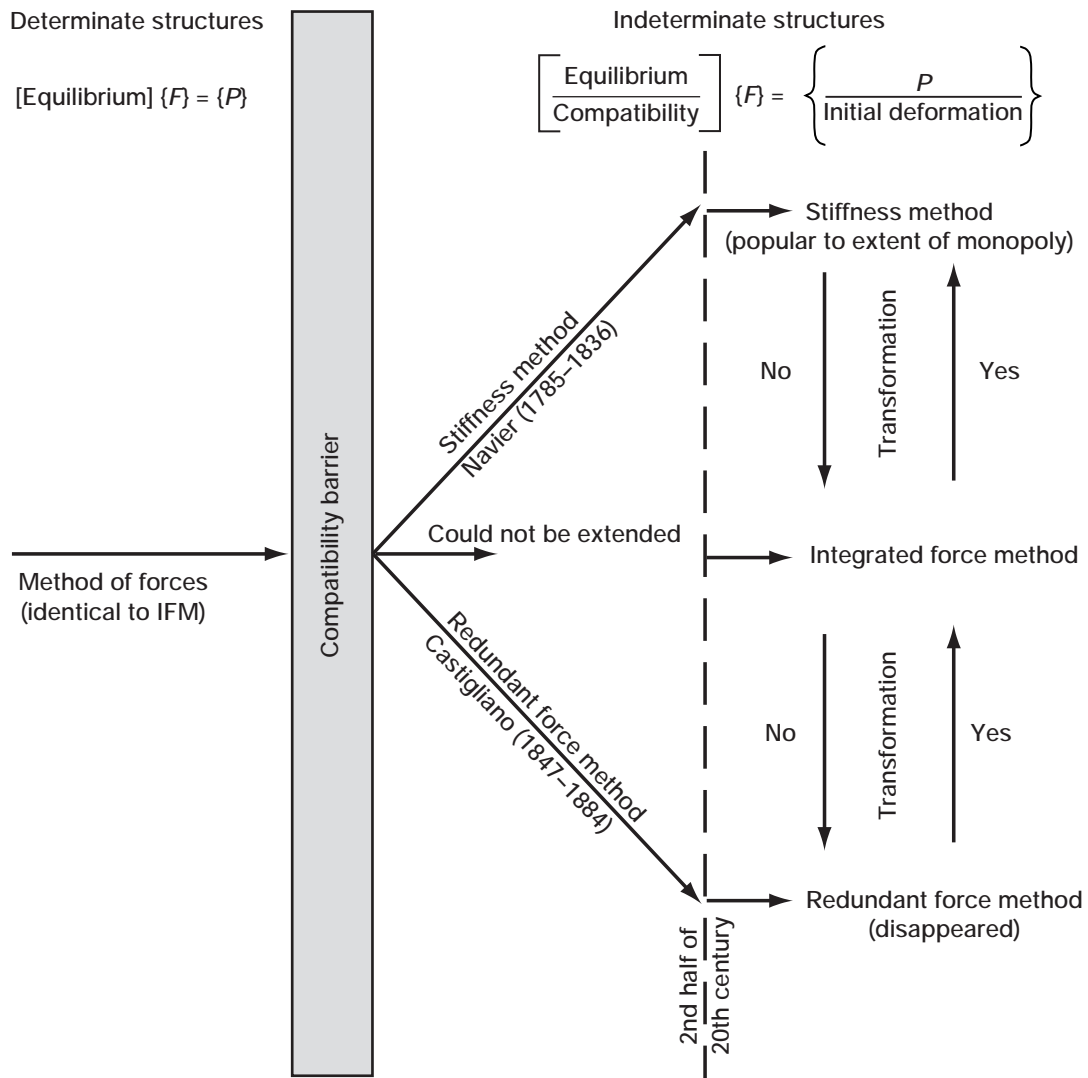


Figure 1.—Compatibility barrier to extending force method for indeterminate structures.

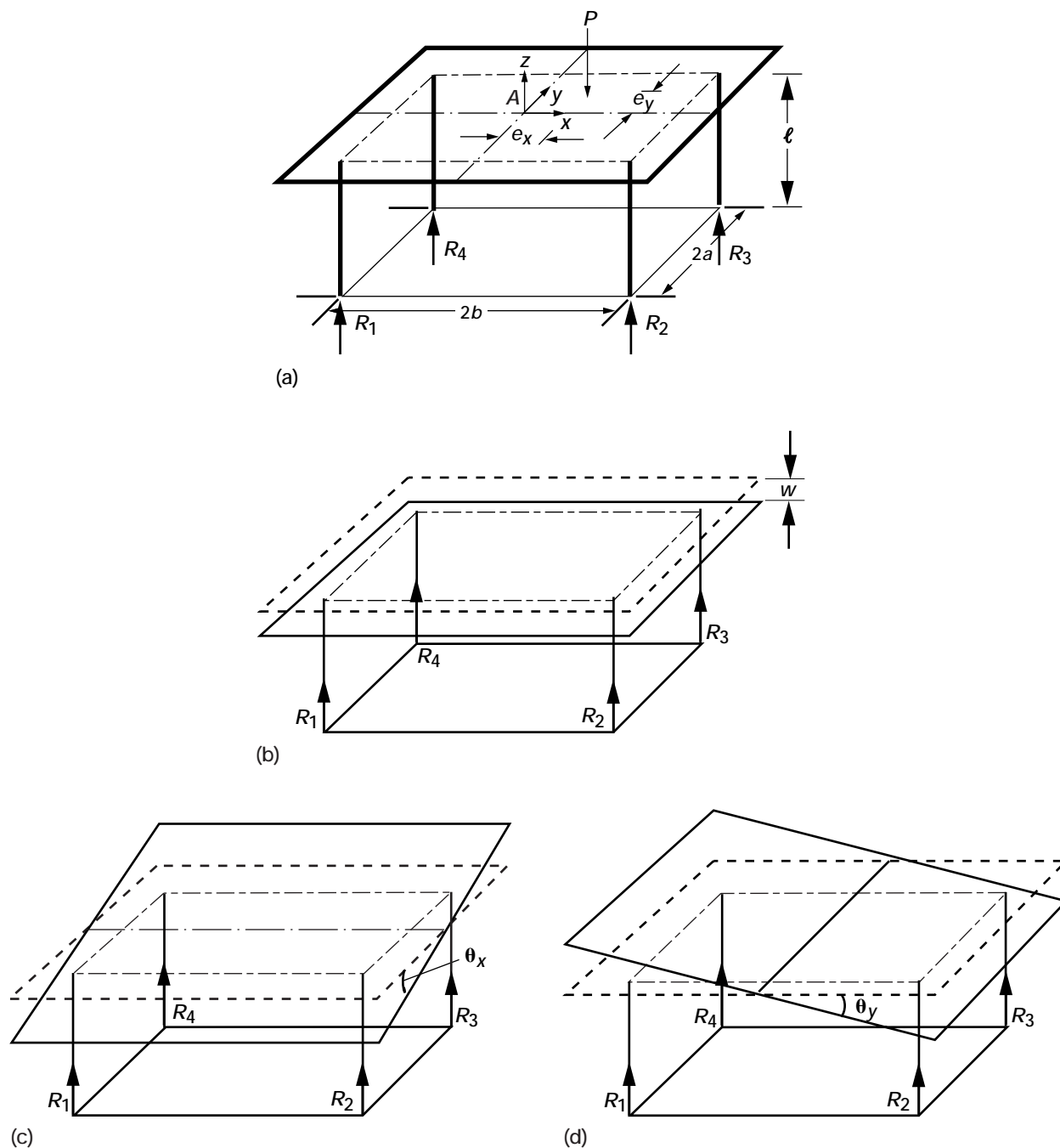


Figure 2.—IFM solution to Navier's table problem. (a) Indeterminate table problem. (b) Average displacement along z-axis. (c) Average tilt about x-axis. (d) Average tilt about y-axis.

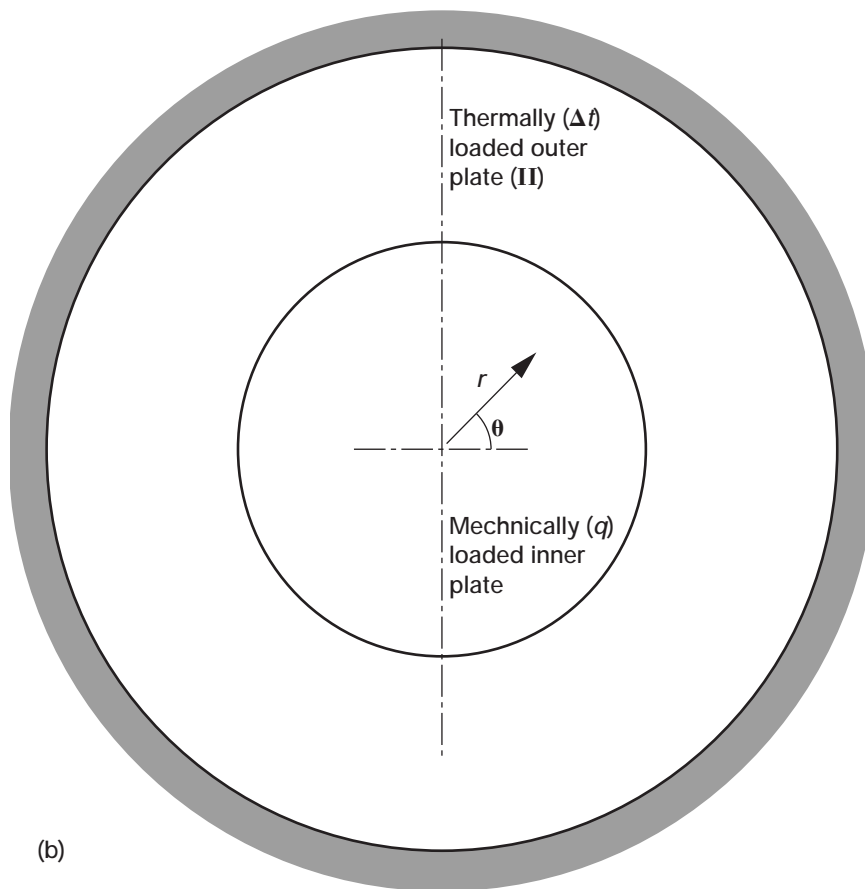
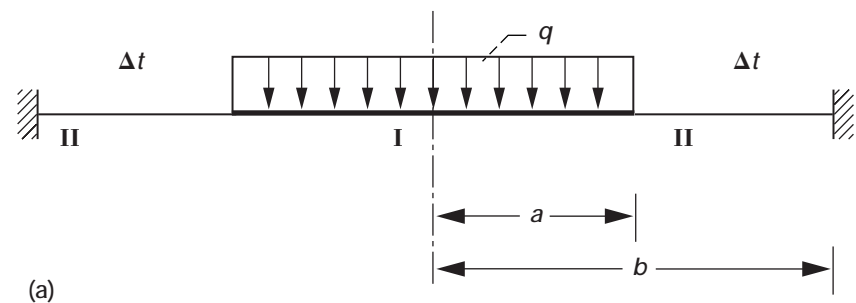


Figure 3.—Composite circular plate subjected to uniform load q and temperature differential Δt . (a) Side view of plate. (b) Planform of plate.

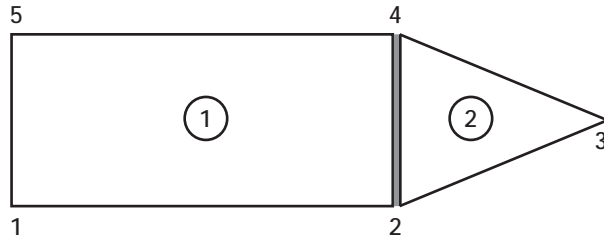


Figure 4.—Boundary compatibility compliance for two-element model.

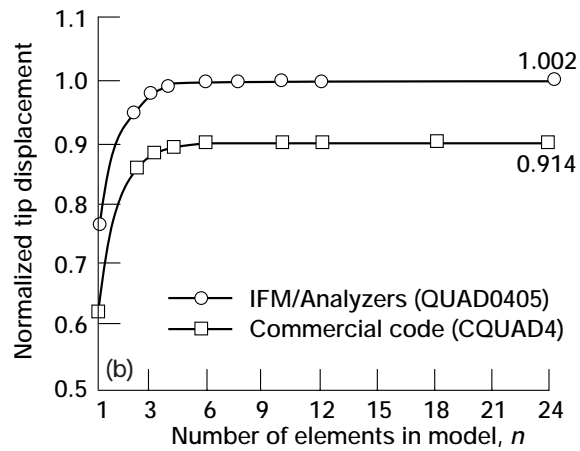
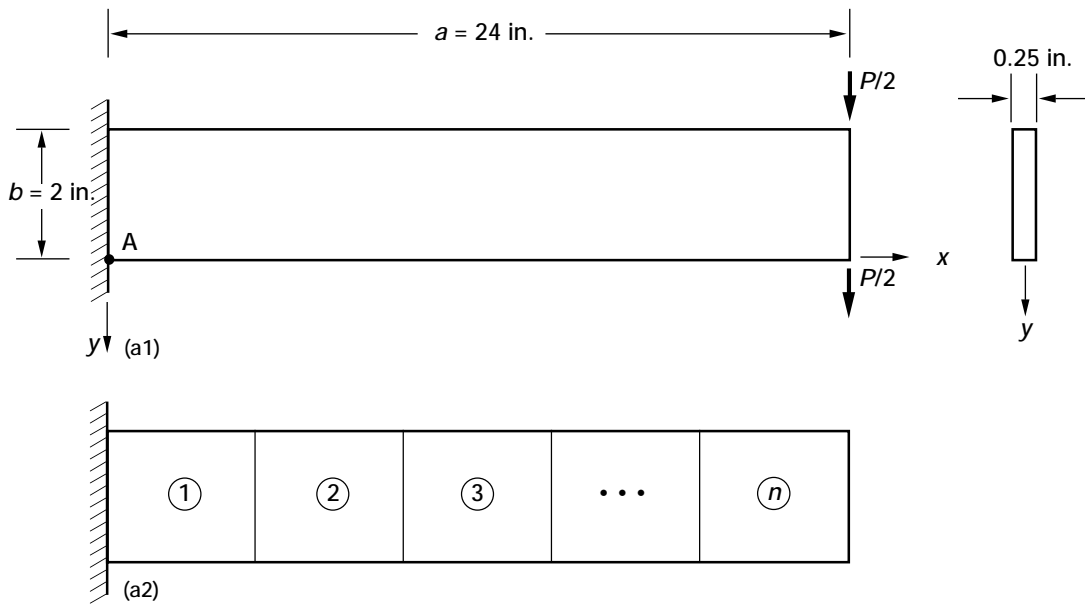
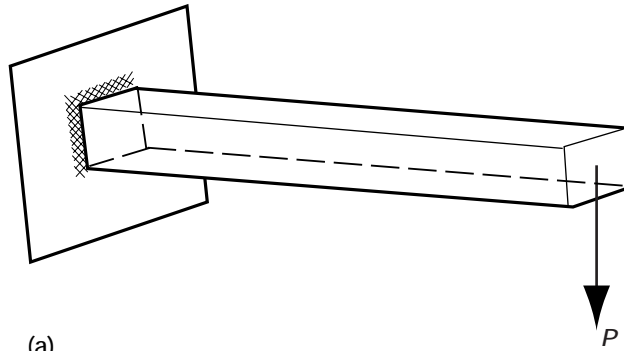


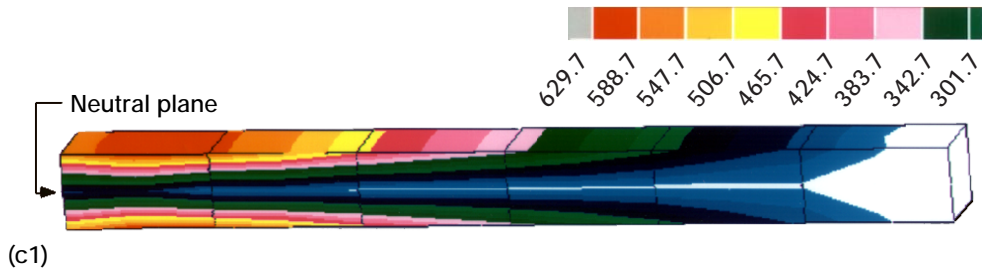
Figure 5.—Numerical example 1: Analysis of cantilever beam. (a1) Geometry and boundary conditions. (a2) Finite element model. (b) Convergence of cantilever beam tip displacement.



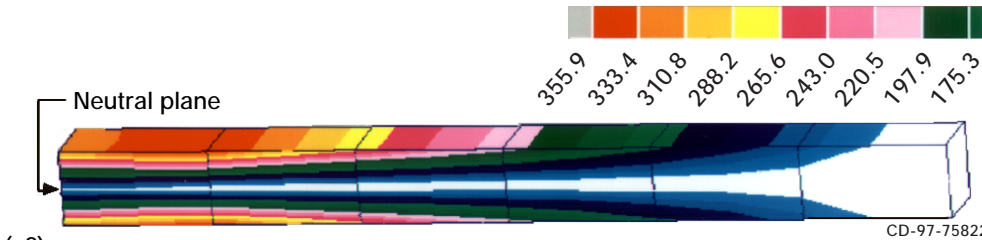
(a)

Model	Normalized displacement		Frequency	
	IFM/IFMD	Commercial code	IFM/IFMD	Commercial code
2x1x1	0.802	0.790	1.198	1.182
3x1x1	.956	.949	1.037	1.032
4x1x1	.985	.979	1.012	1.008
5x1x1	.993	.988	1.005	1.000
6x1x1	.996	.991	1.003	1.000
7x1x1	.997	.993	1.002	.999
7x2x2	.995	.995	1.001	1.001
7x4x4	.997	.996	1.001	1.010

(b)



(c1)



(c2)

CD-97-75822

Figure 6.—Numerical example 2: Stress analysis of cantilever beam. (a) Geometry. (b) Displacement and frequency solution. (c) Stress solution. (c1) IFM/IFMD: stress, 629.7 psi. (c2) Commercial code: stress, 355.9 psi.

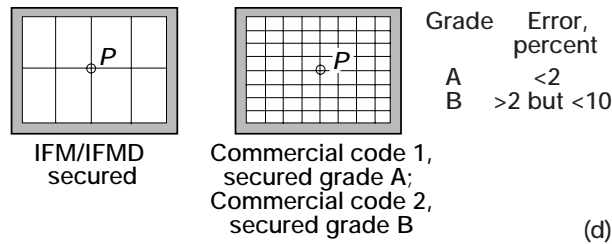
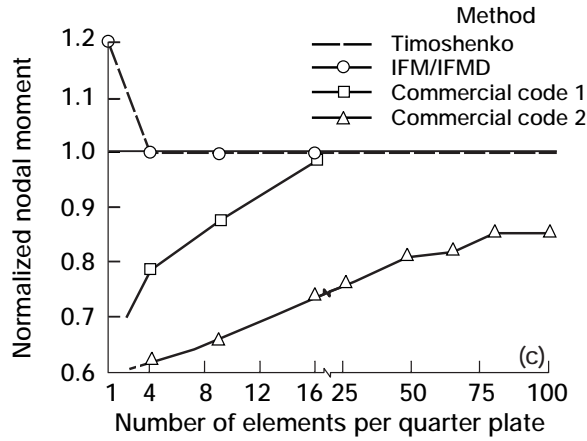
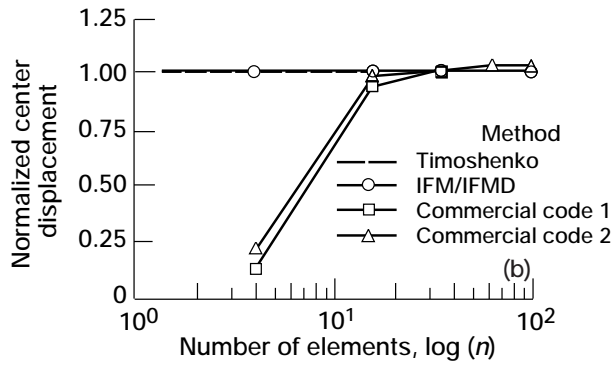
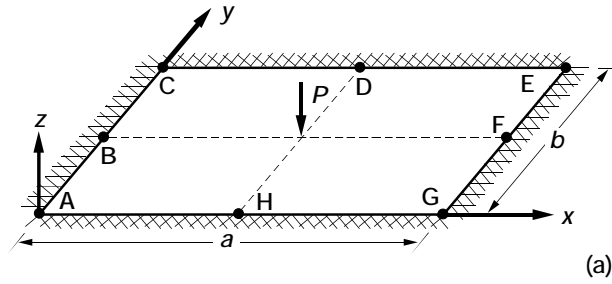


Figure 7.—Numerical example 3: Analysis of clamped plate. (a) Geometry. (b) Convergence of displacement. (c) Convergence of stress. (d) Finite element models.

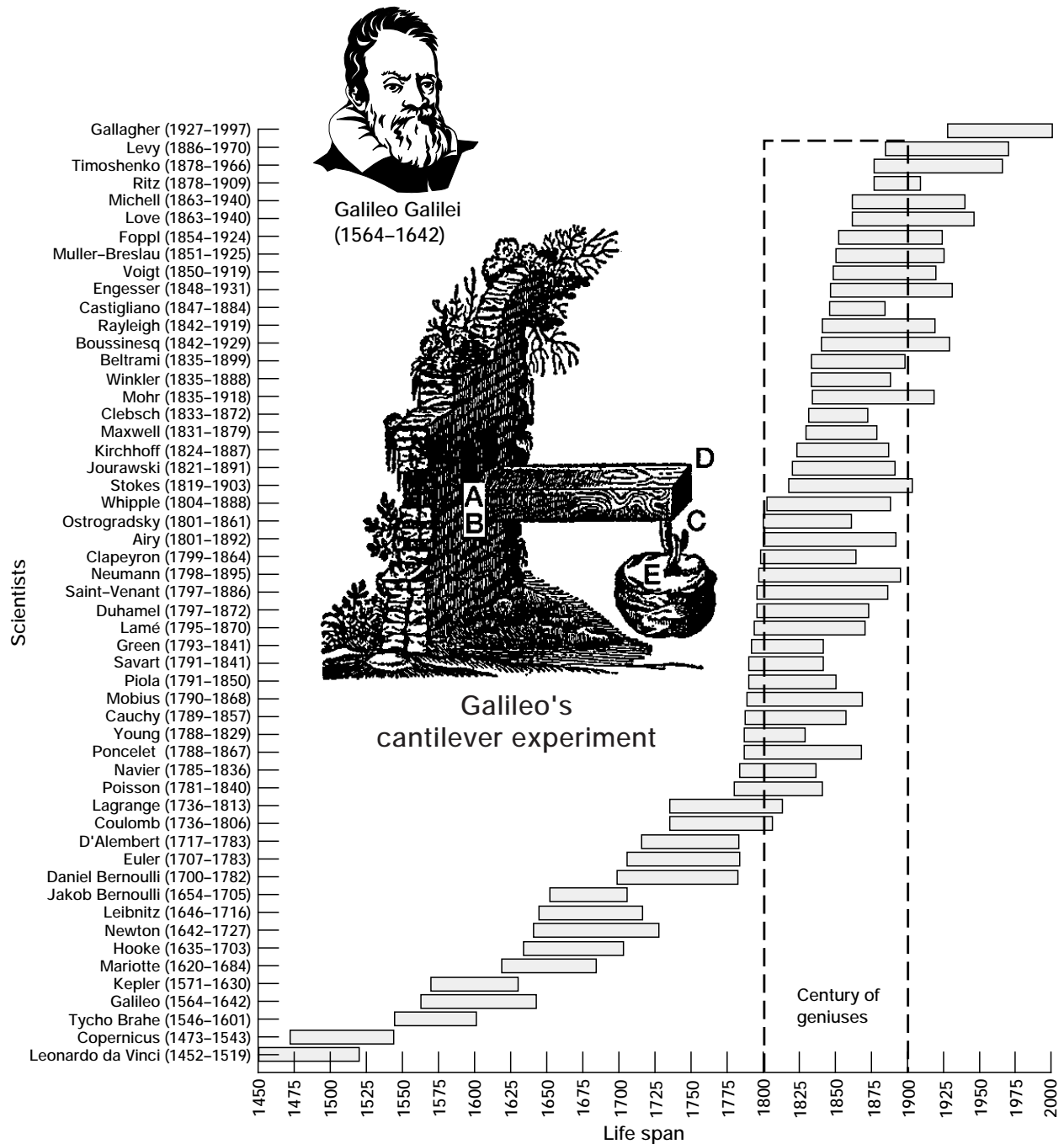


Figure 8.—Scientists who contributed to structural science.

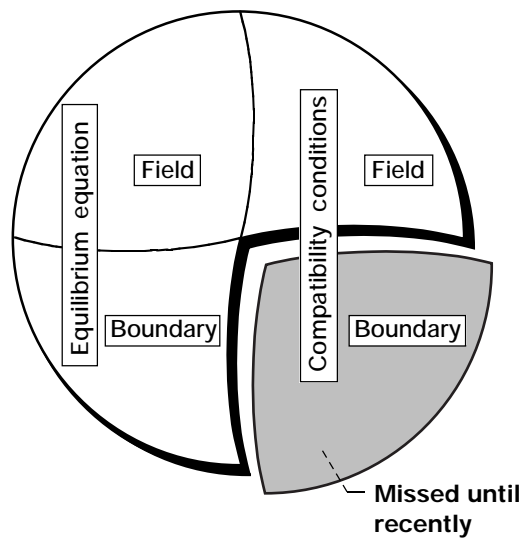


Figure 9.—Equilibrium equations and compatibility conditions in elasticity.

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13. ABSTRACT (Maximum 200 words) The theory of elasticity has camouflaged a deficiency in the compatibility formulation since 1860. In structures the ad hoc compatibility conditions through virtual "cuts" and closing "gaps" are not parallel to the strain formulation in elasticity. This deficiency in the compatibility conditions has prevented the development of a direct stress determination method in structures and in elasticity. We have addressed this deficiency and attempted to unify the theory of compatibility. This work has led to the development of the integrated force method for structures and the completed Beltrami-Michell formulation for elasticity. The improved accuracy observed in the solution of numerical examples by the integrated force method can be attributed to the compliance of the compatibility conditions. Using the compatibility conditions allows mapping of variables and facile movement among different structural analysis formulations. This paper reviews and illustrates the requirement of compatibility in structures and in elasticity. It also describes the generation of the conditions and quantifies the benefits of their use. The traditional analysis methods and available solutions (which have been obtained by bypassing the missed conditions) should be verified for compliance of the compatibility conditions.				
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