

## Effects of adding sisal and glass fibers on the mechanical behaviour of concrete polymer

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**Abstract.** In this study, we investigated the influence of the addition of sisal and glass fibres on the mechanical properties of polymer concrete (PC). These types of concrete are used in many modern civil engineering applications. The prismatic specimens sized according to ASTM C580-02 were elaborated with a PC constituted by 14% constant mass of polyester resin matrix, a granular skeleton based on sand and powder marble. The reinforcement with 60% sand and 26% marble powder adopted in this investigation is the best formulation found in previous authors work. This composition was reinforced by 1 and 2% of sisal and glass fibres, the first one having lengths of 6 mm or 12 mm, however, the second unidirectional cut into bands. These specimens were subjected to 3-point bending monotonic loading. The results obtained were discussed and compared with those obtained for control beams without fibres reinforcement. It is important to note that the incorporation of the glass fibre contributes to an increase of the ultimate load of the polymer composite material produced, however, the addition of the sisal fibre lead to its decreases. In addition, the incorporation of 2% of sisal fibre having 6 mm length leads to a reduction of 26% of the mass of the specimens.

**Key words:** Polymer concrete, sisal and glass fibres, marble, resin, sand.

### 1. Introduction

Polymer concrete (PC) is a material manufactured completely or partially by replacing the Portland cement with a polymer. Since the 1980s, research and development of polymer concrete and mortars have been growing rapidly in various Western countries. The substitution of Portland cement with a polymer results in a substantial increase in the cost of concrete, this should only be done if the cost of labour is lower or if the energy requirements for manufacturing and implementation are lower (Blaga and Beaudoin, 1985).

At present, the PC can be used very efficiently due to its high strength and lightness, made of prefabricated elements in the building, such as: bridge decks, hazardous waste containers, industrial machine bases, floor tile manufacture with synthetic marble and stair panels, facade panels and panels of various structures and window sills (Ohama, 2010).

To improve the physical and mechanical properties, researchers have been interested in optimizing the PC formulation by reducing the resin mass fraction and/or replacing or adding part of the granular skeleton by other components (Gorninski, 2004; Haidar, 2011; Elalaoui, 2012). As a result, the PC's qualities are improved while respecting economic requirements.

Moreover, the growing demand for environmentally friendly materials has led researchers to focus on the incorporation into the PC, industrial waste residue as reinforcement (fly ash, tire rubber, foundry sands, glass powder waste, marble powder etc...) to make one hand more powerful, lighter and less costly (Bignozzi, 2000 ; Ribeiro, 2013). Castro et al.(2013), Saribiyik et

al. (2013) found that the incorporation of a 30% amount of glass powder waste into the PC leads to an increase in bending and compression stresses by 78 and 29% respectively.

In order to reduce the cost of PC produced with synthetic fibres (ie carbon, Kevlar, glass, etc.), the latter are replaced by natural fibres such as jute, sisal, flax, coco, banana, etc., which have gained considerable importance as reinforcements in polymer matrix composites. Compared to synthetic fibre, natural ones issue from plants (lignocelluloses) has the advantage of being very light, issue from a renewable source, cheap and widely available in fibrous form.

In a study conducted by Reis (2006) using 2% of the coconut, bagasse (sugar residue) and banana fibres as addition in the PC. He concluded that coconut and bagasse fibres improve the mechanical properties of PC both its strength and its breaking energy, while banana fiber has only increased the breaking energy. In other words, coconut fibre proves to be an excellent reinforcement for PC thus increasing its ultimate bending strength by 25.1%. While, banana stem fibres is not a good choice for strengthening PC (Reis, 2006).

In another work, Reis (2012) studied the effect of the incorporation of the treated and untreated sisal fibres with NaOH and acetic acid into two polymeric mortars, based on epoxy and polyester resins respectively. The addition of a small quantity of sisal fibres contributes significantly to the improvement of the mechanical properties of the PCs. It has also been observed that polymer mortars reinforced with untreated sisal fibres have the highest ultimate strength and those reinforced with sisal fibres treatment with 10 % of NaOH have the lowest properties.

Benzannache et al. (2018) have shown in an earlier study that the incorporation of marble waste has improved the mechanical and physical properties of polymer concrete. For this purpose, the main objective is to study the effect of the additions on the mechanical strength of the PC beams subjected to 3-point bending. The PC constituted of resin, marble powder waste, sand granulate as well as 1% or 2% additions of the glass fibre in the form of bands located at tensile zone precisely at 1/3 of the height of the beam. In addition, beams with the incorporation of sisal fibre cut in two different lengths (6 mm and 12 mm) and randomly distributed in the mass have been also studied.

## **2. Experimental protocol**

### **2.1. Study Materials**

Previous work by the authors Benzannache et al. (2018) revealed that the best flexural behaviour of the PC (designed as GC60 M26) was obtained with 14% of a polyester resin matrix, 60% of a granular skeleton (sand of particle size 1-3 mm, steamed at 105°) and 26% marble powder waste of particle size (0.02-1.4 mm).

The present work consists of substituting 1 or 2% of the sand aggregate with glass fibre in the form of bands or sisal fibre cut into 6 or 12 mm lengths and randomly distributed in the mass. The characteristics of this type of glass fibre are determined in previous work, by Bouchelaghem et al. (2011), done in the laboratory of applied mechanics (LMANM of the University of 8 Mai 1945 Guelma Algeria). While, the sisal fibre are also characterised in laboratory LMANM by Belaadi et al. (2013), (2014). The seven prepared formulations are shown in Table 1.

### **2.2. Preparation**

The seven formulations of PC can be classified into three groups: the first is a polymeric concrete control (PC\_control), the second consists of four PCS formulations with 1% or 2% sisal fibre having a length of 6 or 12 mm. As for the last group contains two formulations PCG which is added 1 to 2% of glass fibre bands.

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- **Concrete PC\_control**

The constituents of the PC\_control are weighted and dry-mixed and then a polyester resin, previously prepared with 1.5% hardener and 1% accelerator, is added to the mixture. The mixture thus obtained will be placed in prismatic moulds of dimensions 25×25×300 mm according to ASTM C580.02.

**Table 1. Formulations of the PC.**

Formulation	Resin (%)	Sand (%)	Marble (%)	Sisal (%)	Glass fibre (%)	Average specimen Weight M(g)	Decrease in weight Df (%)	Density (Kg/m <sup>3</sup> )
PC_control	14	60	26	0	0	419.60	0	2237.87
PCS_1%_6mm	14	59	26	1	0	355.60	15.25	1896.53
PCS_1%_12mm	14	59	26	1	0	336.97	19.7	1797.17
PCS_2%_6mm	14	58	26	2	0	320.94	23.5	1711.68
PCS_2%_12mm	14	58	26	2	0	310.50	26.0	1656.0
PCG_1%	14	59	26	0	1	413.26	1.5	2204.05
PCG_2%	14	58	26	0	2	388.37	7.3	2076.31

- **Concrete PCS:**

For this type of concrete the sisal, having a length of 6 or 12 mm, has been incorporated and mixed well with the dry granulates. The prepared resin is then added gradually while mixing until the PCS is obtained, which is then poured into the mould may contain seven specimens (Figure 1).



**Fig 1. Mould filled with PCS may contain seven specimens.**

- **Concrete PCG:**

The same procedure for preparing the control concrete, only this time, 1 or 2% of sand is substituted by the glass fibre bands of size 25 × 300 mm impregnated with resin and spread out

at 1/3 of the height of each beam specimens, then continue until the mould is completely filled (Figure 2).



Fig 2. Filling of mould with the glass fibre band incorporations.

### 2.3. Experimental setup

The static 3-point bending tests were carried out on a Zwick/Roell Z005 universal testing machine at a test speed of 2 mm/min. This machine is equipped with a load cell whose capacity is 20 kN (Figure 3).

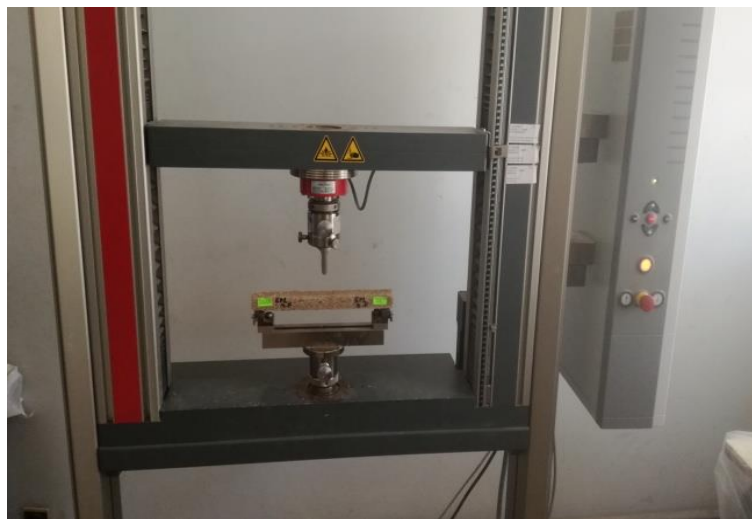


Fig 3. Experimental setup.

## 3. Results and discussion

### 3.1. Bending behaviour of PCs

The loads/displacements behaviour of the control beams presented in Fig.4 occur in two phases: the first one is practically linear until reaching the ultimate load, while sometimes small staircase shapes are noted due to the detachment of sand grains (see Figure 4, PC5\_control). The second short phase is characterized by a sudden drop of force leading to the specimen failures.

It is also important to note that at the beginning of the tests, a displacement with a small force is recorded before the actual response of the material starts. The average ultimate load is of the order of 800 N for an average displacement at failure of 0.80 mm. With the incorporation of the

sisal fibre (Figures 5a to 5d), the evolution of the load/displacement curves is similar to that obtained for the control concrete, nevertheless with more pronounced perturbations in the first phase with a less brutal drop in the load at the second phase.

The addition of the sisal fibre provoke an absorption of resin, leading to an insufficient sticking between the grains, this causes more grain detachment which results in staircase perturbations in the response curves.

The load is reduced for all the test specimens, for the incorporation of 1 or 2% of sisal fibre having lengths of 6 or 12 mm. The maximum recorded loads for the beams PCS\_1%\_6, PCS\_1%\_12, PCS\_2%\_6 and PCS\_2%\_12 are respectively 612, 583, 405 and 396 N and their corresponding to a reduction of 30 %, 37%, 97% et 102% comparatively to the control concrete. While, their displacements at failure are 1.24, 1.25, 1.22 and 3.03 mm respectively corresponding to an increase by 55%, 56%, 52% et 378% respectively. In other words, it was found that cut sisal fibre did not lead to improve the ultimate load on the contrary, it decreased it, however the displacement increased for the best case by 269% for the specimens containing 2% of sisal fibres having 12 mm length. It is important to note that the incorporation of the sisal fibres can lead to a decrease up to 26% of the specimen mass compared to the control one; this is obtained in the case of the addition of 2% of sisal having 12 mm length (see Table 1).

The load/displacement behaviour of the beams reinforced by unidirectional glass fibre bands is shown in (Figures 5e and 5f). This behaviour, which occurs in three phases, is different from that of the control beams and those reinforced by sisal fibres because it has allowed to a considerable increases both in load and displacement. The first linear phase up to a force of about 800 N corresponding to that found for the concrete control.

The second phase is also linear but with lower slope than the first phase and shows disturbances. The average ultimate loads and displacements at failure reached are 2040, 2740 N and 11.3, 10.5 mm respectively for the beams PCG\_1% and PCG\_2%. Finally, a staircase is observed leading to a less brutal failure compared to those found for the control beams and the ones reinforced with sisal fibre.

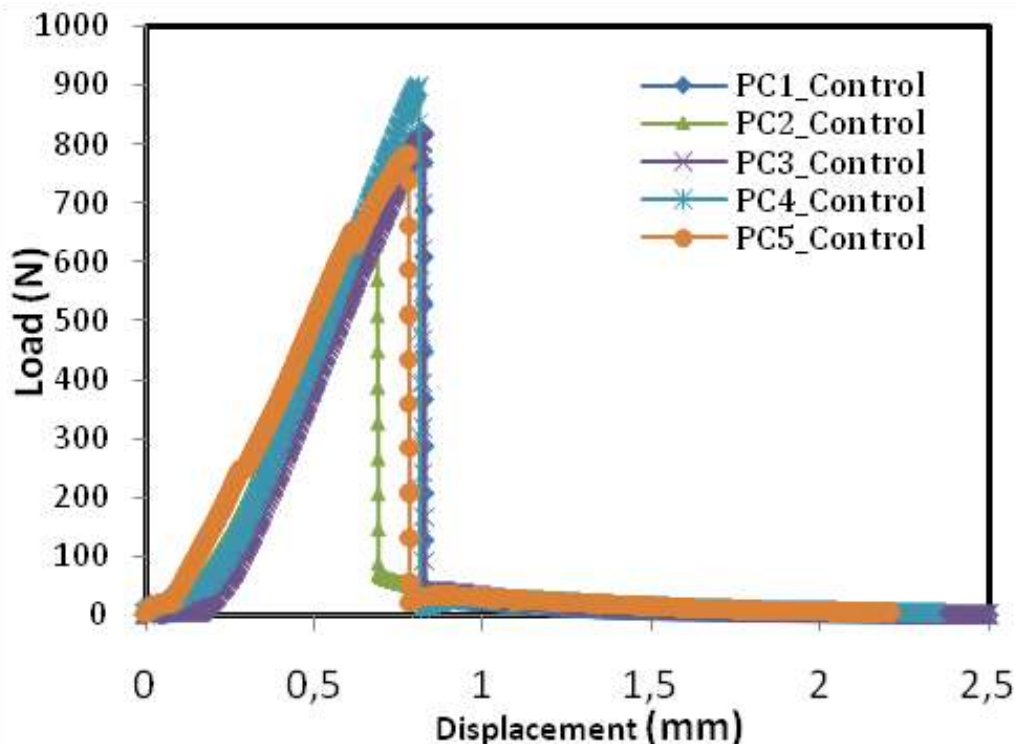


Fig 4. Load/displacement of PC-control.

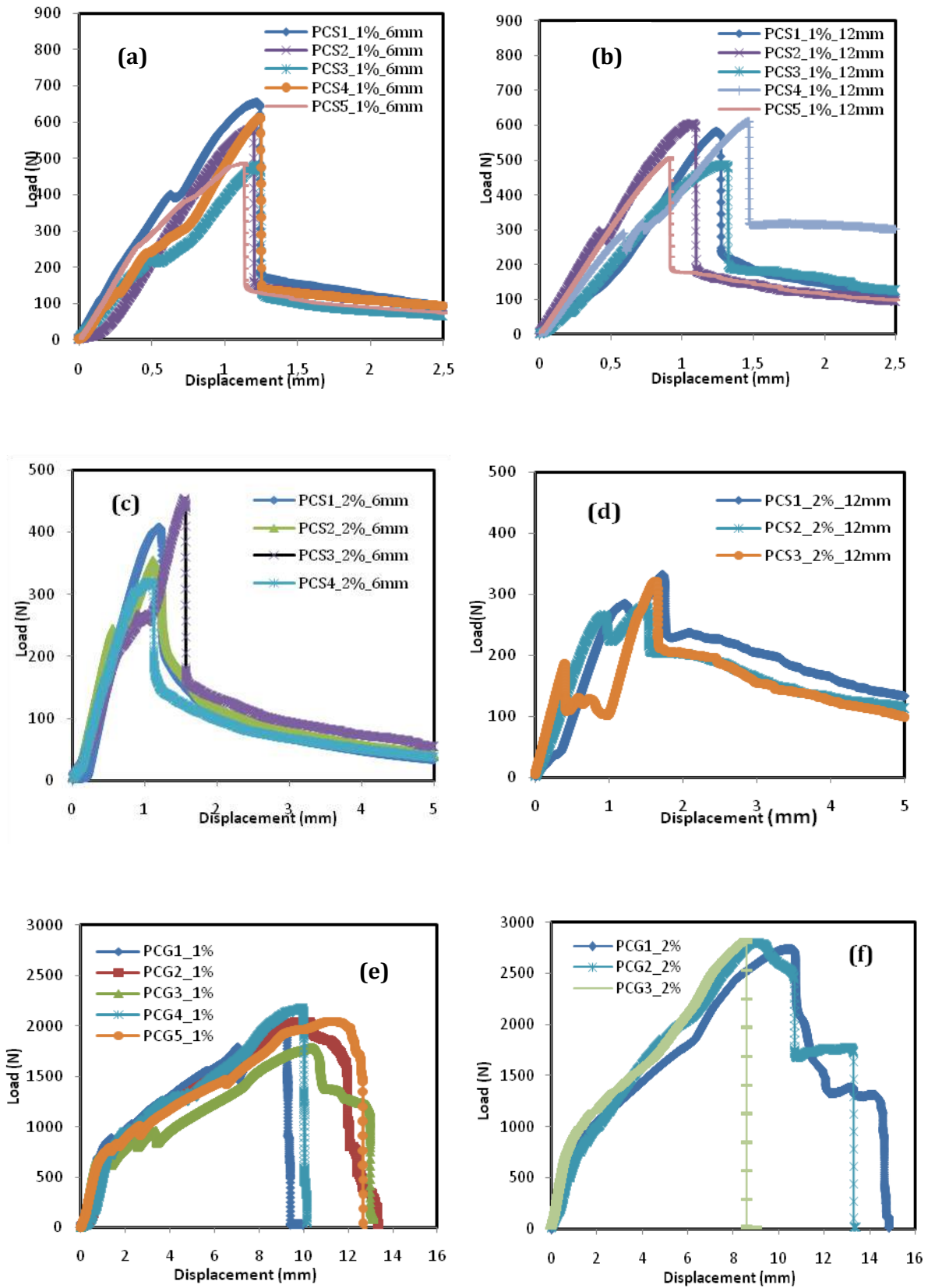


Fig 5. Load/displacement of different PCs: a) PCS\_1%\_6 mm; b) PCS\_1%\_12mm; c) PCS\_2%\_6mm; d) PCS\_2%\_12mm; e) PCG\_1% ; f) PCG\_2%.

The comparisons of the behaviour of the control PC with PCs reinforced by the sisal or glass fibres are shown in (Figure 6). The analysis of the results shows clearly that the addition of the glass fibre bands leads to the best behaviour. Indeed, the addition of 1 or 2% glass fibres allows to an average increase in ultimate load by 153 and 238% respectively and their corresponding displacements by 1281 and 1180%.

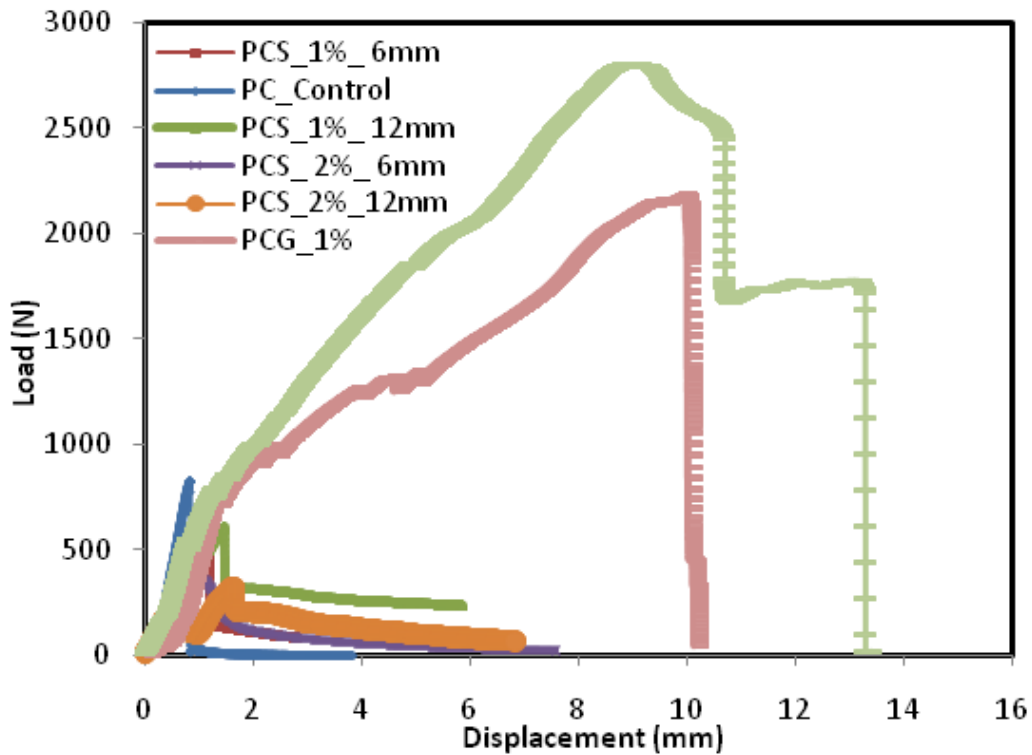


Fig 6. Load/displacement curves of the PCS and PCG compared to the control concrete (PC\_control).

### 3.2. PCs failure modes

Figure 7 shows the different failure modes of the PCs specimens where the cracks are initiated at the tensile zone (bottom surface). For the concrete control beams PC, the failure was brutal leading to divide the specimen in two parts (Figures 7a and 7b). While, for beams reinforced with sisal fibre (PCS), the failure is less brutal and the crack initiated cannot reach the face application load (Figures 7c to 7f).

Whereas, for the beams reinforced with 1% or 2% of glass fibre bands, the failure occurs on three steps following the load/displacement behaviour. The crack initiated in the tensile zone propagates until reaching the composite (step 1), this corresponds to the first phase of the curve load/displacement (Figure 7g). The composite slows the failure propagation until it broke (step 2) where a new crack appear and develop until the top surface (step 3) leading to compression failure of the concrete. The step 2 and 3 are representing the phase two and three of the PCGs Load/displacement behaviour (Figure 7h).

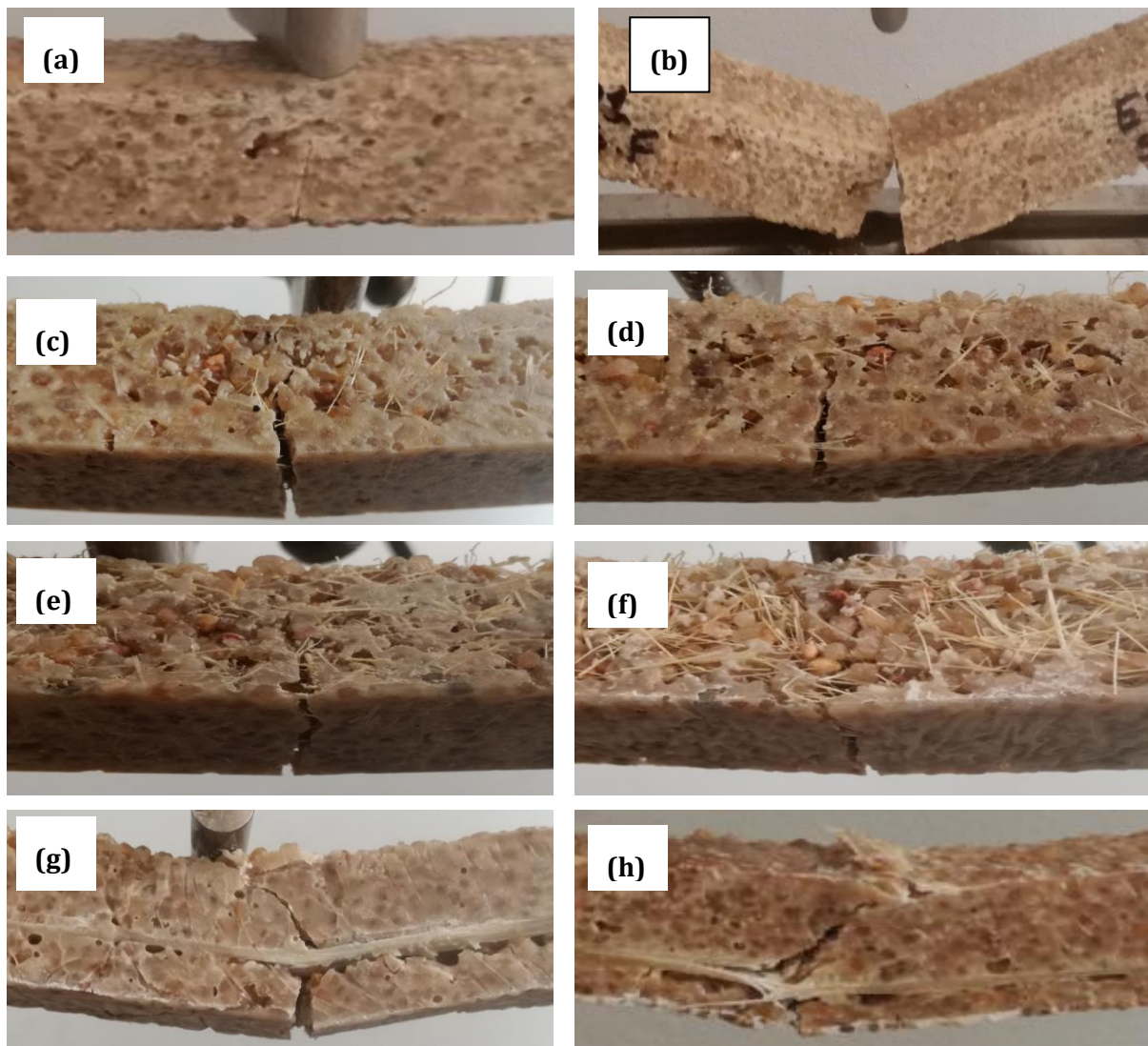


Fig 7. Rupture facies : a) Crack initiation of control beam, b) Failure control beam ; c) EPS-1%\_6 ; d) EPS-1%\_12 ; e) EPS-2%\_6 ; f) EPS-2%\_12 ; g) EPV-1% ; h) EPV-2%.

#### 4. Conclusions

The study of the flexural mechanical behaviour of polymer concrete with the incorporation of 1 or 2% of sisal or glass fibre leads to the following points:

- The addition of the sisal fibre cut and mixed randomly, allows a reduction in the mass of the specimens up to 26% compared to the control specimen. On the other hand, decreases in ultimate load with a slight increase in displacements are recorded.
- The sisal fibre causes the absorption of the resin and therefore the 14% is not sufficient to agglomerate the grains, this causes the separation of the grains which leads to the appearance of the forms in staircase (abrupt decrease of load).
- Incorporation of the unidirectional glass fibre strips introduced at 1/3 of the height of the specimens allowed the improvement of the mechanical characteristics. Indeed, the addition of 1 or 2% of glass fibre has increased the ultimate loads and displacements by 153, 238% and 1280, 1180% respectively.



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