



# Effects of Freestream Turbulence on the Pressure Acting on a Low-Rise Building Roof in the Separated Flow Region

### Pedro L. Fernández-Cabán<sup>1</sup> and Forrest J. Masters<sup>2\*</sup>

<sup>1</sup>Department of Civil and Environmental Engineering, University of Maryland, College Park, MD, United States, <sup>2</sup>Engineering School of Sustainable Infrastructure & Environment, Herbert Wertheim College of Engineering, University of Florida, Gainesville, FL, United States

This paper presents the experimental design and subsequent findings from a series of experiments in a large boundary layer wind tunnel to investigate the variation of surface pressures with increasing upwind terrain roughness on low-rise buildings. Geometrically scaled models of the Wind Engineering Research Field Laboratory experimental building were subjected to a wide range of turbulent boundary layer flows, through precise adjustment of a computer control terrain generator called the Terraformer. The study offers an in-depth examination of the effects of freestream turbulence on extreme pressures under the separation "bubble" for the case of the wind traveling perpendicular to wall surfaces, independently confirming previous findings that the spatial distribution of the peaks is heavily influenced by the mean reattachment length. Further, the study shows that the observed peak pressures collapse if data are normalized by the mean reattachment length and a non-Gaussian estimator for peak velocity pressure.

Keywords: boundary layer wind tunnel, surface pressures, terrain, roughness length, turbulence intensity, reattachment length, gust factor

# INTRODUCTION

In the context of quantifying wind loads on low-rise structures, it has been understood from the time of Jensen (1958) that the mechanical turbulence generated by upwind terrain directly influences the magnitude and spatial distribution of peak pressures of surface-mounted prisms. Numerous studies have shown that accurately simulating freestream turbulence is a necessary condition to achieving dynamic similitude in the boundary layer wind tunnel (BLWT), particularly for characterizing pressure extrema in separated flow regions (e.g., Tieleman et al., 1978; Hillier and Cherry, 1981; Gartshore, 1984; Tieleman, 1992; St Pierre et al., 2005).

Upwind terrain parameters such as the roughness length  $(z_0)$  and the displacement height  $(z_d)$  are often insufficient predictors of pressure in the so-called "bubble." The small-scale turbulence must be characterized to evaluate the roll up of the separated shear layer, the reattachment length, and the strength of the vortices advecting through this region (Melbourne, 1979; Kiya and Sasaki, 1983; Tieleman, 1993). The bubble's average extent, i.e., the mean reattachment length  $(X_R)$ , is also sensitive to the aspect ratio of the building. For example, composite analysis of multiple BLWT studies by Akon and Kopp (2016) have shown that the mean reattachment length monotonically decreases from smooth terrain to nominally open exposure conditions, plateauing as the surface roughness increases.

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> \*Correspondence: Forrest J. Masters masters@ce.ufl.edu

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These findings—along with recent work to characterize separating shear layers around bluff bodies, e.g., Lander et al. (2017)—continue to shape our understanding of the physical processes that cause extreme pressures to act on the building surface. However, given that a universal approach to predict those pressures remains elusive, there is a strong need for a more comprehensive understanding of the relationship between freestream turbulence and the mechanisms in the shear layer region that govern flow around the body. Therefore, the current study introduces into the public domain a new testbed to complement the limited experiments in this area (e.g., Fang and Sill, 1995; Saathoff and Melbourne, 1997). The study building is the Texas Tech University Wind Engineering Research Field Laboratory (WERFL; Levitan and Mehta, 1992a,b).

Boundary layer wind tunnel results from 33 unique terrains, three geometric scales (1:20, 1:30, and 1:50), and three angles of attack (parallel/perpendicular to the ridgeline and cornering) comprise the data set, which consists of nearly 300 different configurations. Here, we apply the 1:20 model data only. A significant feature of these tests is that the model is, for practical purposes, immersed in all conceivable terrains, ranging from the full inertial sublayer for the smoothest upwind case to the roughness sublayer for the most built-up conditions. The use of two element configurations in the development section also introduces a variation in the displacement height, a scarcely studied subject in wind tunnel modeling of low-rise buildings.

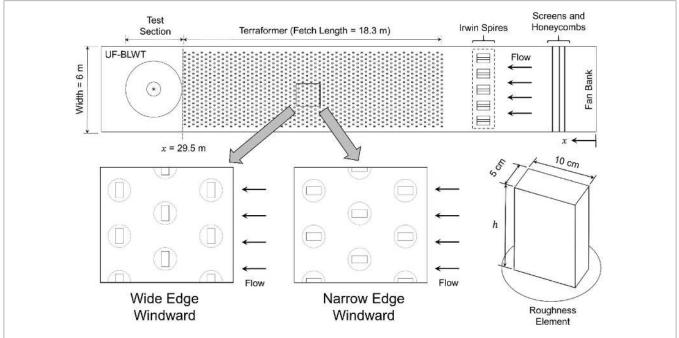
This paper presents comparative results of mean, SD, and peak surface pressures for a subset of the data, and provides new insight on how the pressure loading in separated flow regions changes as a function of higher order moments and the longitudinal turbulence intensity at the eave height of the building. The results show that the spatial distribution of the peaks is heavily influenced by  $X_R$ , confirming the functional relationship described in Akon and Kopp (2016). Further, we show that extreme suction under the separation "bubble" collapse if data are normalized by the mean reattachment length and the gust velocity pressure computed from a non-Gaussian peak factor estimator that accounts for the longitudinal turbulence intensity at eave height.

## **EXPERIMENTAL DESIGN**

Experiments were conducted at the University of Florida Natural Hazard Engineering Research Infrastructure Experimental Facility. The BLWT is a low-speed open circuit tunnel with dimensions of 6 m W  $\times$  3 m H  $\times$  38 m L (Figure 1). Simulation of terrain roughness is performed via the Terraformer, an automated roughness element grid that rapidly reconfigures the height and orientation of 1,116 roughness elements in a  $62 \times 18$  grid to achieve desired upwind terrain conditions 18.3 m along the length of the tunnel. Element dimensions are 5 cm by 10 cm, and they are spaced 30 cm apart in a staggered pattern. Height and orientation can be varied from 0-160 mm and 0-360, respectively. The approach flow was varied by changing the configuration of the Terraformer upwind of the model. Wide and narrow edge windward element orientations were applied. Roughness elements were elevated from 0-160 mm using increments of 10 mm, thus generating 16 upwind terrain conditions for each element orientation-for a total of 33 terrains including the base floor case. The maximum blockage ratio in the tunnel was less than 0.8%.

## **Approach Flow Conditions**

An automated gantry system traversed four Turbulent Flow Instrumentation Cobra pressure probes from across the tunnel for each of the 33 terrain configurations. The probes measure u,





*v*, and *w* velocity components and static pressure within a  $\pm 45^{\circ}$  acceptance cone. Response characteristics include a maximum frequency response of 2 kHz and a 2–100 m/s sensing range. Probe accuracy is  $\pm 0.5$  m/s for standard BLWT operating conditions up to turbulence intensities on the order of 30%. Three vertical traverses were taken at three lateral positions—at the centerline and  $\pm 500$  mm off the centerline of the tunnel—for each element height increment and element orientations. The triple rotation procedure described in Foken and Napo (2008) was performed to align the probe coordinate system into the streamlines and toward the mean flow coordinate system. Velocity was measured for 60 s at a sampling rate of 1,250 Hz.

**Figures 2** and **3** show streamwise mean velocity and turbulence intensity ( $I_u$ ) profiles for a narrow and wide edge windward element orientation, respectively. Mean velocities are normalized by the reference wind velocity  $U_{ref}$ , at a height of  $z_{ref} = 1,670$  mm, which was on average 15.3 m/s for all terrain configurations. The elevation was normalized by the eave height of the model (H = 198 mm). The  $I_u$  profiles show that a greater range of turbulence levels is generated by orienting the roughness elements in a wide edge windward manner. For instance,  $I_u$  exceeds 30% below z/H = 1.5 for an element height of 160 mm for wide edge (**Figure 3B**). In comparison, maximum turbulence levels for the narrow case are around 21% at elevations below

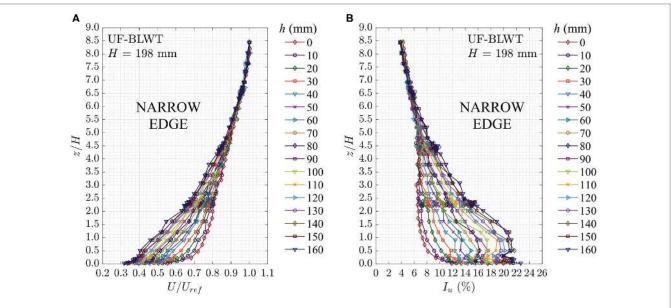
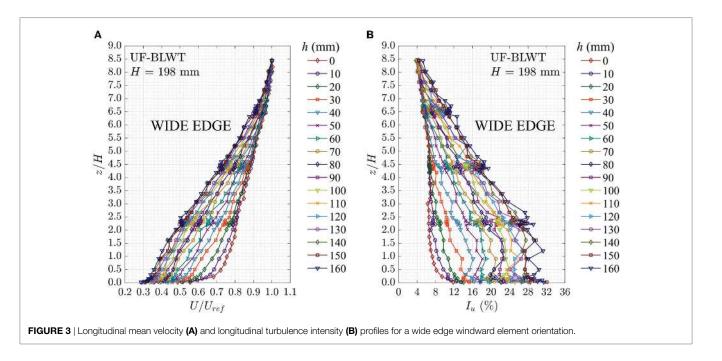


FIGURE 2 | Longitudinal mean velocity (A) and longitudinal turbulence intensity (B) profiles for a narrow edge windward element orientation.



z/H = 1.0. At the eave height of the model, the wide edge element orientation produces  $I_u = 9-30\%$ , while the narrow case generated turbulence levels ranging from 9 to 21%. Reynolds numbers at eave height ( $Re = HU_H/v$ ) ranged from  $7.8 \times 10^4$  ( $U_H = 6 \text{ m/s}$ ) for the roughest upwind case (i.e., h = 160 mm, wide) to  $14.9 \times 10^4$  ( $U_H = 11.4 \text{ m/s}$ ) for the flush element configuration.

**Table 1** summarizes estimated aerodynamic and turbulence parameters values at eave height for the full range of element heights and orientations. Turbulence parameters were obtained from probe measurements taken at the eave height of model (z = H). Equivalent full-scale aerodynamic roughness length estimates were obtained from the logarithmic velocity profile:

$$z_0 = (z - z_d) \exp\left[-\frac{U_z \kappa}{u_*}\right] \tag{1}$$

where  $u_*$  is the shear (friction) velocity and  $\kappa$  is the von Kármán constant (~0.4). The zero-plane displacement height was taken as 13.1% of the roughness element height *h*, following the procedure in Macdonald et al. (1998) that was applied in Fernández-Cabán and Masters (2017). Shear velocities were generated by

directly calculating the Reynolds stress from the fluctuating u and w velocity components—i.e.,  $u_*^2 = \overline{u'w'}$ . The ratio of SD to shear velocity ( $\sigma_u/u_*$ ) was fairly constant in the inertial sublayer, with values ranging from 2.0 to 2.1 for all roughness element configurations. These ratios are marginally lower than  $\sigma_u/u_* = 2.5$  described in ASCE/SEI 49-12 (2012). This difference in  $\sigma_u/u_*$  might be attributed to the lack of large-scale "inactive motion" in the boundary layer (Raupach et al., 1986). Nevertheless, the structure of the separation bubble is mostly dependent on the small-scale turbulent characteristics of the freestream near separated flow regions (Hillier and Cherry, 1981).

Longitudinal turbulence spectra at eave height (z=H) for six representative roughness configurations are shown in **Figure 4**. The spectra are presented in the dimensionless form  $nS_{uu}(n,H)/U^2$  and nH/U after Irwin (1998) and Richards et al. (2007). The Von Kármán (1948) spectrum found in ESDU 83045 (1983) was fitted to the measured spectra using equivalent fullscale  $z_0$  values from **Table 1**, and the measured integral length scales  $L_u^x$ —obtained from integration of the autocorrelation function. In general, measured  $L_u^x$  are at least three times the eave height of the model which is in accordance with minimum requirements in ASCE/SEI 49-12 (2012).

**TABLE 1** | Approach flow parameters for the wide and narrow edge windward element orientations.

h (mm)	I <sub>u,H</sub> (%)		z <sub>0</sub> (cm) (full scale)		H/z <sub>o</sub>		$L_u^x/H$	
	Wide	Narrow	Wide	Narrow	Wide	Narrow	Wide	Narrow
0		7.9	C	).038	10	),446	4	1.7
10	9.7	9.0	0.16	0.088	2,524	4,481	4.8	4.6
20	11.8	9.9	0.68	0.21	585	1,863	4.3	4.0
30	13.9	11.2	1.69	0.47	234	844	3.5	3.3
40	16.2	12.5	3.65	0.90	108	439	5.0	3.2
50	18.1	13.7	6.34	1.49	62	266	4.1	3.7
60	19.5	14.7	9.07	2.12	44	187	3.3	3.2
70	20.9	15.1	12.9	2.64	31	150	4.1	3.3
80	22.1	15.8	17.0	2.99	23	132	3.1	3.2
90	23.2	16.7	18.3	4.24	22	94	3.6	2.9
100	24.5	17.0	20.9	4.81	19	82	3.8	2.7
110	25.2	18.3	23.6	6.31	17	63	3.5	2.2
120	26.3	18.5	29.2	6.97	14	57	2.6	4.4
130	26.2	19.2	29.4	7.22	13	55	3.9	3.2
140	28.0	20.2	33.5	8.74	12	45	2.4	3.2
150	30.4	19.9	36.2	9.48	11	42	3.1	2.2
160	29.6	21.1	37.6	11.4	11	35	2.6	3.0

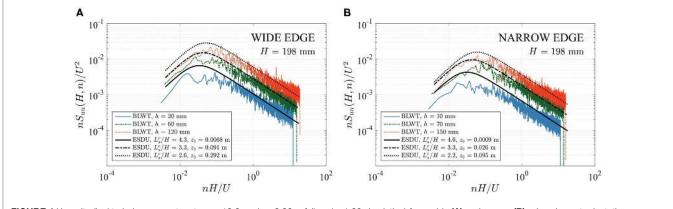


FIGURE 4 | Longitudinal turbulence spectra at a z = 19.8 cm (z = 3.96 m full scale, 1:20 simulation) for a wide (A) and narrow (B) edge element orientations.

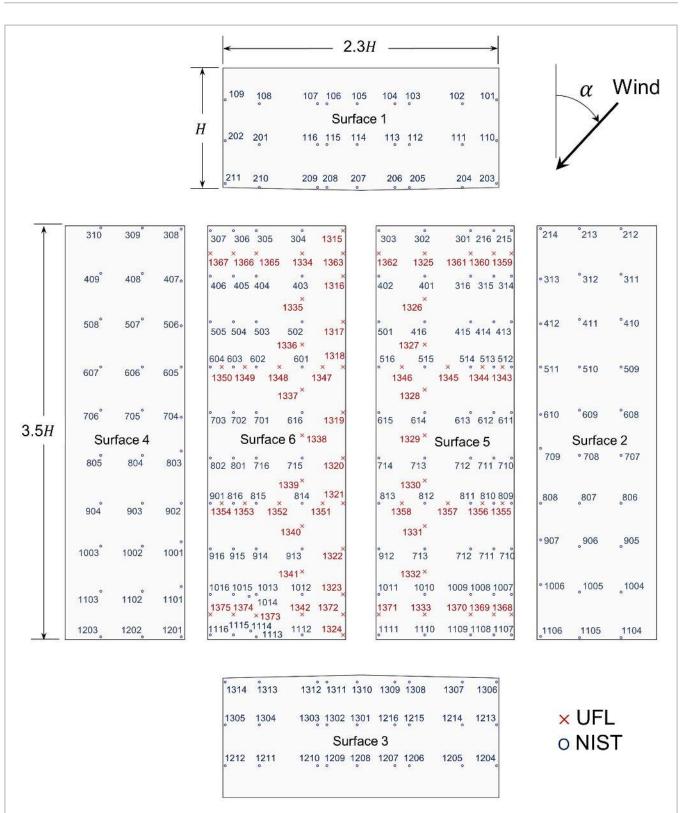


FIGURE 5 | Tap layout for the 1:20 Wind Engineering Research Field Laboratory model. Tap location is identical to building model "st3" in the NIST aerodynamic database (Ho et al., 2003) with the exception of the additional taps indicated by the "X" marker.

# **Building Model Surface Pressure** Measurements

Pressure measurements presented in this study were conducted on the 1:20 rigid building model of the WERFL building. The model was instrumented with 266 pressure taps on the four walls and roof. The tap location follows the same layout as building model "st3" in the NIST aerodynamic database (Ho et al., 2003; see **Figure 5**), however, 60 additional pressure taps were added on the roof of the model to improve the spatial resolution along tap lines parallel to the long and short building dimension. The model was aligned at 0 and 90 in the approach flow (**Figure 6**).

Time series of differential pressures were measured using eight high-speed electronic pressure scanning modules (ZOC33, 2016) from Scanivalve. Urethane tubing cut to a length of 122 cm connected the taps to the pressure scanner. Resonance and damping effects in the tubing system (Irwin et al., 1979) were digitally filtered out using tubing system transfer functions following the approach described in Pemberton (2010). Each experiment was conducted for 300 s at a sampling rate of 625 Hz. Pressure data presented in this work were low-pass filtered 300 Hz to ensure inclusion of significant flow characteristics in the separation bubble—e.g., pseudo-periodic shedding of vortices (Saathoff and Melbourne, 1997). However, cutoff frequencies of 150 and 300 Hz were applied to the complete dataset. Peak pressures that were low-pass filtered at 300 Hz were 5–7% higher than the 150 Hz cutoff frequency.

Unless noted otherwise, external pressure coefficients shown in this paper are calculated as the ratio of the differential pressure and the mean velocity (dynamic) pressure at model eave height:

$$C_{p}\left(t\right) = \frac{P\left(t\right) - P_{0}}{\bar{q}_{H}}$$
(2)

where P(t) is the (absolute) pressure measured,  $P_0$  is the reference (static) pressure,  $\bar{q}_H = 1/2\rho U_H^2$ ,  $\rho$  is the air density, and  $U_H$ is the mean streamwise velocity at eave height estimated from the mean reference velocity pressure in the freestream ( $U_{\text{ref}}$ ). The reference pressure was converted to the eave height of the building model using an empirical adjustment factor (K) obtained when the model was removed from the turntable:

$$U_H = K U_{\rm ref} \tag{3}$$

Static reference pressures ( $P_0$ ) were taken from the static port of the Pitot tube, ensuring stable measurements with negligible fluctuations. Air density ( $\rho$ ) was calculated from the air temperature, barometric pressure, and relative humidity measured during each test.

## RESULTS

## **Spatial Distribution of Surface Pressures**

Contour subplots of mean pressures ( $C_{p,\text{mean}}$ ) measured on the model with the flow streamwise parallel to the long building dimension ( $\alpha = 0^{\circ}$ ), are shown in **Figure 7**. The effect of the freestream turbulence is evident in all cases. Flow over smoother terrains (e.g., h = 10 mm) cause larger reattachment lengths at the leading edge of the structure. As the longitudinal turbulence

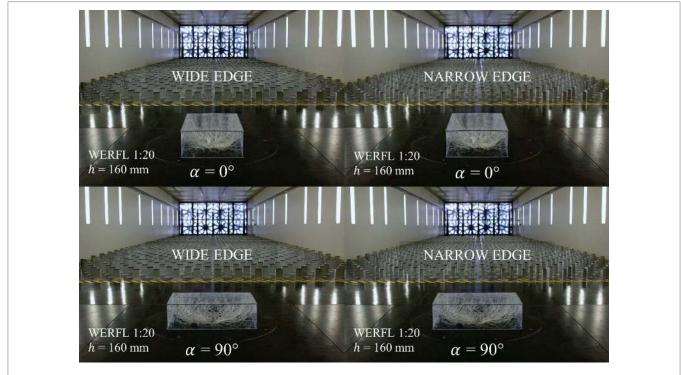


FIGURE 6 | Boundary layer wind tunnel modeling of the 1:20 Wind Engineering Research Field Laboratory (WERFL) building for the narrow and wide roughness element arrangements and the two building orientations considered in this study.

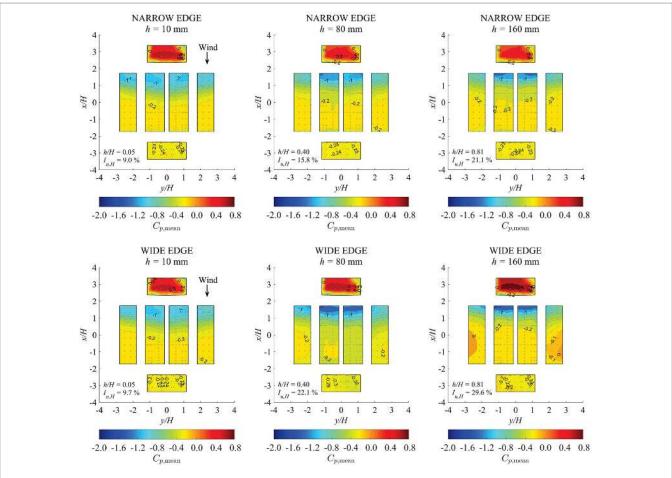


FIGURE 7 | Contours of mean pressure coefficients referenced to the mean velocity pressure at the eave height of the 1:20 model of the Wind Engineering Research Field Laboratory Building.

intensity increases, the mixing in the shear layers increases the rate of entrainment, which decreases the shear layer's radius of curvature (Gartshore, 1973).

Standard deviation contour maps of pressure coefficients  $(C_{p,std})$  are displayed in **Figure 8**. Differences in the spatial distribution of  $C_{p,std}$  are apparent when comparing the three turbulence levels presented. The largest  $C_{p,std}$  occur near the two corners of the leading edge of the roof for all terrain configurations. However, along the centerline of the roof (y/H=0), the maximum pressure fluctuations develop closer to the leading edge as the streamwise turbulence increases. For instance, for the roughest case of h = 160 mm (wide edge),  $C_{p,std} \sim 0.8$  close to the leading edge, then fluctuations of pressure rapidly decay and stabilize to  $\sim 0.3$  at approximately x/H = 0.5. In contrast, for h = 10 mm (wide edge), smaller peak values ( $\sim 0.25$  at  $x/H \sim 1.0$ ) and a more gradual decay of  $C_{p,std}$  is observed (i.e., more spacing between contour lines) along the roof's centerline. This systematic trend also prevails for the narrow element orientation.

## **Transects of Pressure Along the Roof**

The following results describe the surface pressures along designated tap lines shown in **Figure 9**, which depicts two pairs of densely tapped line transects oriented parallel and perpendicular to the long building dimension, i.e., tap line 1 (TL1), TL2, TL3, and TL4. Tap line pairs (e.g., TL1 and TL2) are symmetric with respect to the centerline of the model.

### Statistical Measures of the Surface Pressure

Figure 10 displays the distribution of statistical properties along TL1 for 16 terrains (i.e., turbulence levels). The distance from the leading edge is normalized by the eave height of the model (H). The mean pressure is observed to vary by no more than 30% at the leading edge of the structure, with the more turbulent flow causing larger suction. Farther along the roof (x/H > 0.4), this phenomenon reverses as the more turbulent flow reattaches to the roof. The trend in the SD is pronounced, exhibiting nearly a factor of three difference at the leading edge that is on the order of the ratio of the corresponding longitudinal turbulence intensities. Further, the data generally observe the first-order quasi-steady relationship study described in Uematsu and Isyumov (1998), i.e.,  $C_{p,\text{std}} = 2I_u |C_{p,\text{mean}}|$ , which indicates the variations in the approach flow dominate the distortion of the wind around the building with regard to the time varying loads on the structure.

The higher order moments (e.g., skewness and kurtosis) exhibit non-Gaussian trends observed near flow separation regions

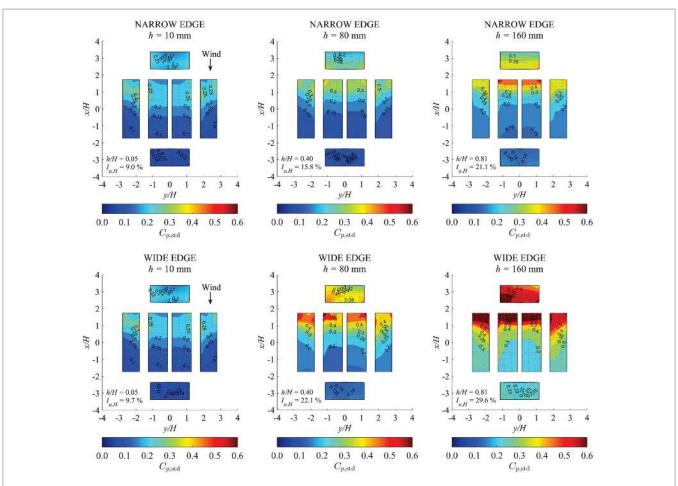
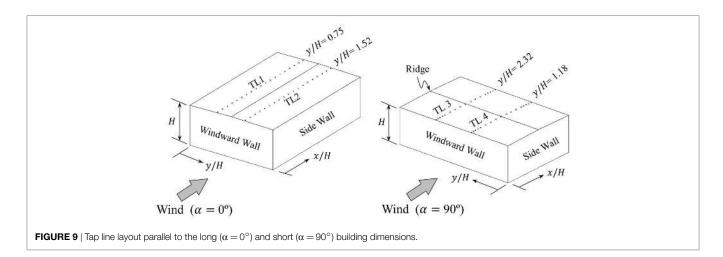


FIGURE 8 | Contours of the pressure coefficient SD referenced to the mean velocity pressure at the eave height of the 1:20 model of the Wind Engineering Research Field Laboratory Building.



(Holmes, 1981; Sadek and Simiu, 2002). The magnitude of skewness values at the leading edge decreases from -0.8 to -1.4 with increasing turbulence. Kurtosis values rise from ~5 to ~8 close to x/H = 0. Furthermore, in smoother freestream flows (e.g.,  $I_{u,H} = 9.7\%$ ), near Gaussian behavior appears to occur from x/H = 1.0 to 1.5. The roughest upwind cases show close to zero

# skewness values at x/H > 2.0. However, no clear trend in kurtosis is present in this region.

## **Reduced Mean Pressure Coefficients**

Akon and Kopp (2016) presented a systematic investigation of surface pressures occurring on surface mounted, three-dimensional

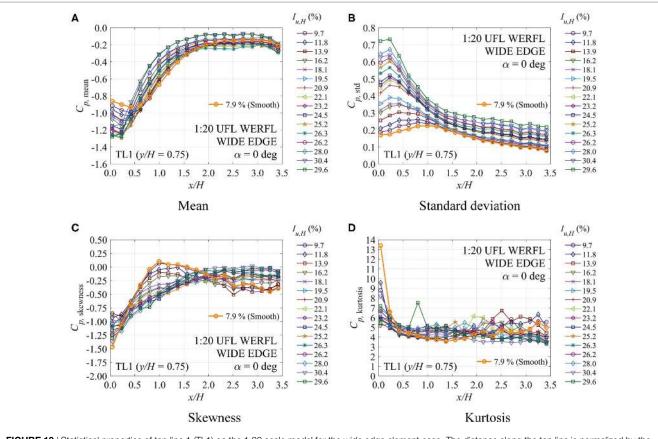


FIGURE 10 | Statistical properties of tap line 1 (TL1) on the 1:20 scale model for the wide edge element case. The distance along the tap line is normalized by the eave height of the model. (A) Mean, (B) SD, (C) skewness, and (D) kurtosis.

 $\ensuremath{\mathsf{TABLE 2}}\xspace | \ensuremath{\mathsf{Reduced}}\xspace pressure coefficient and mean reattachment length estimates (wide edge).$ 

 
 TABLE 3 | Reduced pressure coefficient and mean reattachment length estimates (narrow edge).

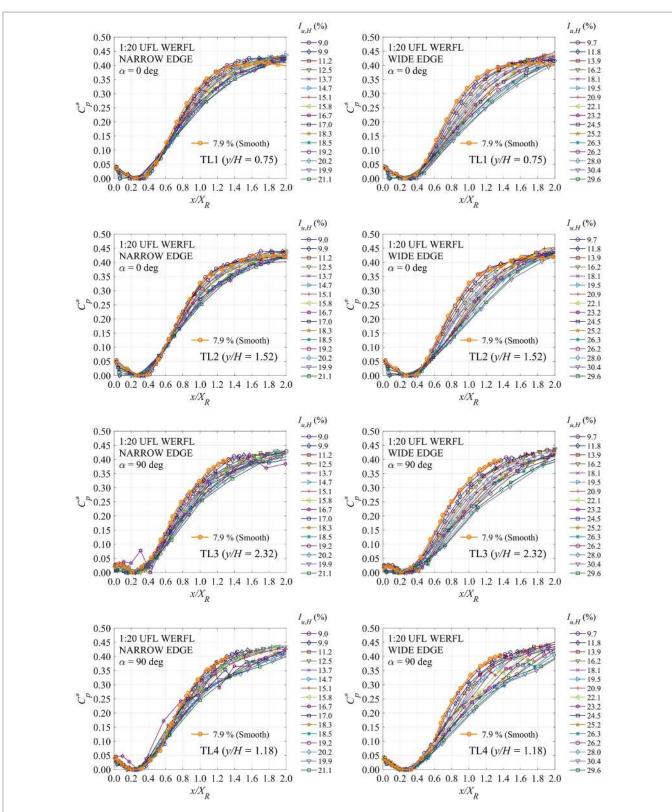
h (mm)	I <sub>u,H</sub> (%)	$C_{p}^{*}$	X <sub>R</sub> /	h (mm	
			TL1, TL2	TL3, TL4	
0	7.9	0.33	1.59	1.68	10
10	9.7	0.32	1.45	1.54	20
20	11.8	0.30	1.29	1.34	30
30	13.9	0.29	1.16	1.19	40
40	16.2	0.28	1.04	1.09	50
50	18.1	0.26	0.96	1.03	60
60	19.5	0.26	0.90	0.98	70
70	20.9	0.25	0.83	0.90	80
80	22.1	0.24	0.78	0.84	90
90	23.2	0.23	0.75	0.79	100
100	24.5	0.23	0.71	0.72	110
110	25.2	0.22	0.65	0.67	120
120	26.3	0.21	0.66	0.67	130
130	26.2	0.21	0.60	0.60	140
140	28.0	0.20	0.56	0.57	150
150	30.4	0.19	0.52	0.53	160
160	29.6	0.19	0.48	0.51	

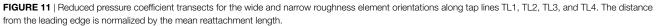
h (mm)	I <sub>u,H</sub> (%)	$C_{p}^{*}$	X <sub>R</sub> /H			
			TL1, TL2	TL3, TL4		
10	9.0	0.32	1.48	1.60		
20	9.9	0.31	1.38	1.47		
30	11.2	0.31	1.27	1.32		
40	12.5	0.30	1.16	1.21		
50	13.7	0.29	1.07	1.10		
60	14.7	0.28	0.97	1.03		
70	15.1	0.28	0.93	1.00		
80	15.8	0.28	0.87	0.91		
90	16.7	0.27	0.84	0.96		
100	17.0	0.27	0.80	0.87		
110	18.3	0.26	0.72	0.72		
120	18.5	0.26	0.70	0.70		
130	19.2	0.26	0.67	0.67		
140	20.2	0.25	0.62	0.64		
150	19.9	0.25	0.60	0.61		
160	21.1	0.25	0.57	0.56		

bluff bodies, finding that the reduced mean pressure coefficient  $C_p^*$  defined in Ruderich and Fernholz (1986):

$$C_{p}^{*} = \frac{C_{p} - C_{p,\min}}{1 - C_{p,\min}}$$
(4)

produces a suitable match for a broad range geometric scales, building aspect ratios, and terrain conditions. Here  $C_p$  refers to the mean pressure coefficient at the location of interest and  $C_{p,\min}$  refers to the lowest mean pressure coefficient observed in the tap line under the separation bubble. The application of





the Terraformer system enabled characterization of the variation in surface pressure at much finer resolution than previous studies. Figure 11 presents the results for 16 terrains resulting from increasing the element height from 10-160 mm and rotating the elements in the wide (left column) and narrow (right column) orientation to the oncoming flow. In the absence of particle image velocimetry measurements to quantify the distance from the leading edge to the reattachment point  $(X_R)$ , the empirical relationship developed by Akon and Kopp (2016) (Figure 8) was applied to estimate the value based on the longitudinal turbulence intensity  $(I_{u,H})$ . Tables 2 and 3 list values for the wide and narrow edge element orientations. Tables 2 and 3 show slightly smaller mean reattachment lengths when compared with  $X_R$  values presented in Akon and Kopp (2016). The lower  $X_R$  values might be a result of the lateral location (y/H) selected for the line transects. In Akon and Kopp (2016), the pressure taps were located along the centerline of the roof surface. Larger mean reattachment lengths should be expected along the roof's centerline. Conversely, the four tap lines considered in this study were offset from the centerline (**Figure 9**) of the roof. Therefore, a slight reduction in  $X_R$ should be anticipated.

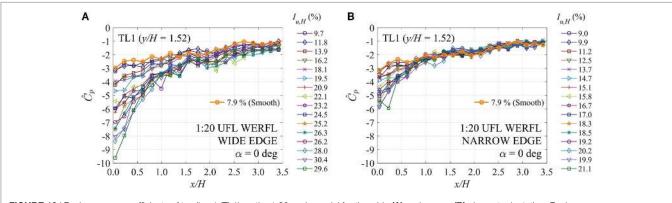
A clear pattern appears in all four tap lines. The minimum  $C_p^*$ , which corresponds to the worst suction, occurs at  $x/X_R = 0.3$  for every case. In less turbulent flows (i.e., nominally  $I_{u,H} < 20\%$ ), a reduction in suction at the leading edge is observed (i.e.,  $C_p^*$  increases). Above  $I_{u,H} = 20\%$  the worst-case suction shifts from  $x/X_R = 0.3$  to  $x/X_R = 0$ , indicating a larger suction occurs. Beyond  $x/X_R = 0.3$ , the reduced pressure coefficients exhibit a consistent trend. The rougher terrains produce lower  $C_p^*$  in the field of the roof than that of the open exposure, which indicates that the freestream turbulence has a clear effect on the radius of curvature of the shear layer.

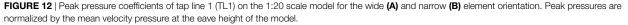
#### **Peak Pressure Coefficients**

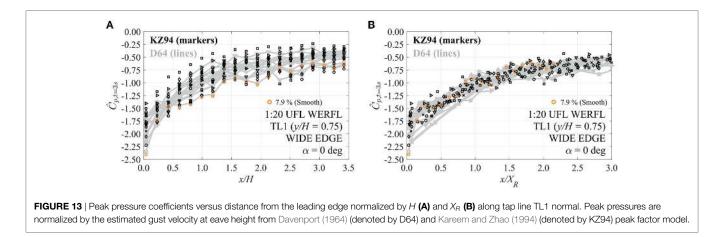
Extreme value analysis was applied to estimate peak surface pressures (Lieblein, 1974) along roof line transects. **Figure 12** shows peak pressure coefficients measured along TL1 for wide (**Figure 12A**) and narrow (**Figure 12B**) element orientations. Values are computed from a Fisher–Tippett Type I (Gumbel) distribution for a 78% probability of non-exceedance (Cook and Mayne, 1979), and normalized by the mean velocity pressure at eave height as follows:

$$\hat{C}_{p} = \frac{\hat{P}_{78} - P_{0}}{\bar{q}_{H}}$$
(5)

where  $\hat{P}_{78}$  is the peak suction of the time history. Figure 12 shows a large spread in peak pressures at the roof's leading edge with a systematic trend of increasing  $\hat{C}_p$  with freestream turbulence. Turbulence levels for both element orientations show







#### TABLE 4 | Gust factor estimates based on turbulence levels at eave height (wide edge).

<i>h</i> (mm)	I <sub>и , Н</sub> (%)	Skewness, $\gamma_{3u}$	Kurtosis, γ <sub>4u</sub>	Peak factor	r, g (t = 3 s, T = 3,600 s)	Gust factor, GF	
				D64 <sup>a</sup>	KZ94 <sup>b</sup>	D64 <sup>a</sup>	KZ94 <sup>b</sup>
0	7.9	-0.056	2.91	3.47	3.25	1.27	1.26
10	9.7	-0.14	2.73	3.47	2.84	1.34	1.28
20	11.8	0.03	2.66	3.47	2.98	1.41	1.35
30	13.9	0.03	2.70	3.47	3.04	1.48	1.42
40	16.2	0.08	2.62	3.47	3.00	1.56	1.49
50	18.1	0.12	2.63	3.47	3.06	1.63	1.55
60	19.5	0.27	2.67	3.47	3.38	1.68	1.66
70	20.9	0.26	2.87	3.47	3.67	1.73	1.77
80	22.1	0.39	3.02	3.47	4.05	1.77	1.90
90	23.2	0.34	2.90	3.47	3.82	1.80	1.89
100	24.5	0.25	2.85	3.47	3.63	1.85	1.89
110	25.2	0.33	2.91	3.47	3.83	1.87	1.96
120	26.3	0.43	3.11	3.47	4.20	1.91	2.10
130	26.2	0.37	2.96	3.47	3.94	1.91	2.03
140	28.0	0.29	3.11	3.47	4.01	1.97	2.12
150	30.4	0.47	3.39	3.47	4.54	2.05	2.38
160	29.6	0.27	2.99	3.47	3.84	2.03	2.14

<sup>a</sup>Gaussian peak factor model from Davenport (1964).

<sup>b</sup>Non-Gaussian peak factor model from Kareem and Zhao (1994).

TABLE 5 | Gust factor estimates based on turbulence levels at eave height (narrow edge).

h (mm)	I <sub>и,Н</sub> (%)	Skewness, $\gamma_{3u}$	Kurtosis, γ <sub>4u</sub>	Peak facto	r, g (t = 3 s, T = 3,600 s)	Gust factor, GF	
				D64 <sup>a</sup>	KZ94 <sup>b</sup>	D64 <sup>a</sup>	KZ94 <sup>b</sup>
10	9.0	-0.10	2.70	3.47	2.84	1.31	1.25
20	9.9	-0.04	2.82	3.47	3.16	1.34	1.31
30	11.2	0.03	2.75	3.47	3.14	1.39	1.35
40	12.5	0.00	2.64	3.47	2.89	1.43	1.36
50	13.7	0.14	2.75	3.47	3.30	1.48	1.45
60	14.7	0.07	2.71	3.47	3.13	1.51	1.46
70	15.1	0.12	2.70	3.47	3.20	1.52	1.48
80	15.8	0.04	2.73	3.47	3.12	1.55	1.49
90	16.7	0.13	2.66	3.47	3.14	1.58	1.52
100	17.0	0.13	2.70	3.47	3.20	1.59	1.54
110	18.3	0.15	2.73	3.47	3.30	1.63	1.60
120	18.5	0.16	2.79	3.47	3.42	1.64	1.63
130	19.2	0.22	2.87	3.47	3.61	1.66	1.69
140	20.2	0.16	2.79	3.47	3.40	1.70	1.69
150	19.9	0.11	2.74	3.47	3.24	1.69	1.64
160	21.1	0.27	2.94	3.47	3.78	1.73	1.80

<sup>a</sup>Gaussian peak factor model from Davenport (1964).

<sup>b</sup>Non-Gaussian peak factor model from Kareem and Zhao (1994).

an increase in the magnitude of  $\hat{C}_p$  from approximately -2.5 to -5 for  $I_{u,H} = 9.7-21\%$  close to x/H = 0. The higher turbulence intensities produced by the wide edge element orientation results in peak pressures raging from -5 to -9 for  $I_{u,H} = 21-29\%$ . Peak pressure curves for both element orientations appear to converge as they progress toward the trailing edge of the roof (x/H = 3.5) where all the peak pressures are approximately unity.

### Normalization of Peak Pressure Coefficient Transects

In contrast to the previous section that examined the spatial distribution of pressure minima along the length of the roof, this section evaluates the effect of normalizing the data by the mean reattachment length and the equivalent 3 s gust velocity pressures at full scale, factoring in non-Gaussian behavior. This non-Gaussian trend of the streamwise component has been observed

in field experiments and wind tunnel tests (Fernández-Cabán and Masters, 2017). Here, the pressure minima are calculated as:

$$\hat{C}_{p,t=3\ s} = \frac{\hat{P}_{78} - P_0}{\hat{q}_{H,t=3\ s}} \tag{6}$$

In Eq. 6,  $\hat{q}_{H,t=3 s} = \bar{q}_H (\text{GF})^2$  where GF is the 3 s gust factor computed from the following equation:

$$GF = 1 + gI_{u,H} \tag{7}$$

where *g* is the 3 s peak factor calculated using a moment-based model presented in Kareem and Zhao (1994) that accounts for the crossing rate, skewness, and kurtosis of streamwise velocity:

$$g = \alpha \left\{ \begin{cases} \left(\beta + \frac{\gamma}{\beta}\right) + h_3 \left(\beta^2 + 2\gamma - 1\right) + \cdots \\ h_4 \left[\beta^3 + 3\beta \left(\gamma - 1\right) + \frac{3}{\beta} \left(\frac{\pi^2}{12} - \gamma + \frac{\gamma^2}{2}\right) \right] \right\}$$
(8)

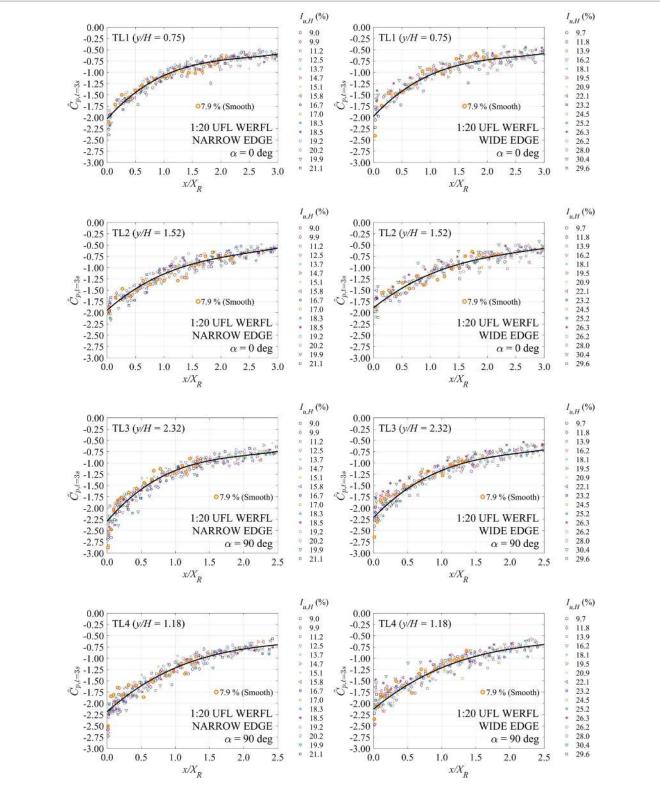




TABLE 6 | Polynomial coefficients from third-order curve fitting of peak pressures as a function of the normalized mean reattachment length.

Aspect ratio (W/H)	Tap line	Element orientation	Cubic polynomial coefficients				R <sup>2</sup>	RMSE
			a <sub>1</sub>	a <sub>2</sub>	a <sub>3</sub>	a4		
2.3	TL1	Narrow	0.07	-0.52	1.41	-2.03	0.95	0.10
		Wide	0.07	-0.49	1.34	-1.98	0.94	0.10
	TL2	Narrow	0.05	-0.36	1.12	-1.95	0.92	0.12
		Wide	0.03	-0.29	1.00	-1.89	0.91	0.12
		Mean	0.05	-0.42	1.22	-1.96	0.93	0.11
		SD	0.02	0.11	0.19	0.06	0.02	0.01
3.5	TL3	Narrow	0.12	-0.76	1.76	-2.29	0.95	0.12
		Wide	0.09	-0.62	1.58	-2.21	0.95	0.12
	TL4	Narrow	0.05	-0.43	1.36	-2.19	0.95	0.12
		Wide	0.04	-0.38	1.25	-2.13	0.93	0.13
		Mean	0.08	-0.55	1.49	-2.20	0.94	0.12
		SD	0.04	0.18	0.23	0.07	0.01	0.004

where  $\gamma$  is Euler's constant, 0.5772,  $\beta = \sqrt{2 \ln (\nu T)}$ ,  $\nu$  is the crossing rate, and *T* is the duration of the record. The parameters  $\alpha$ ,  $h_3$ , and  $h_4$  are dependent on the third (skewness) and fourth (kurtosis) moments (see Balderrama et al., 2012). Equation 8 is reduced to the well-known peak factor model from Davenport (1964) when the skewness and kurtosis values are set to zero and three, respectively—i.e., Gaussian.

**Figure 13** presents peak pressure coefficients for line transect TL1. The distance from the leading edge are normalized by eave height (x/H, left pane) and mean reattachment length ( $x/X_R$ , right pane), respectively. It is observed that normalizing the abscissae by  $X_R$  shifts the data into horizontal alignment, while normalizing the data by the non-Gaussian velocity pressure causes the data to collapse vertically.

**Tables 4** and 5 contain peak and gust factors at eave height (3.96 m at full scale) for the wide and narrow element orientations, respectively. Peak factors from Davenport (1964) were computed using Eq. 8 and setting the skewness and kurtosis values to zero and three, respectively. This results in g = 3.47 for all element heights and orientations. Thus, it is implied that the peak factor is terrain independent for the Gaussian. In contrast, the non-Gaussian model varies the peak factor from 2.84 to 4.54, which produces a better fit to the data by accounting for increased skewness results in higher peak factors in rougher terrains ( $I_{u,H} > 25\%$ ) and lower peak factors for smoother exposures ( $I_{u,H} \sim 10\%$ ) when compared with the Gaussian model (Davenport, 1964).

**Figure 14** expands the results of **Figure 13** to include all tap lines for the narrow and wide edge element orientations. Data fitted to third-degree (i.e., cubic) polynomial curves using a robust linear least-squares fitting method to develop an empirical relation of peak pressures and mean reattachment lengths:

$$\hat{C}_{p,t=3\,s}\left(k=x/X_R\right) = a_1k^3 + a_2k^2 + a_3k + a_4 \tag{9}$$

where  $a_1$ ,  $a_2$ ,  $a_3$ , and  $a_4$  are four polynomial coefficients found from the fit. **Table 6** includes the corresponding polynomial coefficients, correlation, and root-mean square errors. The results show that the data reasonably collapse for all cases. Little variability is seen in the coefficients and the value at the leading edge (i.e.,  $a_4$ ) are nearly constant for all cases associated with a given wind direction. The effect of the building aspect ratio is evident, however. TL3 and TL4 (associated the with wide side of the building facing windward) produce larger coefficients than TL1 and TL2 due to higher peak pressures near the leading edge caused by the larger flow distortion of the building (see **Figure 14**).

#### Normalization of Fluctuating Pressure Coefficients

Analysis was also performed to investigate the effect of such normalization scheme on the distribution of fluctuating roof pressures  $(C_p)$ . Figure 15 includes subplots of normalized SD coefficient  $(C'_{p,t=3 s})$  transects for the narrow and wide roughness element orientations. Similar to Eq. 6, fluctuating pressures are normalized by the 3-s gust velocity pressure  $(\hat{q}_{H,t=3 s})$  at eave height using the non-Gaussian peak factor model (Kareem and Zhao, 1994). Further, the distance from the leading edge is normalized by  $X_R$  (contrary to Figure 10B). The family of  $C_{p,t=3s}$ transects in Figure 15 appear to collapse when compared with fluctuating pressure distributions shown in Figure 10B. However, the location of maximum  $C'_{p,t=3 s}$  along line transects varies slightly with freestream turbulence. For smoother terrain, the peak  $C_{p,t=3 \text{ s}}$  occurs at ~0.6  $X_R$  from the leading edge. Maximum pressure fluctuations for the rougher upstream cases are observed at distances of ~0.35  $X_R$  from the leading edge. These spatial variations of peak  $C_{p,t=3 s}^{'}$  with upstream conditions suggests that normalizing by the non-Gaussian gust velocity pressure might not be sufficient to fully characterize the distribution of pressure fluctuations in the separation bubble, which suggests that the spatial distribution of pressure fluctuations is mainly controlled by the interaction of the approach flow with the structure of the separation bubble and less so by the freestream flow conditions.

## DISCUSSION

The normalization procedure presented in Section "Normalization of Peak Pressure Coefficient Transects" appears to successfully collapse peak pressures under the separation bubble for a wide range of freestream turbulence levels, with some variation observed at the lead edge of the roof. This collapse of pressure extrema appears to be, in part, attributed to the introduction of non-Gaussian behavior to gust velocity pressure ( $\hat{q}_{H,t=3s}$ ) estimates—at eave height—used for normalization of surface

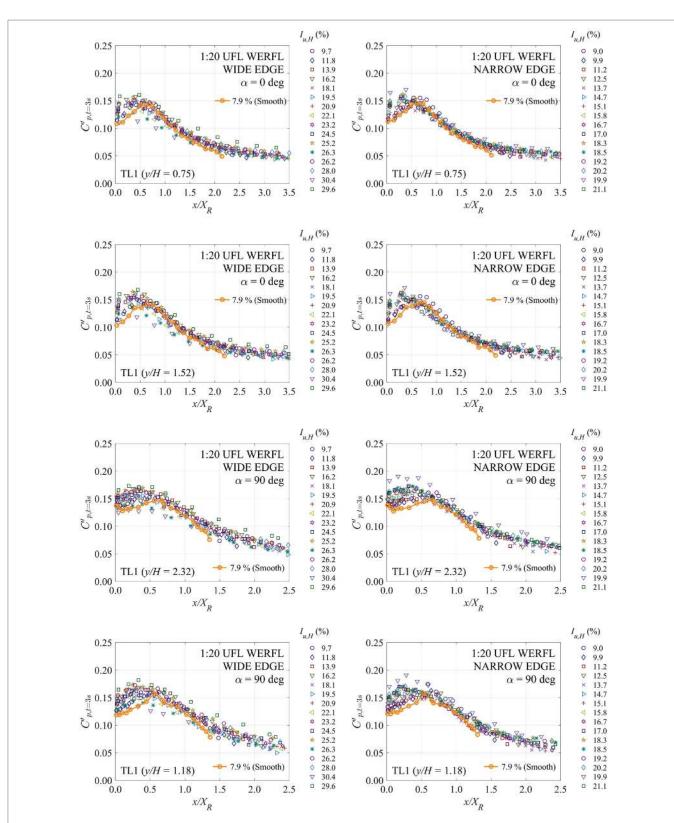


FIGURE 15 | Standard coefficient transects for the narrow (left column) and wide (right column) roughness element orientations normalized by the gust velocity pressure at eave height using a non-Gaussian peak factor model (Kareem and Zhao, 1994).

pressures. However, the normalization scheme was unable to fully collapse fluctuating pressure coefficients within the separation bubble, where the location of maximum pressure fluctuations along the roof tap line varied with freestream turbulence. Thus, the distribution of fluctuating pressures appear to be controlled by the interaction of the turbulent boundary layer with the structure of the separation bubble, while peak pressures can be fully characterized by the freestream flow conditions.

The non-Gaussian peak factor model applied to gust velocity estimates accounts for increased skewness ( $\gamma_{3u}$ ) values observed in the freestream velocity with increasing upwind roughness (or turbulence levels). In rougher terrain, low-rise buildings are partially or entirely immersed in the roughness sublayer (Tieleman, 2003). This layer is not part of inertial sublayer (i.e., "constant" stress region), thus many traditional wind engineering assumptions are no longer valid (e.g., Gaussian behavior). In the roughness sublayer, the flow field is dominated by the presence of non-Gaussian coherent structures (Raupach, 1981; Rotach, 1993). In particular, positively skewed wind fields in the roughness sublayer have been linked to downward motions (i.e., sweep) of highvelocity fluid into the canopy space (Poggi et al., 2004). The increasing trend in skewness of the freestream at eave height with upwind roughness is evident in the present work (Tables 3 and 4). Nevertheless, further research is required to investigate the relation between organized motions in the roughness sublayer and surface pressures on low-rise buildings.

## **CONCLUSION AND FUTURE WORK**

A series of BLWT experiments were conducted to investigate the effects of freestream turbulence on low-rise building roofs. A 1:20 model of the WERFL experimental building were immersed in 33 turbulent boundary layer flows via precise regulation of a computer control terrain generator called the Terraformer. The system permitted a fine resolution study of freestream turbulence effects on surface pressures and mean size of separating shear layers around bluff bodies. The paper confirms previous work from Akon and Kopp (2016) concerning the systematic reduction in mean reattachment length with rougher upwind terrains. Furthermore, a standardized form for displaying peak surface pressures close to separating shear layers is presented, where peak pressures are normalized by the gust velocity at eave height and distances along transects are normalized by the mean reattachment length. Gust velocities were computed from a non-Gaussian peak factor model, which appears to collapse the family of peak pressure transects. The normalization scheme is also applied to fluctuating roof pressures. SD coefficient transects

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corresponding to a family of freestream turbulence levels appear to collapse when normalized by the non-Gaussian gust velocity pressure. However, noticeable variations in the location of maximum pressure fluctuation along line transects are observed for different freestream turbulence intensities. This suggests that the spatial distribution of pressure fluctuations is mostly dominated by the interaction of the turbulent boundary layer with the structure of the separation bubble and less so by the freestream flow conditions.

Subsequent studies will center on further expanding on this work by examining effects of model scale (Stathopoulos and Surry, 1983) and buildings aspect ratio and incorporate more complex upwind terrain conditions (Fang and Sill, 1995) through the generation of random fields of roughness elements to simulate real-world heterogeneous terrain conditions. Further analysis will also be performed on the dataset to closely investigate the coherent (non-Gaussian) flow features in the roughness sublayer and their physical significance to the spatial distribution of surface pressures on low-rise buildings.

## **AUTHOR CONTRIBUTIONS**

This research builds on the dissertation work of PF (Fernández-Cabán and Masters, 2017), which may be accessed through the University of Florida Library's Electronic Theses and Dissertations platform (https://cms.uflib.ufl.edu/etd). FM provided valuable guidance in developing the paper in addition to revising the data analysis procedures.

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**Conflict of Interest Statement:** The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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