

Fig. 1. Plan view of the deep excavation (Xu, 2007)



Fig. 2. Cross section A-A, see Fig. 1 (Xu, 2007)



Fig. 3. Plan view of the ground floor slab and supporting beams (Xu, 2007)



 $s_{\mu}$  = field vane shear strength, c = cohesive strength,  $\phi$  = internal friction angle





Fig. 5. Undrained shear strength profiles determined from four separate sites in Shanghai



Fig. 6. Profile of  $G_0$  determined from shear-wave velocity tests in Shanghai



Fig. 7. Variation of normalised secant shear modulus  $G_{_{\!S}}/G_{_{\!0}}$  with shear strain  $_{\gamma}$ 



Fig. 8. Key field instrumentation locations



Fig. 9. Geometry and mesh of the whole model



Fig. 10. Diaphragm wall geometry and mesh



Fig. 11. Supporting system and superstructures



Fig. 12. Plot of  $G_t / G_0$  vs  $I_r$  for Equation (5) and also for the kinematic hardening model based on the parameters in Table 2.



Fig. 13. Plot of normalized deviator stress  $\overline{q}$  vs. normalized triaxial shear strain  $\overline{\varepsilon}_s$ . Comparison between the results of a triaxial compression tests determined on the basis of Equation (3) and the performance of the kinematic hardening model (including an unload-reload cycle) based on the parameters used in Table 2) The filled circles indicate intersections with one of the yield surfaces.



Fig. 14. Analysis strategy



Fig. 15. Wall deflection and ground settlements (effects of soil models)



Fig. 16. Wall deflections and Ground movements (thermal effects)



Fig. 17. Wall deflections and ground movements (effects of joints)



Fig. 18. Wall deflections and ground movements (effects of shell elements)



Fig. 19. Wall deflections and ground movements ( $K_0^t$  effects)