

## Hydromechanics of the Koyna–Warna Region, India

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**Abstract**—Continuous reservoir-induced seismicity has been observed in the Koyna–Warna region in western India following the beginning of impoundment of Koyna and Warna Reservoirs in 1961 and 1985, respectively. This seismicity includes 19 events with  $M \geq 5.0$  which occurred in 7 episodes (I–VII) between 1967 and 2005 at non-repeating hypocentral locations. In this study, we examined the first six episodes. The seismicity occurs by diffusion of pore pressures from the reservoirs to hypocentral locations along a saturated, critically stressed network of NE trending faults and NW trending fractures. We used the daily lake levels in the two reservoirs, from impoundment to 2000, to calculate the time history of the diffused pore pressures and their daily rate of change at the hypocentral locations. The results of our analysis indicate that Episodes I and IV are primarily associated with the initial filling of the two reservoirs. The diffused pore pressures are generated by the large (20–45 m) annual fluctuations of lake levels. We interpret that critical excess pore pressures  $> \sim 300$  kPa and  $> \sim 600$  kPa were needed to induce Episodes I–III and Episodes IV–VI, respectively, suggesting the presence of stronger faults in the region. The exceedance of the previous water level maxima (stress memory) was found to be the most important, although not determining factor in inducing the episodes. The annual rise of 40 m or more, rapid filling rates and elevated  $dp/dt$  values over a filling cycle, contributed to the rapid increase in pore pressure.

**Key words:** Reservoir-induced seismicity, Koyna reservoir, pore pressure diffusion, stress memory, strain hardening.

### 1. Introduction

Continuous reservoir-induced seismicity (RIS) has been observed since 1963 in the Koyna–Warna region (KWR) after the impoundment of the Koyna Reservoir (in western India) in 1961 and Warna Reservoir in 1985. The seismicity covers a volume  $\sim 30$  km long,  $\sim 20$  km wide and  $\sim 10$  km deep. Through 2006, more than 100,000 earthquakes have been recorded, including more than 170 events with

$M \geq 4$  and 19 events with  $M \geq 5$  (henceforth referred as M 5 events) with non-repetitive epicenters (GUPTA, 2002; GUPTA *et al.*, 2007). Over the years, the KWR has been the subject of numerous studies (see e.g., in the last two decades, GUPTA, 1992, 2002; TALWANI *et al.*, 1996; TALWANI, 1997b, 2000; RASTOGI *et al.*, 1997; MANDAL *et al.*, 1998; GUPTA *et al.*, 2002, 2005, 2007; SATYANARAYANA *et al.*, 2005; KUMAR *et al.*, 2006; SARMA and SRINAGESH, 2007; CHADHA *et al.*, 2008). A few studies have been aimed at understanding the mechanism of RIS at the KWR (e.g., SIMPSON *et al.*, 1988; KALPNA and CHANDER, 2000; TALWANI 1995, 1997a; RAJENDRAN and HARISH, 2000; PANDEY and CHADHA, 2003).

A unique observation in the region is the continuing occurrence of M 5 earthquakes, 44 and 20 years after the impoundment of the Koyna and Warna Reservoirs, respectively. In this paper, we combine our improved understanding of both the seismotectonic framework of the KWR and the mechanism of RIS in order to explain the RIS and hydromechanics of the KWR, by a study of the pore pressure history associated with the spatial and temporal evolution of M 5 earthquakes. We chose M 5 events because of their non-repeating hypocentral locations, each of which was associated with a  $\sim 10$  km<sup>2</sup> rupture area. Understanding the origin of these earthquakes is challenging and fascinating, and is the subject of this study.

We first present the revised seismotectonic framework of the KWR in light of results of studies of recent seismicity and geophysical investigations. We suggest the association of various fractures and faults with the M 5 earthquakes. Our analysis suggests that the saturated fractures in the KWR are critically stressed (Sect. 2). Next we describe our understanding of the predominant role of pore pressure diffusion (CHEN and

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TALWANI, 2001) in isolated, discrete fractures (CORNET and YIN, 1995; TALWANI *et al.*, 1999; EVANS *et al.*, 2005; TALWANI *et al.*, 2007) in the onset of RIS due to lake level fluctuations. We address (a) the role of long-period lake level fluctuations (ROELOFFS, 1988) on the diffusion of pore pressure to long distances; (b) the increase in pore pressures to threshold levels to induce M 5 earthquakes; (c) the observation that large  $dp/dt$  changes help trigger RIS (NUR and BOOKER, 1972; TALWANI *et al.*, 2007) (Sect. 3). Then we present observations that show that some of the episodes of M5 earthquakes display evidence of stress memory in this large, critically stressed volume (Sect. 4). The specific goals of this study to understand the hydromechanics of the KWR are described in Sect. 5. To achieve them, we use the daily lake-level data at Koyna and Warna Reservoirs up to 2000 and the assumption of one-dimensional pore pressure diffusion through fractures comprising the seismotectonic framework to calculate the pore pressure ( $p$ ) and the rate of pore pressure change ( $dp/dt$ ) histories at hypocentral locations of M 5 earthquakes (Sect. 6).

We conclude that the continued occurrence of seismicity in the KWR for more than 45 years is a result of a combination of factors, which include (a) the renewed generation of pore pressure ( $\sim 100$  s kPa) due to large (20–45 m) annual fluctuations of lake levels of the Koyna and Warna Reservoirs; (b) the availability of a network of fractures with permeabilities greater or equal than the seismogenic permeability ( $k \geq k_s$ ) to diffuse pore pressure to long distances in a (c) critically stressed volume ( $30 \times 20 \times 10$  km<sup>3</sup>); (d) the cumulative effect of annual cycles of lake-level changes contributes to an increase of the pore pressure to threshold levels; (e) the exceedance of lake levels over the previous maxima; (f) large  $dp/dt$  amplitudes and filling rates, which appear to contribute to higher pore pressures; (g) elastic–plastic deformation with strain hardening, which possibly strengthens the fractures, accounts for the observation of stress memory in this critically stressed volume (Sect. 7).

## 2. Seismotectonic Framework in the KWR

In the KWR the seismicity is associated with lake-level changes in the two reservoirs. Figure 1a

compares the daily histogram of the number of  $M \geq 4.0$  recorded earthquakes (catalog from Gupta 2002) with the lake levels in the two reservoirs since their impoundment in 1961 (KR) and 1985 (WR) to 2000. Figure 1b compares the times of occurrence of M 5 events with the two lake levels in and their locations are given in Fig. 2.

TALWANI (1997b) integrated seismological data from 1962 to 1995 with geomorphic, geological and geophysical data to describe the seismotectonic framework of the Koyna–Warna area. In the revised seismotectonic framework (Fig. 2), we use additional information based on the analysis of over 600 digitally recorded earthquakes from April 1996 to December 1997 (SRINAGESH and SARMA 2005; SARMA and SRINAGESH 2007) and magnetotelluric surveys in the Koyna area (SARMA *et al.* 2004; HARINARAYANA *et al.* 2007).

The revised seismotectonic framework (Fig. 2) consists of a pattern of crisscrossing steeply dipping, NE trending faults, associated with primarily left-lateral strike-slip motion; and a system of sub-parallel, NW trending near vertical fractures, associated with primarily normal faulting; in response to a  $\sim$ N-S oriented stress field (TALWANI, 1997b; GUPTA *et al.*, 2002; SARMA and SRINAGESH, 2007). The former include the Koyna River fault zone (KRFZ), Donachiwada, Patan, and P1 faults, and the latter include linear features L1–L7 (Fig. 2). (We have renamed the lineaments given in TALWANI (1997b) to follow the nomenclature provided by GUPTA *et al.*, 2002). The steep NW-SE trending faults divide the area between the Koyna and Warna rivers into distinct blocks, providing conduits for fluid pressure flow to hypocentral depths.

L3 is a major NW-SE lineament that crosses the Koyna–Warna region and extends up to the Warna dam. It was found by LANGSTON (1981) from the inspection of Landsat images. L1 and L2 are based on the analysis of seismicity data by TALWANI *et al.* (1996) and TALWANI (1997b). L4 and L5 are based on aeromagnetic data (TALWANI *et al.*, 1996; PESHWA, 1991). The location of the Patan fault is based on geomorphological features and 1994 seismicity data (TALWANI *et al.*, 1996, TALWANI, 1997b). P1 is exclusively based on the analysis of seismicity data (TALWANI *et al.*, 1996; TALWANI, 1997b). L6 and L7 are based on the revised hypocentral locations obtained by SRINAGESH and SARMA (2005) and SARMA

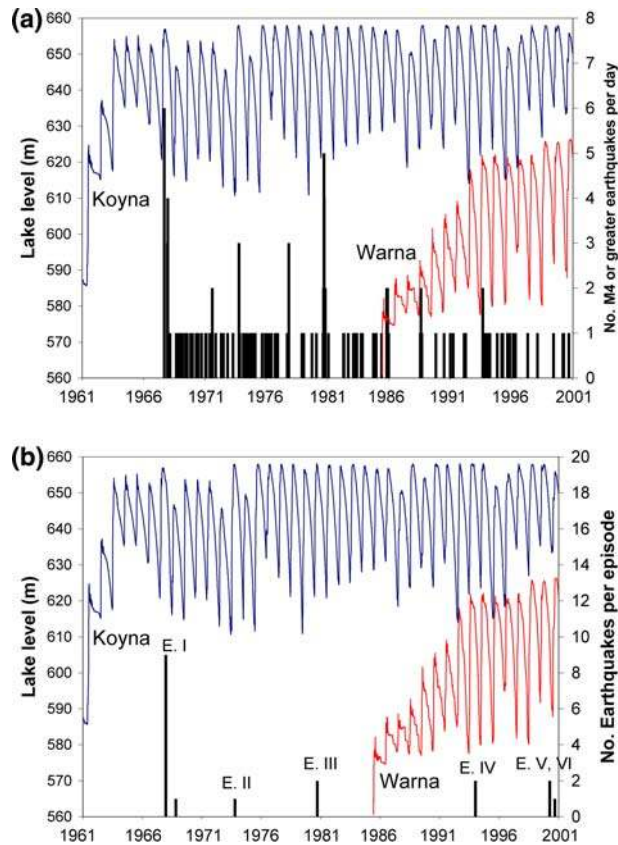


Figure 1

**a** Koyna and Warna water level fluctuations from impoundment to 2001, and daily histogram of the number of  $M \geq 4.0$  recorded earthquakes (catalog from GUPTA, 2002). **b** Koyna and Warna water level fluctuations from impoundment to 2001, and times of occurrence of  $M \geq 5.0$  earthquakes (Episodes I–VI)

and SRINAGESH (2007). They divided the revised hypocentral locations into three zones, one of which lies along the Donachiwada fault, the other two NW–SE trending zones were labeled SEZ and WSZ. Cross sections across WSZ and SEZ show that earthquakes lie along four vertical  $\sim$ parallel NW–SE trending planes, two of which coincide with L4 and L5 described earlier, and the other two we have named L6 and L7 (Fig. 2). The locations of the 19  $M \geq 5$  earthquakes in the KWR are shown in Fig. 2, and their causative faults are listed in Table 1a and b.

### 3. Mechanisms of RIS Applied to the KWR

TALWANI (1997a) determined that the protracted induced seismicity in the Koyna–Warna region is a consequence of the large annual water level

fluctuations associated with the monsoon season. The study of the spatio-temporal pattern of seismicity in the KWR for over four decades, and comparison with the anomalously large (25–45 m) annual lake-level fluctuations reveals some unique features. The seismicity is primarily associated with, and delayed with respect to, the large annual filling between June and August, with about two-thirds of the annual seismic energy release occurring from September to December, including the continuing occurrence of  $M4$  and 5 events long after the initial impoundment (TALWANI, 1997b, 2000; TALWANI *et al.*, 1996). The seismicity is induced by excess pore pressures associated with the filling, as they diffuse through saturated, critically stressed fractures (TALWANI, 1997b; TALWANI *et al.*, 2007). It is confined to a  $\sim 30$  km long, 20 km wide and 10 km deep seismogenic volume (Fig. 2).

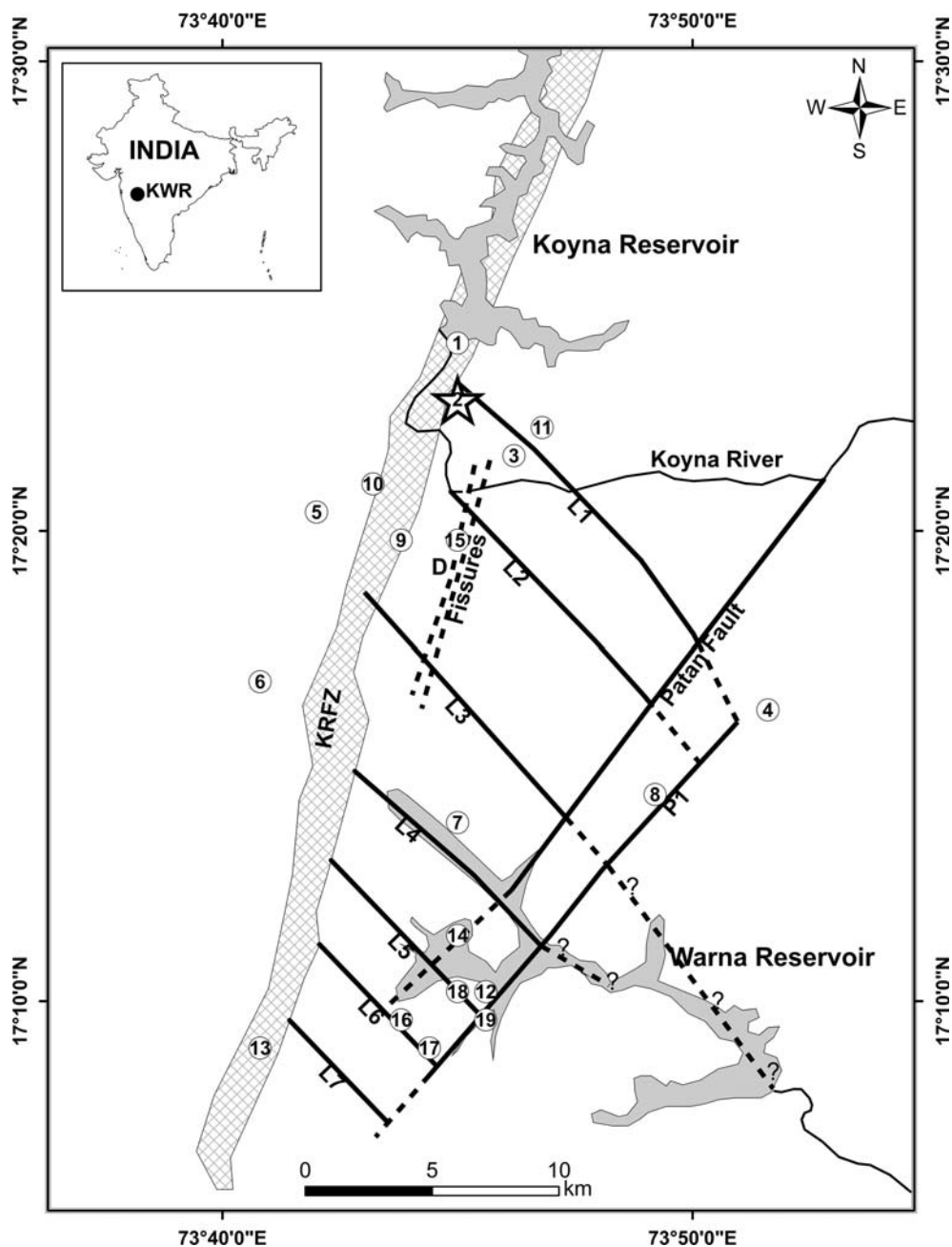


Figure 2

Location of Koyna–Warna region in western India, (*inset*), showing the chronological locations of 19  $M \geq 5$  events, taken from TALWANI et al. (1996), GUPTA et al. (2002 and 2005) superposed on the revised seismotectonic framework (this study) modified from TALWANI (1997a) (see text for details). D is the Donachiwada fault, with fissures associated with the 1967 shock

Several other factors are known to be associated with reservoir-induced seismicity (RIS). These include the total height of the water column, the filling rate, the duration of the highest lake level and the amplitude and frequency of the lake level

fluctuations (see e.g., GUPTA, 2002). Another observation, the occurrence of large magnitude events when the lake level exceeds the previous maximum lake-level (GUPTA and COMBS, 1976; SIMPSON and NEGMATULLAEV, 1981) was recognized to occur in the

Table 1  
 Lake levels and episodes of  $M \geq 5.0$  events near (a) Koyna Reservoir<sup>1</sup>, (b) Warna Reservoir<sup>2</sup>

Previous $H_{\max}$ exceeded on Y M D	New $H_{\max}$ reached on Y M D	New $H_{\max}$ (m)	$\Delta h$ (m)	Episode	Eq. #	Event Date Y M D	Magnitude	Days after exceeded $H_{\max}$	Fault mechanism	Causative fault
<b>(a) Koyna</b>										
	61 07 11	624.68			1	67 09 13	5.2	14	Strike-slip (LANGSTON 1981)	KRFZ
62 07 11	62 08 15	636.65			2	67 12 10	6.3		Strike-slip (TALWANI 1997b)	Near the intersection of KRFZ and L1
63 07 09	63 08 14	654.10			3	67 12 10	5.0			L1
64 08 01	64 08 13	654.94			4	67 12 11	5.2			P1
65 07 22	65 07 22	655.16			5	67 12 12	5.4		Normal (LANGSTON and FRANCO-SPERA 1985)	L3
67 08 30	67 10 04	656.99	1.83	I	6	67 12 12	5.0		Strike-slip (RAO et al. 1975)	KRFZ
					7	67 12 12	5.0			L4
					8	67 12 24	5.5			P1
					9	67 12 25	5.1			KRFZ
					10	68 10 28	5.4			KRFZ
73 09 02	73 09 27	657.98	0.99	II	11	73 10 17	5.1	45	Strike-slip? (GUPTA et al., 1980) CFPS	L1
80 09 01	80 09 03	658.13	0.15	III	12	80 09 02	5.5	1		L5
94 09 02	94 09 02	658.2	~0.1		13	80 09 20	5.3	19	Normal (DZIEWONSKI et al., 1988)	KRFZ/L7
02 09 07	02 09 07	658.3	~0.1			No EQ.				
<b>(b) Warna</b>										
86 06 25	85 08 01	581.30				No EQ.				
87 07 08	86 06 30	586.15				No EQ.				
88 07 14	87 07 09	588.50								
89 07 05	88 07 19	592.65								
90 07 09	89 07 24	601.50								
91 07 15	90 07 16	604.15								
92 07 29	91 07 28	609.06								
93 07 03	92 08 19	617.90								
	93 08 04	621.80	3.9	IV	14	93 12 08	5.2	158	Normal (TALWANI 1997b)	Patan or L5
94 09 02	94 09 02	622.05	0.25		15	94 02 01	5.4	213	Normal (TALWANI 1994)	Patan
98 08 28	98 09 28	625.85	3.8	V	16	00 03 12	5.2	562	Normal (GUPTA et al. 2002)	L6 or Patan
00 09 11	00 10 16	626.30	0.45	VI	17	00 04 06	5.1		Normal (GUPTA et al. 2002)	L6 or P1
03* 09 10 (est)	03 09 13	627.88	1.58	VII	18	00 09 05	5.3		Normal (GUPTA et al. 2002)	L5
					19	05 03 14	5.2	551	Normal (KUMAR et al. 2006)	L5

<sup>1</sup> Data compiled from TALWANI et al. (1996), TALWANI (2000), GUPTA et al. (2002)

<sup>2</sup> Data compiled from TALWANI et al. (1996), GUPTA et al. (2002, 2005) and SATYANARAYANA et al. (2005)

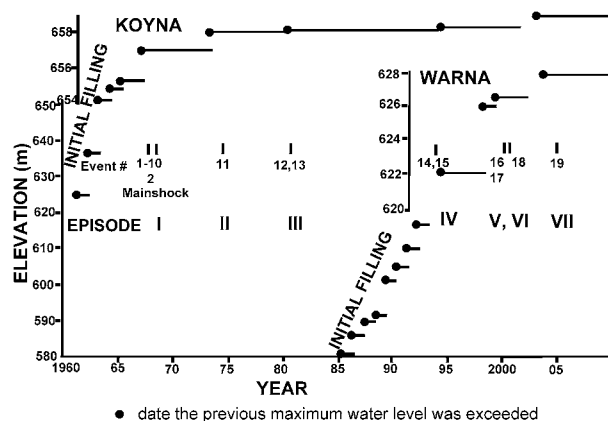


Figure 3

Temporal distribution of  $M \geq 5$  events and the seven episodes of stress memory, compared with the lake levels. The *solid dots* show the date when the previous maximum water level was exceeded, and the horizontal bars when it was not. The initial fillings of the Koyna and Warna Reservoirs (1961–1965 and 1985–1992) were followed by small rises in those two reservoirs (shown with an exaggerated scale).  $M \geq 5$  events, constituting the episodes were found to occur only when the previous lake level was exceeded, displaying stress memory in the rocks of the KWR

KWR for three  $M \geq 5$  events, and the phenomenon, stress memory, to be a crustal manifestation of the Kaiser effect observed in the laboratory (SIMPSON *et al.*, 1988; TALWANI *et al.*, 1996; TALWANI, 1996, 2000). More recent studies show that most of the  $M \geq 5$  events in the KWR manifest evidence of stress memory (next section).

Assuming a simple 2-D reservoir, ROELOFFS (1988) calculated that the stress and pore pressure fields produced by time oscillatory reservoir loading are governed by a dimensionless frequency  $\Omega$  given by  $\Omega = \omega L^2/2c$ , where  $\omega$  is the loading frequency,  $L$  is the width of the reservoir and  $c$  is the hydraulic diffusivity. The depth below which diffusion associated with cyclic pore pressure changes at the surface is negligible, is given by  $z = \pi(2c/\omega)^{1/2}$ . TALWANI (1997a) calculated this distance for a range of  $c$  and  $\omega$ . For the Koyna and Warna Reservoirs (loading frequency is 1/year) he found it to be 25 km for  $c = 1 \text{ m}^2/\text{s}$  and 78 km for  $c = 10 \text{ m}^2/\text{s}$ . KESSELS and KÜCK (1995) showed the applicability of the 1-D diffusion analysis along horizontal fractures. The observation of RIS more than 30 km south of the Koyna Reservoir suggests that the pore pressures associated with the annual filling cycles diffused along fractures connecting the reservoirs to the hypocenters.

NUR and BOOKER (1972) showed that when aftershocks could be attributed to pore pressure changes,

their frequency was proportional to the rate of pore pressure increase. TALWANI *et al.* (2007, Fig. 3a and b in their paper) demonstrated that the time of occurrence of induced earthquakes at the Monticello Reservoir coincided with a peak in the rate of pore pressure change,  $dp/dt$ , with time.

In summary, earlier work suggests that RIS is associated with highly diffused pore pressures (in accordance with the Coulomb failure criterion) and that it is triggered by high  $dp/dt$  values. We will test these ideas with over 40 years of lake level and seismicity data from the KWR.

#### 4. Stress Memory

Stress memory of rocks is the ability to accumulate, retain and, under certain conditions, to reproduce information on the stresses experienced in the past (LAVROV, 2003). The Kaiser effect and strain hardening are manifestations of stress memory in various types of materials (e.g., metals and rocks), and are called stress memory effects (see LAVROV, 2003, for a review). The Kaiser-effect was discovered in the laboratory by KAISER (1953). During cyclic loading with increasing stress, he observed a marked increase in acoustic emission activity (or microseismicity) when the largest stress level of the previous cycle was exceeded.

Another possible stress memory effect is strain hardening. Strain hardening is a strengthening mechanism that increases the yield point of a material, i.e., upon reloading a material will not deform plastically further unless the load is increased above its previous level (HILL, 1950; MENDELSON, 1983). Strain hardening is associated with plastic contraction of rocks, their subparticles become irreversibly more densely packed, which strengthens their elastic interactions that better withstand the subparticles from being moved apart (COUSSY, 1995). Seminal work on elastic–plastic deformation of rocks was carried out by Rice (1975, 1977) and RUDNICKI and RICE (1975). Strain hardening can only take place during elastic–plastic deformation of rocks. Given the stress-strain curve of a material, its behavior can be characterized as elastic, elastic–plastic or perfectly plastic, depending on how large the plastic (permanent) strain is with respect to the elastic strain (MENDELSON 1983). In the elastic case, the elastic strain is dominant and the stress-strain curve follows the generalized Hooke’s law. In the rigid-plastic or perfectly plastic case, the plastic strain is dominant and the theory of plasticity is applicable. Finally, in the elastic–plastic case, the elastic and plastic strains are comparable, and both the generalized Hooke’s law and the theory of plasticity have to be applied. Recent work by TEMPLETON and RICE (2008) and VIESCA *et al.*, (2008) examine off-fault elastic–plastic deformation for dry and fluid-saturated materials, respectively, focusing mainly on the undrained (no fluid diffusion) effects. We suggest from our study that elastic–plastic deformation with strain hardening in drained conditions might explain the phenomena of stress memory observed in the KWR.

#### 4.1. Stress Memory in the KWR

Figure 2 shows the location of the 19 events ( $M \geq 5$ ) plotted on an outline of the revised seismotectonic framework. These 19 events, which include the largest reservoir induced event, M 6.3 in December, 1967, occurred in seven episodes (Table 1a and b). An episode is defined as a sequence of one or more  $M \geq 5.0$  events that follows the exceedance of a lake-level elevation above the previous highest value ( $H_{\max}$ ). (Different magnitudes

have been listed for the larger events by various workers. In this paper, the magnitudes of these events have been taken from GUPTA *et al.*, 2002).

Episode I which includes the M 6.3 main shock in December, 1967, consists of ten  $M \geq 5$  events, while the other episodes consist of one or two such events (Table 1a and b). The temporal distribution of these events, vis-a-vis the water levels in the Koyna and Warna Reservoirs is shown in Fig. 1b.

Evidence of stress memory was first realized with the comparison of the daily water level changes with the times of onset of the first three episode of  $M \geq 5.0$  (TALWANI, 1996, 2000; TALWANI *et al.*, 1996). The highest water level and filling rate in any year depends on the annual rainfall between June and August, and on the draw-down governed by engineering considerations. In Fig. 3 we show those times when the previous maximum water levels ( $H_{\max}$ ) are exceeded (solid dots), and the durations when they are not (horizontal lines). We compare the times of maximum water levels ( $H_{\max}$ ) for the Koyna and Warna Reservoirs with the times of Episodes I–VII.

Filling of the Koyna Reservoir started in 1961 and  $H_{\max}$  was exceeded on July 11, 1962, and the new  $H_{\max}$  (636.65 m) was reached on August 15, 1962. The initial filling stage lasted until 1965, with  $H_{\max}$  value of 655.16 m. This value of  $H_{\max}$  was first exceeded on August 30, 1967. It was followed by a M 5.2 event (#1) on September 13, 1967. The highest water level observed that year was on October 4, 1967 (656.99 m) and the largest reservoir induced event, M 6.3, occurred on December 10, 1967 (UTC), (4.21 a.m. on December 11, 1967 local time). This event was followed by five aftershocks with  $M \geq 5.0$  in the following 2 days, and three later, the last one on October 29, 1968 (GUPTA *et al.*, 2002).

The farthest event (#8 on December 24, 1967) was located  $\sim 20$  km to the southeast. Other  $M \sim 4$  aftershocks were located near the present-day Warna Reservoir, which had not been impounded at that time (TALWANI, 1997b). The locations of the aftershocks are probably good to better than  $\sim 5$  km. These ten events constituting the foreshock-main shock-aftershock sequence constitute Episode I, and their pattern is similar to RIS at other locations following the initial impoundment (TALWANI, 1997a).

The 1967  $H_{\max}$  (656.99 m) was exceeded on September 2, 1973 and the new  $H_{\max}$  (657.98 m) was reached on September 27, 1973. It was followed by a magnitude 5.1 event on October 17, 1973, (earthquake #11) which constitutes Episode II.

The 1973  $H_{\max}$  was exceeded on September 1, 1980 and was followed by an M 5.5 event the next day.  $H_{\max}$  (658.13 m) was reached on September 3, 1980. It was followed by an M 5.3 event (#13) on September 20, 1980. These two events, which were located more than 25 km from the Koyna Reservoir, constitute Episode III. Earthquake #13 was associated with the Koyna River fault zone, and earthquake #12 was located on L5. TALWANI (1997b) speculated that these locations were connected to the Koyna Reservoir by a network of cross-cutting faults and fractures (Fig. 2).

The next episode did not occur until after the impoundment of the Warna Reservoir, which began in 1985. Following its impoundment there were four more episodes of  $M \geq 5$  events (Table 1b).

After the initial filling of the Warna Reservoir (1985–1992), the 1992  $H_{\max}$  (617.90 m) was exceeded on July 3, 1993 and the new  $H_{\max}$  (621.80 m) was reached on August 4, 1993. Two  $M \geq 5$  events (#14 and #15, constituting Episode IV) occurred on December 8, 1993 and on February 1, 1994 at distances of  $\sim 13$  and  $\sim 25$  km from the deepest part of the Warna Reservoir (near the dam) respectively, the latter event was associated with normal faulting on the Patan fault. Its aftershocks lie along L2 (Fig. 2). These events, which occurred 158 and 213 days after the 1992  $H_{\max}$  was exceeded, showed evidence of pore pressure diffusion to the northwest of the Warna Reservoir, possibly along the same sets of fractures that were associated with southward pore pressure diffusion from the Koyna Reservoir during earlier episodes. We attribute these events to the initial filling of the Warna Reservoir.

The  $H_{\max}$  in the Warna Reservoir was next exceeded in September 2, 1994 by 0.25 m and then again in 1998 by 3.8 m (Table 1b). The next Episode, V, consists of two events that occurred during a period of lake-level decline in March and April 2000, about 10 km from the deeper part of the Warna Reservoir. The March 2000 event occurred after a lag of 562 days after the 1994  $H_{\max}$  was exceeded on

August 28, 1998. These two events were the farthest south of the  $M \geq 5$  events in the KWR, and occurred in a previously aseismic area.

Earthquake #18 on September 5, 2000 occurred 6 days before the 1998  $H_{\max}$  (625.85 m) was exceeded. Although not strictly consistent with our definition of an episode, we consider earthquake #18 as a new Episode, VI, because it was associated with a water level rise and the elevation was a few cm of the ultimate  $H_{\max}$  (626.30 m). This event was located closer to the reservoir than the previous two.

The 2000  $H_{\max}$  was exceeded on  $\sim$ September 10, 2003, and earthquake #19 constituting Episode VII, occurred on March 03, 2005, 551 days later, during a period of lake-level decline.

Summarizing, there have been seven episodes of  $M \geq 5$  events in the KWR in the last four decades. The 19 events that form these episodes did not repeat their hypocentral locations, and are contained in a critically stressed  $30 \times 20 \times 10 \text{ km}^3$  volume. Comparing the temporal patterns of these episodes with laboratory data, we can think of Episodes I and IV, which followed the impoundment of the Koyna and Warna Reservoirs, as being associated with the initial filling. We suggest that Episodes II, III, IV, VI and plausibly also V and VII, are manifestations of stress memory on a crustal scale. That is, a  $\sim 6,000 \text{ km}^3$  seismogenic volume remembers the water level maximum  $H_{\max}$  that preceded a  $M \geq 5$  event, and when that water level is exceeded, even by 0.15 m, a new episode occurs.

If the excess lake load above the previous  $H_{\max}$  was the only contributing factor, it would imply very small triggering stresses associated with these M 5 events. Next we examine other possible causes of additional stress perturbations, which are summarized in the next section.

## 5. Goals of the Study

Several factors have been suggested as possible contributors to the development of the critical pore pressure ( $p_c$ ) and the triggering of M 5 events. To obtain an estimate of  $p_c$  values associated with various episodes and the relative contributions of different factors, we analyzed the excess pore pressures associated with the filling histories of the two reservoirs.



Specifically, we evaluated the contributions of the following factors: (a) the annual cycle of lake-level fluctuations; (b) the amplitude of the annual increase in lake level,  $\Delta H$ , especially for those years when it exceeds 40 m; (c) the maximum rate of pore pressure ( $dp/dt$ ) increase over a filling cycle,  $A$ ; (d) exceedance of the lake level over a previous maximum level (identified as a manifestation of stress memory in rocks); (e) the cumulative effect of annual cycles of filling and emptying of the reservoir between episodes; (f) complete leakage or no leakage of built-up pore pressures between episode and (g) the combined effect of the two reservoirs.

To study the *relative* contributions of these factors, we calculated the excess pore pressure by assuming simple one-dimensional diffusion of pore pressure through a network of fractures based on our current understanding of the seismotectonic framework. Although the fractures are two-dimensional features, hypocentral locations suggest that only finite strips of the fractures are seismogenic and conduits for pore pressure diffusion. As the details of these are not known, we assumed simple one-dimensional pore pressure diffusion between the reservoirs and the hypocentral locations. The calculated pore pressures may overestimate the true excess pore pressures nonetheless they give meaningful estimates of the relative contributions of the various factors.

## 6. Calculation of Diffused Pore Pressure Histories at Hypocentral Locations

### 6.1. Theory

ROELOFFS (1988) calculated the pore pressure at time  $t > 0$  and a depth  $z$  due to a change in the level of a reservoir lake behind a dam at time  $t = 0$ . The total pore pressure at a depth  $z$  and time  $t$ ,  $p(z, t)$  can be expressed as the coupled response of diffused pore pressure and undrained pore pressure.

$$p(z, t) = (1 - \alpha) \cdot p_0 \cdot \operatorname{erfc} \left[ \frac{z}{(4ct)^{1/2}} \right] + \alpha \cdot H(t) \cdot p_0, \\ \alpha = B \cdot (1 + \nu_u) / [3 \cdot (1 - \nu_u)] \quad (1)$$

where  $\operatorname{erfc}$  is the complementary error function,  $H(t)$  is the Heaviside unit step function,  $B$  is the Skempton's

coefficient,  $\nu_u$  is the undrained Poisson's ratio,  $z$  is the depth beneath the reservoir,  $p_0$  is the vertical stress at a depth  $z = 0$ ,  $c$  is the hydraulic diffusivity and  $t$  is time.

The undrained pore pressure term in Eq. (1) builds-up instantaneously in the vicinity of a reservoir via the Skempton effect due to the load of water. For a point load, the undrained pore pressure effect is contained within a cone whose horizontal and vertical dimensions are about 3–4, and 6–7 times the width of the reservoir (TALWANI et al., 2007). For the Koyna Reservoir, assuming the deepest part to be adjacent to the dam and a maximum width of  $\sim 2.5$  km, the undrained pore pressure is contained within a horizontal distance of  $\sim 10$  km and a vertical distance of  $\sim 17.5$  km. Our study focuses on M 5 earthquakes, some of which occurred in the vicinity of the Koyna Reservoir within the cone shaped volume with a non-zero undrained effect. We estimate this undrained effect at hypocentral locations within this volume.

The pore pressure increase due to compression (undrained response) at a distance  $r$  from the reservoir is

$$\Delta p_u(r) = -B\Delta\sigma_{kk}/3, \quad (2)$$

where  $\Delta\sigma_{kk}/3$  is the change in the mean stress at the given distance  $r$ . Considering a uniform circular load, AHLVIN and ULERY (1962) tabulated the vertical, radial and transverse stresses at a point at any radial distance from the center of the circular load and at a given depth. Assuming  $B \sim 0.7$  for crystalline rocks (TALWANI et al., 1999) and water height of  $\sim 70$  m, for an earthquake at a depth of 4 km and a radial distance of 2 km, the undrained response is of the order of 20 kPa (Appendix 1), one order of magnitude less than the diffused pore pressure values calculated in this study (see next section). Therefore, we disregard the undrained effect in our calculations and consider only the changes in pore pressure due to diffusion. Substituting  $\alpha = 0$  (no undrained effect) in Eq. (1), the diffused pore pressure component can be written as

$$p(z, t) = p_0 \cdot \operatorname{erfc} \left[ \frac{z}{(4ct)^{1/2}} \right] \quad (3)$$

RAJENDRAN and TALWANI (1992) and CHEN and TALWANI (2001) calculated the diffused pore

pressures away from a reservoir. We have revised the formulation following the methodology given in those papers and, applying the principle of superposition to the equation obtained by ROELOFFS (1988). The following mathematical expression can be written for one-dimensional pore pressure diffusion in any direction:

$$P_n = \delta p_1 \cdot \operatorname{erfc} \left[ \frac{r}{(4cn\delta t)^{1/2}} \right] + \delta p_2 \cdot \operatorname{erfc} \left[ \frac{r}{(4c(n-1)\delta t)^{1/2}} \right] + \delta p_3 \cdot \operatorname{erfc} \left[ \frac{r}{(4c(n-2)\delta t)^{1/2}} \right] + \dots + \delta p_n \cdot \operatorname{erfc} \left[ \frac{r}{(4c\delta t)^{1/2}} \right] \quad (4)$$

where  $\delta t$  is a fixed increment of time (i.e., 1 day, see next section),  $n$  is the number of time increments considered in our calculation (i.e., number of days),  $\delta p_n$  is the water load change for the  $n$ -th day,  $r$  is the distance from the reservoir,  $c$  is the hydraulic diffusivity and  $\operatorname{erfc}$  is the complementary error function. The equation states that the diffused pore pressure  $n$  days after the filling of a reservoir at a distance  $r$  from the reservoir is the sum of the diffused pore pressures generated by discrete water load changes in a given time step  $\delta t$ . The first discrete water load change  $\delta p_1$  contributes for a total of  $n$  days to the total diffused pore pressure, while the last discrete water load change  $\delta p_n$  contributes only for one day.

We use Eq. (4) to calculate the diffused pore pressure associated with the impoundment and annual filling cycles of the Koyna and Warna Reservoirs at the times and locations of episodes of M 5 earthquakes. The water load changes  $\delta p$  correspond to time steps  $\delta t$  of 1 day. The rate of pore pressure change  $dp/dt$  is calculated by subtracting the pore pressure values in two consecutive days and dividing them by the time step of 1 day. Next we describe how we estimate the distances from the reservoirs to the earthquakes and the hydraulic diffusivity values along the diffusion paths.

## 6.2. Data and Assumptions

Water-level data cover the period from 1961 (impoundment) to 2000 for the Koyna Reservoir

(14,606 data points) and 1985 (impoundment) to 2000 for the Warna Reservoir (5,685 data points). Figure 1b shows the daily water-level curves for the Koyna and Warna Reservoirs, and the times of the episodes (vertical lines). The hypocentral distance from the reservoir,  $r$ , assuming that pore pressure diffusion is restricted to faults and fractures (CORNET and YIN, 1995; TALWANI *et al.*, 1999; EVANS *et al.*, 2005), was estimated from the inferred paths of pore pressure diffusion along them. The values of hydraulic diffusivity,  $c$ , along these paths were obtained from TALWANI *et al.* (2007, Tables A.1 and A.3 in that paper), and vary between  $\sim 1$  and  $10 \text{ m}^2/\text{s}$ . Based on this range of values, we calculate diffused pore pressures in the Koyna–Warna region for three values of  $c$ , i.e., 1, 5 and  $10 \text{ m}^2/\text{s}$ .

The distance to the hypocenter for pore pressure diffusion,  $r$ , was calculated from the dams because they represent the deepest part of the reservoir and water levels are measured next to the dams. In calculating this distance,  $r$ , we first estimated the total horizontal distance ( $x$ ) along the chosen surface path. Then, taking the hypocentral depth ( $z$ ), we estimated  $r = (x^2 + z^2)^{1/2}$ . Table 2a–c list the estimated values of  $x$ ,  $z$  and  $r$  for the different earthquakes. Because of the uncertainties in the estimation of distances, and to obtain a meaningful order of magnitude estimates of the diffused pore pressures, we used a range of representative distances for  $r$ , rounded to the nearest 5 km (shaded in Table 2a–c).

Given the uncertainties in the horizontal distances along fractures, depths and hydraulic diffusivity values, we calculated the diffused pore pressures for a range of distances and hydraulic diffusivity values. For example, we assigned earthquake #11 to occur on L1. We assumed the path of pore pressure diffusion from the Koyna Reservoir to the hypocenter of earthquake #11 to be along KRFZ and L1 (Fig. 2). Taking the horizontal distances along KRFZ and L1 to be  $\sim 2.0$  and  $4.3 \text{ km}$  we obtain  $x = 6.3 \text{ km}$ . With the hypocentral depth  $z = 4.0 \text{ km}$ , we obtain the hypocentral distance  $r = 7.5 \text{ km}$ . We used 5 km and 10 km as representative distances in our calculations of diffused pore pressures. In common with other earthquakes, the pore pressure for earthquake #11

Table 2

*Distances from Koyna dam to Episodes (a) I, II and III, (b) IV and V, (c) Warna dam to Episode IV, V and VI*

Earthquake # (Date)	Route followed	Depth, $z$ (km)	Horizontal distance along fractures, $x$ (km)	Hypocentral Distance, $r$ (km)	Representative $r$ (km)
(a)					
Episode I					
2 (67 12 10)	KRFZ	10.0 (TALWANI <i>et al.</i> , 1996)	2.7	10.4	5, 10
Episode II					
11 (73 10 17)	KRFZ, L1	4.0 (TALWANI <i>et al.</i> , 1996)	6.3	7.5	5, 10
Episode III					
12 (80 09 02)	KRFZ, L5	8.0 (GUPTA <i>et al.</i> , 2002)	29.4	30.5	30
13 (80 09 20)	KRFZ	12.0 (GUPTA <i>et al.</i> , 2002)	28.8	31.2	
(b)					
Episode IV					
14 (93 12 08)	KRFZ, L4 and Patan	8.2 (TALWANI <i>et al.</i> , 1996)	27.3	28.5	30
15 (94 02 01)	KRFZ, L3 and Patan	8.8 (TALWANI <i>et al.</i> , 1996)	7.4	11.5	15
Episode V					
16 (00 03 12)	KRFZ and L6	5.0 (GUPTA <i>et al.</i> , 2002)	28.2	28.6	30
17 (00 04 06)	KRFZ and L6	7.8 (GUPTA <i>et al.</i> , 2002)	30.3	31.3	
Episode VI					
18 (00 09 05)	KRFZ and L5	6.9 (GUPTA <i>et al.</i> , 2002)	28.2	29.0	30
(c)					
Episode IV					
14 (93 12 08)	Lake, L4 and Patan	8.2 (TALWANI <i>et al.</i> , 1996)	15.0	17.1	15
15 (94 02 01)	L3 and Patan	8.8 (TALWANI <i>et al.</i> , 1996)	25.0	26.5	30
Episode V					
16 (00 03 12)	Lake, L4, P1 and L6	5.0 (GUPTA <i>et al.</i> , 2002)	20.0	20.6	20
17 (00 04 06)	Lake, L4, P1 and L6	7.8 (GUPTA <i>et al.</i> , 2002)	17.7	19.3	
Episode VI					
18 (00 09 05)	Lake, L4, P1 and L5	6.9 (GUPTA <i>et al.</i> , 2002)	16.0	17.4	15

was calculated for three values of  $c$ , i.e. 1, 5 and  $10 \text{ m}^2/\text{s}$ .

### 6.3. Pore Pressure Changes Associated with M 5 Earthquakes

To obtain realistic values of diffused pore pressure we incorporated in our calculations how M 5 earthquakes affect the pore pressure field. MUIR-WOOD and KING (1993) found that the character of the hydrological changes which follow major earthquakes depend on the style of faulting. Earthquakes associated with normal faulting expel sizeable amounts of water however earthquakes associated with reverse faulting do not. Their coseismic strain model explains the differences between the hydrological signatures of normal and reverse fault earthquakes. For normal faulting, cracks open and the effective porosity increases during the interseismic period; at the time

of an earthquake, cracks close and water is expelled. We interpret that, after the normal faulting earthquake, the pore pressure values are lower than those before the earthquake, there is a leakage (decrease) of pore pressure during normal faulting. For reverse faulting—according to their model—cracks close and water is expelled during the interseismic period; at the time of an earthquake, cracks open and water is drawn in. We interpret that, after the reverse faulting earthquake, the pore pressure values are higher than those before the earthquake. For strike-slip faulting we have a mixture of the two. Pore pressure values after an earthquake can drop (normal faulting) or increase (reverse faulting) depending on the style of faulting. As normal faulting is the dominant focal mechanism in the Koyna–Warna region (except for the main shock in December, 1967), we expect that there is at least some release of build-up pore pressures after each event. In calculating the pore

pressure increase following each episode, we considered that a) there is no loss of pore pressure and that it continuously builds-up after impoundment and b) all the built-up pore pressure is released (complete leakage) during each episode, and starts to increase anew in response to the total water column in the reservoir. These two extreme cases comprise any other possibility, where there is a partial release of pore pressures. In the absence of reverse faulting mechanism for any M 5 earthquake, we do not consider any pore pressure increase after an episode.

#### 6.4. Observed Duration Between Episodes

From the Coulomb failure criterion assuming no cohesion, a new episode can occur when the pore pressures reach a threshold or critical value ( $p_c$ ) that depends on the pre-existing stress state and strength of faults,

$$p_c = \sigma_n - \tau/\mu, \quad (5)$$

where  $\sigma_n$  and  $\tau$  are the normal and shear stresses, and  $\mu$  is the coefficient of friction. The durations of 5–7 years between consecutive episodes (except between V and VI) suggests that there is a partial or complete leakage of pore pressure after each episode, and that this duration reflects the time needed to raise the pore pressure again to seismogenic levels. The following calculation illustrates how the cumulative effect of several years of annual lake-level fluctuations can result in a diffused pore pressure increase to critical levels. From Fig. 1a and b we note that the annual lake-level changes in the two reservoirs vary from 20 to 45 m. These fluctuations occur above a ‘base’ water level of about 20–30 m in the two reservoirs. Accordingly the contributions of the water levels in the reservoirs can be divided into two parts. The first is the contribution of a constant head of water (20–30 m) above the pre-impoundment level, and the second is due to the annual lake-level fluctuations above that level. The contribution of a constant head of water above pre-impoundment levels can be calculated using Eq. (4). For  $c = 1 \text{ m}^2/\text{s}$  and at a 5-km distance, the diffused pore pressures reach  $\sim 80\%$  of the load 6 years after impoundment (Fig. 4a). For  $\sim 20\text{--}30 \text{ m}$  of water, corresponding to a load of 0.2–0.3 MPa at the surface, the pore pressure increase

is 0.16–0.24 MPa after 6 years. Using Eq. (4) we evaluate pore pressure changes due to diffusion associated with six annual filling-emptying cycles, at a distance of 5 km from a reservoir, assuming  $c = 1 \text{ m}^2/\text{s}$ . We note that the maxima in pore pressures lag behind the annual maxima in lake levels, and that the pore pressures increase from about 35% of the load after the first year, to  $\sim 55\%$  after 6 years (Fig. 4b). For an annual fluctuation of the Koyna Reservoir between  $\sim 30\text{--}40 \text{ m}$ , excluding the small stress release associated with M 4 and smaller events, the excess pore pressure at 5 km from the reservoir by diffusion alone would be  $\sim 0.17\text{--}0.22 \text{ MPa}$  after 6 years. In summary, the two contributions to the pore pressures at hypocentral depths are of the same order ( $\sim 0.15\text{--}0.2 \text{ MPa}$ ). Both effects need to be taken into consideration when attempting to calculate diffused pore pressures generated after each episode.

#### 6.5. Combined Effects of the Two Reservoirs

Annual seismicity maps of the KWR (TALWANI *et al.*, 1996; TALWANI, 1997b), depict a migration of seismic activity from the Koyna Reservoir towards the Warna River region from 1967 to 1983 and then a decrease of activity near the Warna River in 1984. This observation suggests that pore pressures generated by Koyna from 1967 to 1983 caused earthquakes along the weakest faults of the Warna region. Pore pressures from Koyna were not sufficient to destabilize the stronger faults (as per the Coulomb failure criterion) and seismicity decreased dramatically. After the impoundment of the Warna Reservoir, diffused pore pressures began to increase again in the region, amounting to the pore pressures generated by Koyna. The sudden increase of seismicity in 1993 suggests that the combined diffused pore pressure (from Koyna and Warna) was able to destabilize stronger faults in the KWR. Episode IV consists of earthquakes #14 and #15 which occurred in December 1993 and February 1994 (Table 1a and b). While earthquake #14 occurred near the Warna Reservoir, earthquake #15 occurred near Koyna Reservoir (Fig. 2). Interestingly, earthquake #15 was the first M 5 earthquake to occur near Koyna after 1973 (Episode II). This observation suggests that it required the combined effect of the Koyna and

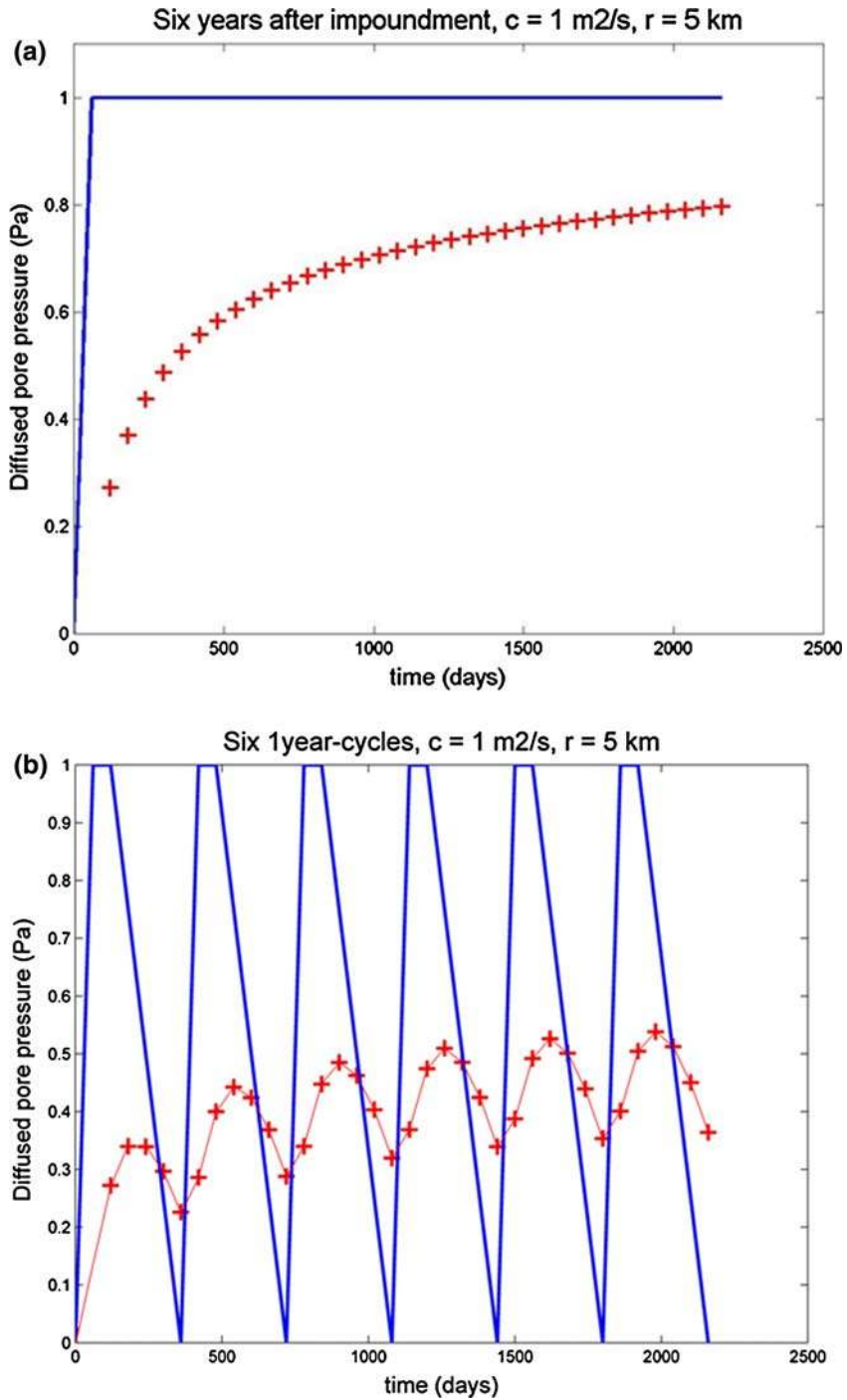


Figure 4

**a** Effect of maintaining a constant level of water above pre-impoundment levels, calculated for a 6-year period. The pore pressures (shown in *red* crosses) have been normalized with respect to the lake levels. **b** Effect of six loading and unloading 1 year-cycles (*blue*) on the diffused pore pressure (*red*). The pore pressures have been normalized with respect to the lake levels

Warna Reservoirs to cause the occurrence of earthquake #15. If we consider that the Patan fault, where earthquake #15 occurred, is stronger than the surrounding faults near Koyna, we can understand why pore pressures from Koyna alone were unable to destabilize the fault. In 1994, 9 years after the impoundment of Warna, the combined effect of both reservoirs provided adequate pore pressure to destabilize the fault and generate earthquake #15. Possible explanations for the inference of a stronger fault include differences in the rheological properties and the orientation of the fault with respect to the regional stress field. We also suggest that the presence of fluids and changing pore pressures could have facilitated elastic–plastic deformation with strain hardening in the fault. In such case, the strain hardening provided a strengthening mechanism for rocks in the KWR, which would explain the need for increased time and increased pore pressures to destabilize the causative faults.

## 6.6. Results

We calculate the pore pressure and  $dp/dt$  histories for different distances from the assumed time of start of pore pressure accumulation ( $t_0$ ) to the time of the episode ( $t_E$ ). We present two curves for episodes occurring after the initial impoundment, Episode I for the Koyna Reservoir and Episode IV for the Warna Reservoir. The first curve assumes no leakage of pore pressure after an episode, while the second one assumes complete leakage. For the former,  $t_0$  is the start of impoundment in January 1961 for Koyna

Reservoir and June 1985 for the Warna Reservoir. For the latter, pore pressures are calculated at the end of the previous episode (I, II, IV and V). For each case, we present the corresponding lake-level data between  $t_0$  and  $t_E$ , as well as the pore pressure and  $dp/dt$  histories at various distances for the different values of the hydraulic diffusivity  $c$ . The representative distances,  $r$ , for each episode are given in Table 2a–c.

In Tables 3, 4, and 5, we present the calculated pore pressure at time  $t_E$  at the chosen distances for three values of  $c$  (1, 5 and 10  $m^2/s$ ). For example, the pore pressure due to diffusion for an assumed  $c = 1 m^2/s$  at distances of 5 and 10 km for Episode I (December, 1967) are 450 and 320 kPa (Fig. 5 and Table 3).

For episodes after the impoundment of the Warna Reservoir, we add the pore pressure due to diffusion at the hypocentral distances to both Koyna and Warna Reservoirs. Next we present our selected values of  $r$  and  $c$  for each episode and the calculated pore pressures due to diffusion.

### 6.6.1 Excess Pore Pressures Associated with Episodes of $M 5$ Earthquakes in the KWR

Episode I, consisting of 10 earthquakes, took place during 1967 and 1968 (Table 1a and b). As the depth of the foreshock (event #1) is not known, we calculate the pore pressure corresponding to the location and time of the  $M 6.3$  main shock in December 1967 (#2). To account for any uncertainties in the hypocentral distance  $r$  (estimated at

Table 3

Calculated values of pore pressure,  $p$ , for Episodes I, II and III, associated with lake level changes in the Koyna Reservoir

	Episode I		Episode II				Episode III	
	$p$ from start of impoundment (kPa)		$p$ from start of impoundment (kPa)		$p$ after Episode I (kPa)		$p$ from start of impoundment (kPa)	$p$ after Episode II (kPa)
	$p$ ( $r = 5$ km)	$p$ ( $r = 10$ km)	$p$ ( $r = 5$ km)	$p$ ( $r = 10$ km)	$p$ ( $r = 5$ km)	$p$ ( $r = 10$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 30$ km)
$c = 1 m^2/s$	<b>450</b>	<b>320</b>	410	340	380	270	200	90
$c = 5 m^2/s$	<b>560</b>	<b>470</b>	<b>540</b>	<b>430</b>	<b>530</b>	<b>400</b>	<b>380</b>	<b>290</b>
$c = 10 m^2/s$	590	520	580	490	580	470	<b>420</b>	<b>360</b>

Table 4

Calculated values of pore pressure,  $p$ , for Episodes IV, V and VI, associated with lake level changes in the Koyna Reservoir

Episode IV				Episode V		Episode VI		
$p$ from start of impoundment (kPa)		$p$ after Ep. III (kPa)		$p$ from start of impoundment (kPa)	$p$ after Ep. IV (kPa)	$p$ from start of impoundment (kPa)	$p$ after Ep. V (kPa)	
$p$ ( $r = 15$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 15$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 30$ km)	
KOYNA								
$c = 1$ m <sup>2</sup> /s	410	280	340	170	300	70	300	0.026
$c = 5$ m <sup>2</sup> /s	<b>510</b>	<b>420</b>	<b>480</b>	<b>360</b>	<b>450</b>	<b>290</b>	<b>450</b>	<b>4</b>
$c = 10$ m <sup>2</sup> /s	<b>540</b>	<b>460</b>	<b>530</b>	<b>420</b>	<b>490</b>	<b>380</b>	<b>490</b>	<b>30</b>

Table 5

Calculated values of pore pressure,  $p$ , for Episodes IV, V and VI, associated with lake level changes in the Warna Reservoir

Episode IV		Episode V		Episode VI		
$p$ from start of impoundment (kPa)		$p$ from start of impoundment (kPa)	$p$ after Ep. IV (kPa)	$p$ from start of impoundment (kPa)	$p$ after Ep. V (kPa)	
$p$ ( $r = 15$ km)	$p$ ( $r = 30$ km)	$p$ ( $r = 20$ km)	$p$ ( $r = 20$ km)	$p$ ( $r = 15$ km)	$p$ ( $r = 15$ km)	
WARNA						
$c = 1$ m <sup>2</sup> /s	160	50	210	140	280	1
$c = 5$ m <sup>2</sup> /s	<b>290</b>	<b>180</b>	<b>370</b>	<b>330</b>	<b>380</b>	<b>70</b>
$c = 10$ m <sup>2</sup> /s	<b>350</b>	<b>240</b>	<b>420</b>	<b>390</b>	<b>400</b>	<b>140</b>

$\sim 10$  km, Table 2a) and hydraulic diffusivity  $c$  (estimated at 1.7 m<sup>2</sup>/s; TALWANI, 1981; TALWANI *et al.*, 2007), we calculate the diffused pore pressure at distances of 5 and 10 km, and for hydraulic diffusivities of 1 and 5 m<sup>2</sup>/s (Table 3, Figs. 5 and 6). Note that Fig. 6 covers the period from initial impoundment to the time of Episode I. The largest value of pore pressure, 560 kPa, corresponds to  $r = 5$  km and  $c = 5$  m<sup>2</sup>/s, whereas the smallest value, 320 kPa, corresponds to  $r = 10$  km and  $c = 1$  m<sup>2</sup>/s (Table 3). We treat this range, 320–560 kPa, as a plausible range of excess pore pressure required to induce Episode I.

For subsequent episodes, we consider two scenarios. In the first, there is no leakage of build-up pore pressure after an episode; in the second, there is complete leakage after an episode and the build up pore pressure starts new.

For Episode II, the selected hypocentral distances are 5 and 10 km, while the hydraulic diffusivity is 5 m<sup>2</sup>/s in view of the increased hydraulic diffusivities

(TALWANI, 1981; TALWANI *et al.*, 2007) immediately after the 1967 main shock, and  $\sim 1$  m<sup>2</sup>/s (TALWANI, 1981; TALWANI *et al.*, 2007) immediately preceding the 1973 earthquake (Episode II). For hypocentral distances of 5 and 10 km, assuming  $c = 5$  m<sup>2</sup>/s and no leakage of pore pressure, we obtain diffused pore pressures of 540 and 430 kPa (Table 3, Fig. 7, vertical yellow line). In the case of complete leakage of pore pressure after Episode I (Fig. 8), the time for the start of building up a pore pressure is immediately after Episode I, and the duration of pore pressure increase is from 1968 to 1973. For the same range of values of  $r$  and  $c$ , the corresponding pore pressures are 530 and 400 kPa. Thus we find the plausible range of excess pore pressure between 400 kPa (with complete leakage of pore pressure after Episode I, and a hypocentral distance of 10 km) and 540 kPa (for no leakage and a hypocentral distance of 5 km).

Episode III occurred 7 years after Episode II, 30 km away from the Koyna Reservoir (Table 2a). Estimates of the hydraulic diffusivity in the region

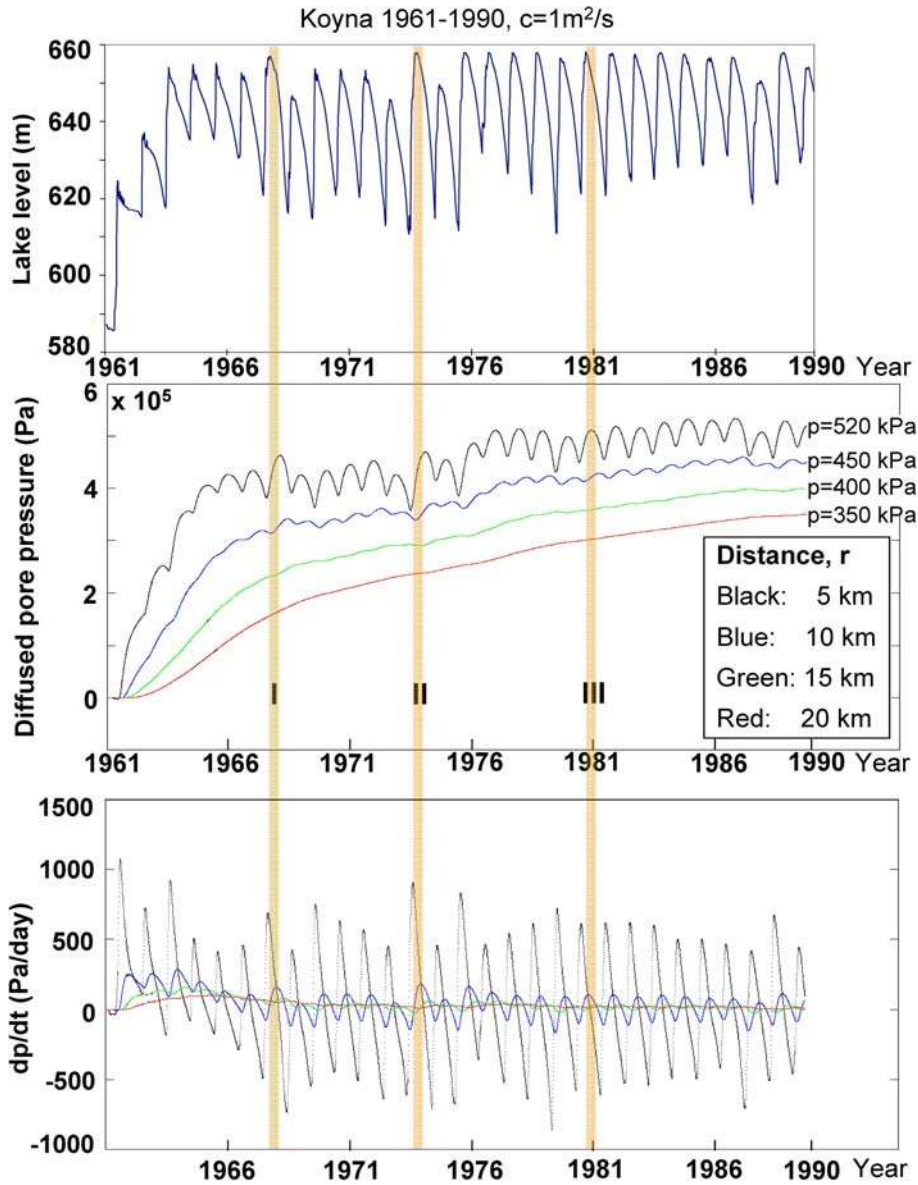


Figure 5

The top panel shows the water levels in Koyna Reservoir from the beginning of impoundment in 1961–1990. The middle panel shows the calculated build-up of diffused pore pressure for  $c = 1 \text{ m}^2/\text{s}$  at different distances (5, 10, 15, 20 km), and the times of Episodes I, II and III. The accumulated pore pressures on 1/1/1990 for various distances are given for each curve in kPa. The bottom panel shows the daily rate of pore pressure change in Pa/day

suggest values of  $c$  between 5 and  $10 \text{ m}^2/\text{s}$  (RASTOGI *et al.*, 1997; TALWANI *et al.*, 2007). For this range of  $c$  and a 30 km hypocentral distance, we obtain pore pressure values ranging from 380 to 420 kPa for the case of no leakage, and 290 to 360 kPa for the case of complete leakage (Table 3, Figs. 7 and 9). Thus for Episode III, the plausible range of excess pore

pressure by diffusion ranges between 290 kPa and 420 kPa.

For Episodes IV, V and VI, we add the combined build-up of pore pressure at hypocentral locations due to the water level fluctuations in both reservoirs. Estimates of the hydraulic diffusivity from July to August 1993 in the area close to the Warna Reservoir



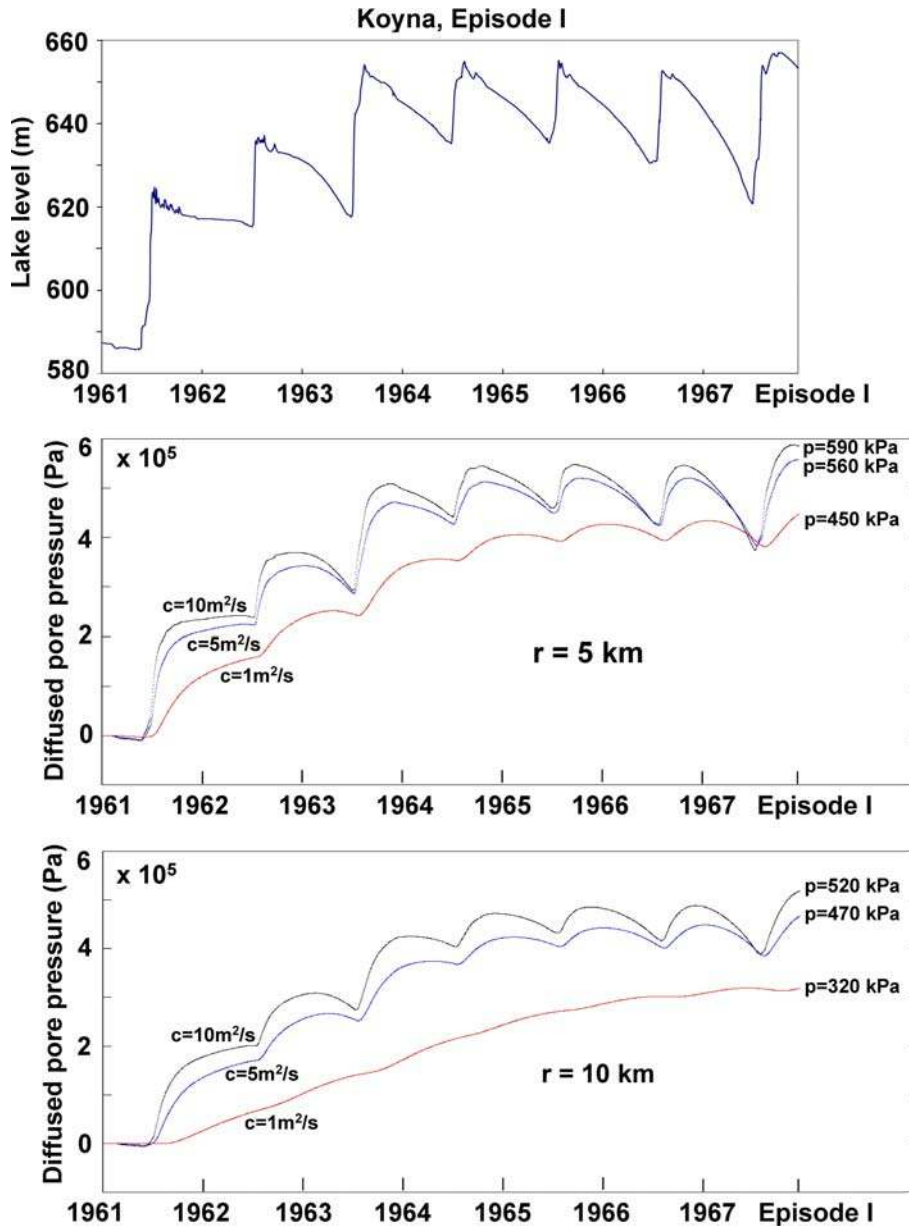


Figure 6

Build-up of diffused pore pressures from beginning of impoundment of the Koyna Reservoir to the time of the main shock in 1967 (earthquake #2) at distances of 5 and 10 km for different values of  $c$  (middle and bottom panels). The pore pressures are compared with the daily water level (top panel)

(RASTOGI *et al.*, 1997) suggest  $c = 6.9 \text{ m}^2/\text{s}$ . Correspondingly, we selected two values of  $c$  to estimate pore pressures, 5 and  $10 \text{ m}^2/\text{s}$ . Figures 10 and 11 show the diffused pore pressure histories at 10, 15 and 20 km for  $c = 5$  and  $10 \text{ m}^2/\text{s}$  assuming no leakage. The first part of Figs. 10 and 11 has been replotted for  $r = 15 \text{ km}$ —corresponding to earthquake

#14 in Episode IV—and different values of  $c$  (Fig. 12).

Earthquake #14 (Episode IV) occurred  $\sim 15 \text{ km}$  away from Warna (Table 2c) and  $\sim 30 \text{ km}$  away from Koyna (Table 2b). Pore pressure values due to Koyna at the hypocentral location range from 360 kPa (with complete leakage of pore pressure

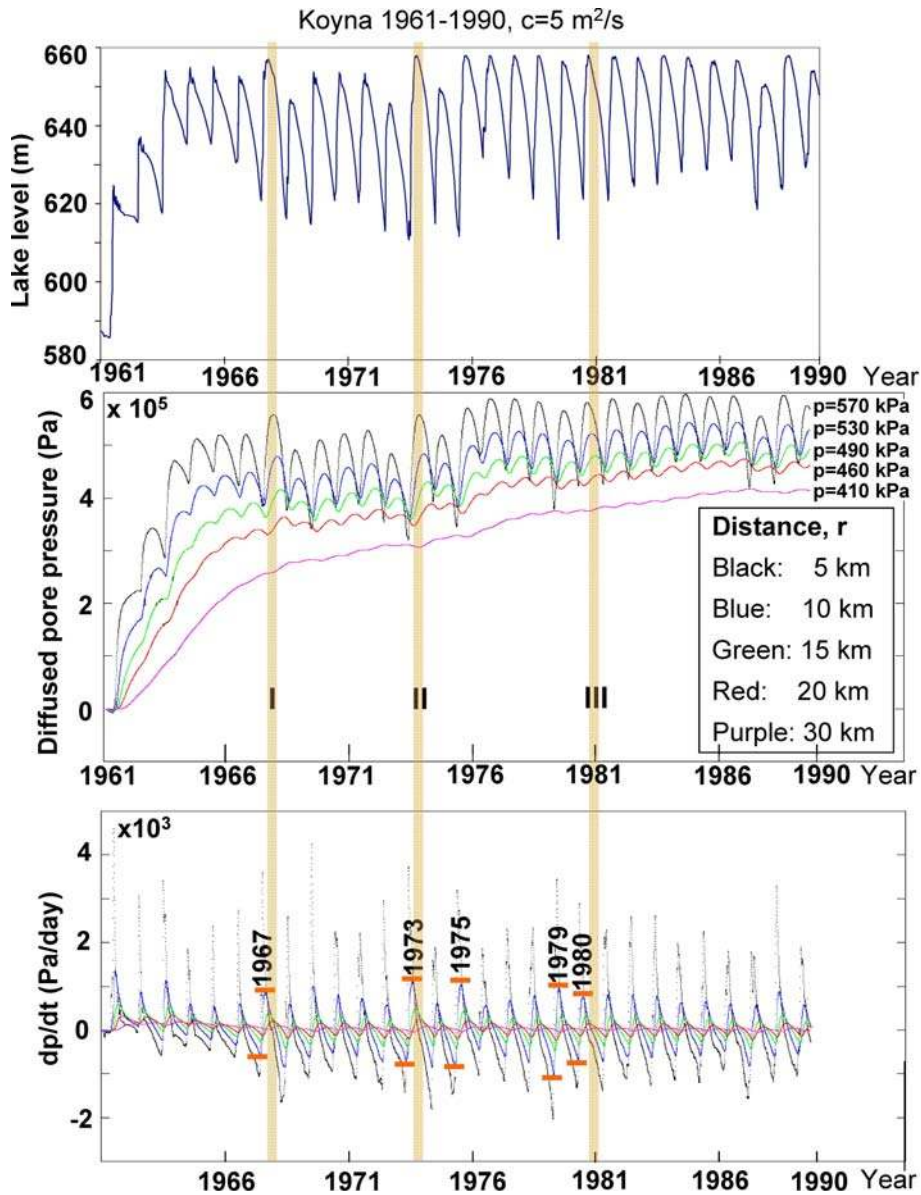


Figure 7

The *top panel* shows the water levels in the Koyna Reservoir from the beginning of impoundment in 1961–1990. The *middle panel* shows the calculated build-up of diffused pore pressure for  $c = 5 \text{ m}^2/\text{s}$  at different distances (5, 10, 15, 20, 30 km), and the times of Episodes I, II and III. The accumulated pore pressures on 1/1/1990 for various distances are given for each curve in kPa. The *bottom panel* shows the daily rate of pore pressure change in Pa/day. In the curve showing  $dp/dt$ , the horizontal ticks define the amplitude of annual  $dp/dt$  increase ( $A$ ) for the 1973 cycle (Pa/day) for  $c = 5 \text{ m}^2/\text{s}$  and  $r = 10 \text{ km}$ , i.e. 1900 Pa/day.  $A$  is listed in Table 6 for various filling cycles

during Episode III and  $c = 5 \text{ m}^2/\text{s}$ ; Table 4) to 460 kPa (for no leakage of pore pressure and  $c = 10 \text{ m}^2/\text{s}$ ; Table 4). Pore pressure values due to the Warna Reservoir at the hypocentral location range from 290 kPa (for  $c = 5 \text{ m}^2/\text{s}$ ; Table 5; Fig. 12) to 350 kPa (for  $c = 10 \text{ m}^2/\text{s}$ ; Table 5;

Fig. 11). Therefore, adding the effects of the two reservoirs, the plausible excess pore pressure values could vary between 650 kPa and 810 kPa.

Earthquake #15 (Episode IV) took place  $\sim 15 \text{ km}$  away from the Koyna Reservoir (Table 2b) and  $\sim 30 \text{ km}$  away from Warna Reservoir (Table 2c).

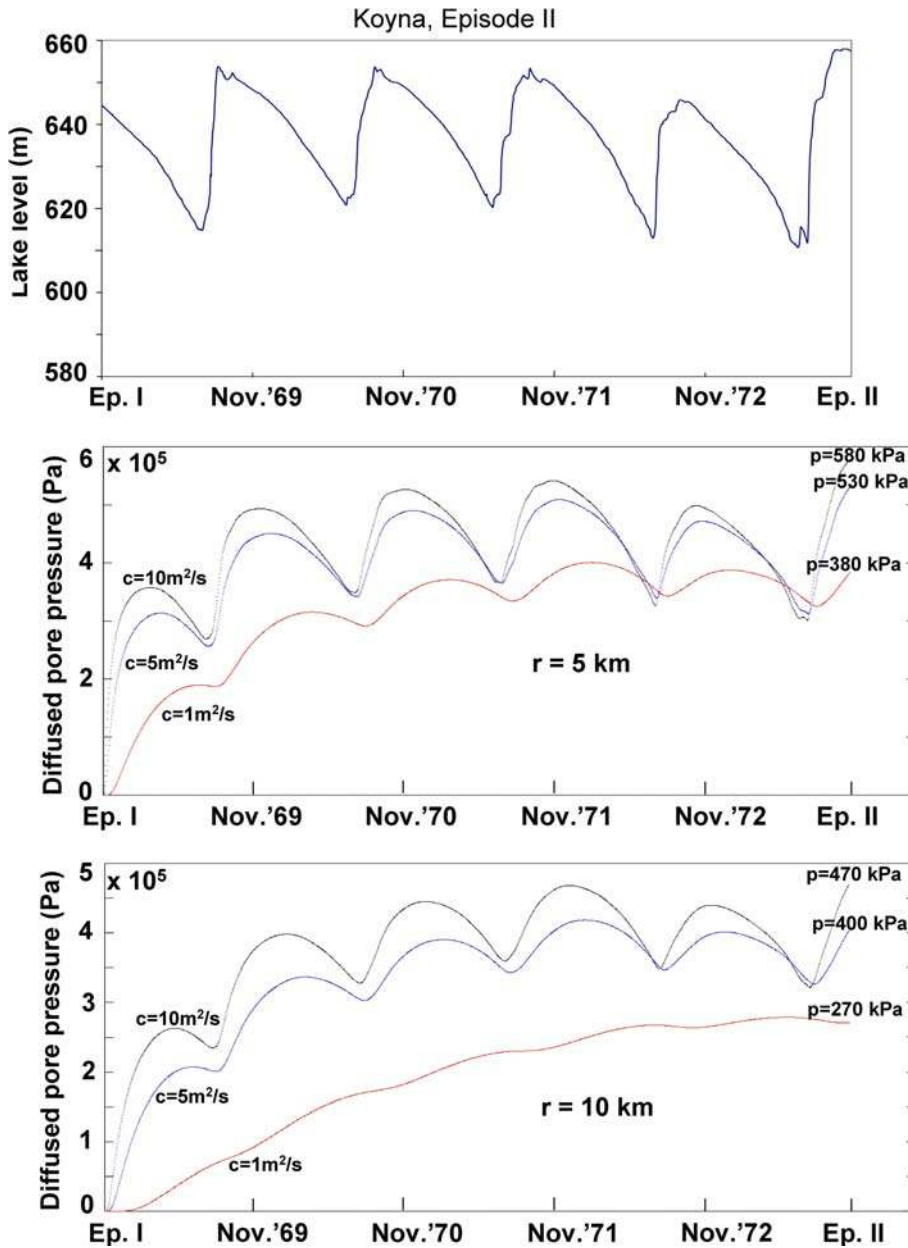


Figure 8

Build-up of diffused pore pressure from the Koyna Reservoir at a distances of 5 and 10 km, assuming complete leakage of pore pressure during Episode I. Pore pressures are calculated for different values of  $c$  after the end of Episode I (earthquake #10) up to the beginning of Episode II (earthquake #11)

Pore pressure values generated by the Koyna Reservoir at these distances and time of the earthquake vary from 480 kPa (with complete leakage of pore pressure during Episode III and  $c = 5 \text{ m}^2/\text{s}$ ; Table 4) and 540 kPa (for no leakage of pore pressure and  $c = 10 \text{ m}^2/\text{s}$ ; Table 4). For this earthquake, pore

pressure values due to the Warna Reservoir range from 180 kPa (for  $c = 5 \text{ m}^2/\text{s}$ ; Table 5) to 240 kPa (for  $c = 10 \text{ m}^2/\text{s}$ ; Table 5; Fig. 11). The combined effect of both reservoirs generated the earthquake, with plausible excess pore pressures ranging from 660 kPa to 780 kPa.

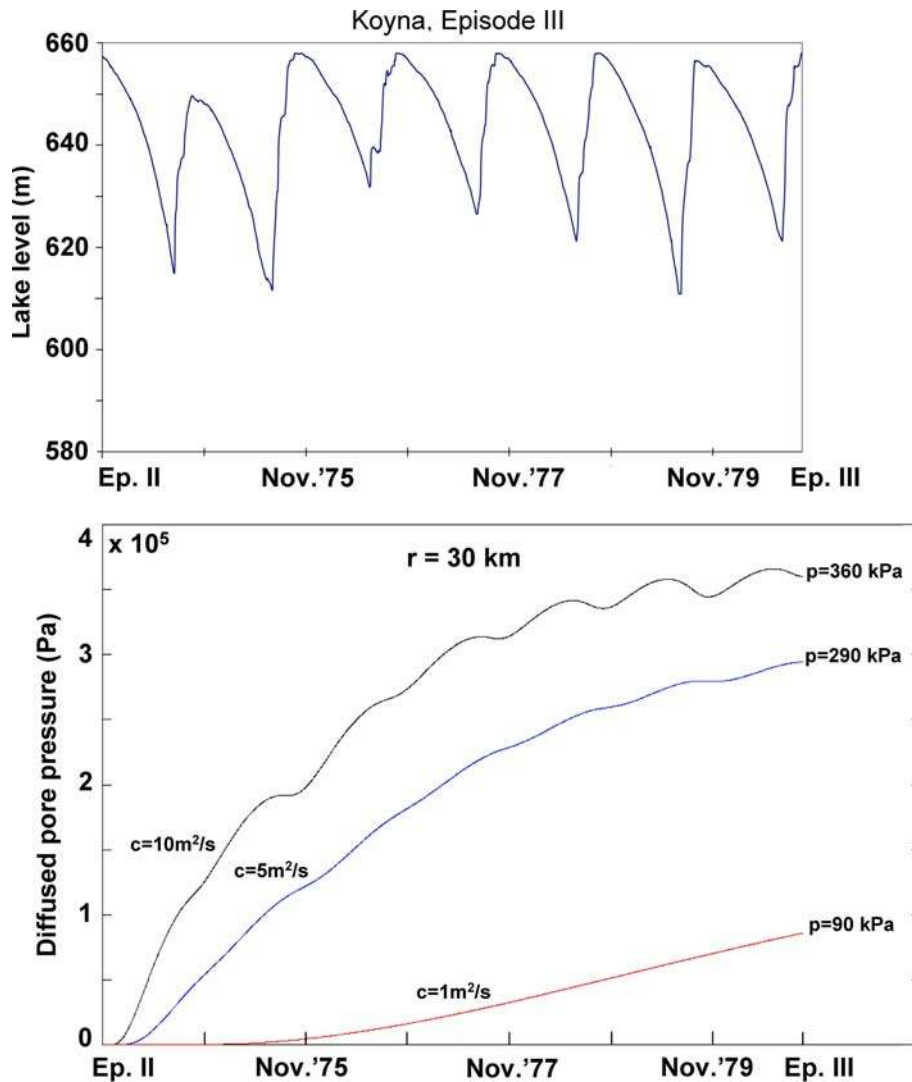


Figure 9

Build-up of diffused pore pressure from the Koyna Reservoir at a distance of 30 km, assuming complete leakage of pore pressure during Episode II. Pore pressures are calculated for different values of  $c$  from the end of Episode II (earthquake #11) to the beginning of Episode III (earthquake #12)

Episode V (earthquakes #16 and #17) occurred  $\sim 30$  km away from the Koyna Reservoir (Table 2b) and  $\sim 20$  km away from the Warna Reservoir (Table 2c). The range of pore pressures generated by the Koyna Reservoir at the hypocentral location and time of the earthquake range from 290 kPa (with complete leakage of pore pressure during Episode IV and  $c = 5 \text{ m}^2/\text{s}$ ; Table 4) and 490 kPa (for no leakage of pore pressure and  $c = 10 \text{ m}^2/\text{s}$ ; Table 4). Similarly, the range of pore pressures due to the Warna Reservoir varies from 330 kPa (with complete

leakage of pore pressure during Episode IV and  $c = 5 \text{ m}^2/\text{s}$ ; Table 5; Fig. 13) to 420 kPa (for no leakage of pore pressure and  $c = 10 \text{ m}^2/\text{s}$ ; Table 5; Fig. 11). Therefore, after combining the effect of the two reservoirs, plausible excess pore pressure values range from 620 kPa to 910 kPa.

Episode VI (earthquake #18) occurred after the monsoon rains, 5 months after Episode V, at 15 km distance from Warna Reservoir (Table 2c) and 30 km distance from the Koyna Reservoir (Table 2b). The two previous earthquakes in March and April of 2000

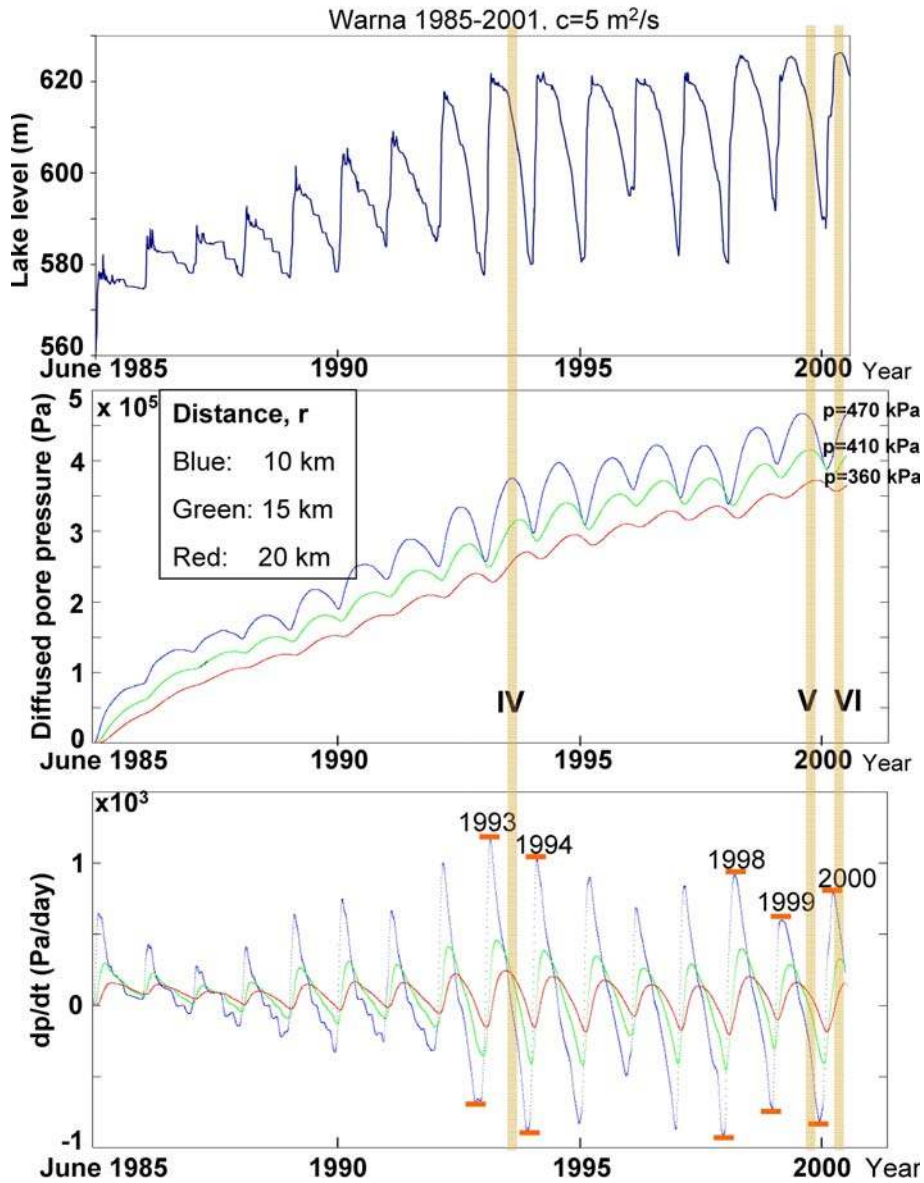


Figure 10

The *top panel* shows the water levels in the Warna Reservoir from the beginning of impoundment in 1985 to 2000. The *middle panel* shows the calculated build-up of diffused pore pressure for  $c = 5 \text{ m}^2/\text{s}$  at different distances (10, 15, 20 km), and the times of Episodes IV, V and VI. The accumulated pore pressures on 1/1/1990 for various distances are given for each curve in kPa. The *bottom panel* shows the daily rate of pore pressure change in Pa/day

were associated with normal faulting and therefore we should expect to have leakage of pore pressure in the hypocentral area associated with Episode VI. The range of pore pressures generated by the Koyna Reservoir at the hypocentral location and time of the earthquake range from 4 kPa (with complete leakage

of pore pressure during Episode V and  $c = 5 \text{ m}^2/\text{s}$ ; Table 4) to 490 kPa (for no leakage of pore pressure and  $c = 10 \text{ m}^2/\text{s}$ ; Table 4). Similarly, the range of pore pressures due to the Warna Reservoir varies from 70 kPa to 400 kPa (Table 5). The combined excess pore pressures range from 74 kPa to 890 kPa.

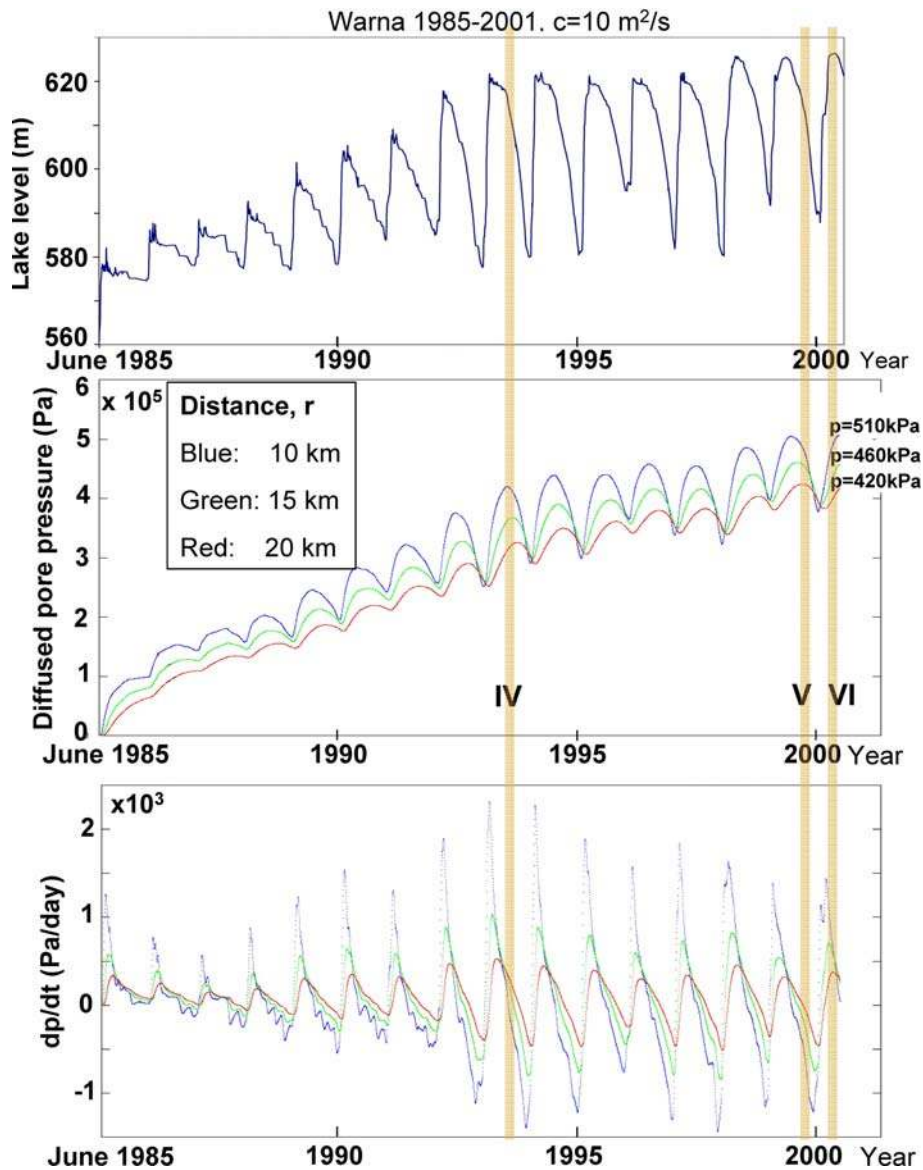


Figure 11

The *top panel* shows the water levels in the Warna Reservoir from the beginning of impoundment in 1985–2000. The *middle panel* shows the calculated build-up of diffused pore pressure for  $c = 10 \text{ m}^2/\text{s}$  at different distances (10, 15, 20 km) and the times of Episodes IV, V and VI. The accumulated pore pressures on 1/1/1990 for various distances are given for each curve in kPa. The *bottom panel* shows the rate of pore pressure change in Pa/day

This large range is not consistent with those obtained for the previous episodes, the reason being the short period to build-up pore pressures after Episode V. Bearing in mind that the interval of time required to build-up pore pressures from episode to episode has been normally 5–7 years, we do not fully understand

how Episode VI followed Episode V in such a brief amount of time. Pore pressures might not have dropped as expected during the two previous normal faulting earthquakes (Episode V) in the absence of fractures extending to the surface (MUIR-WOOD and KING 1993). This is the subject of future research.

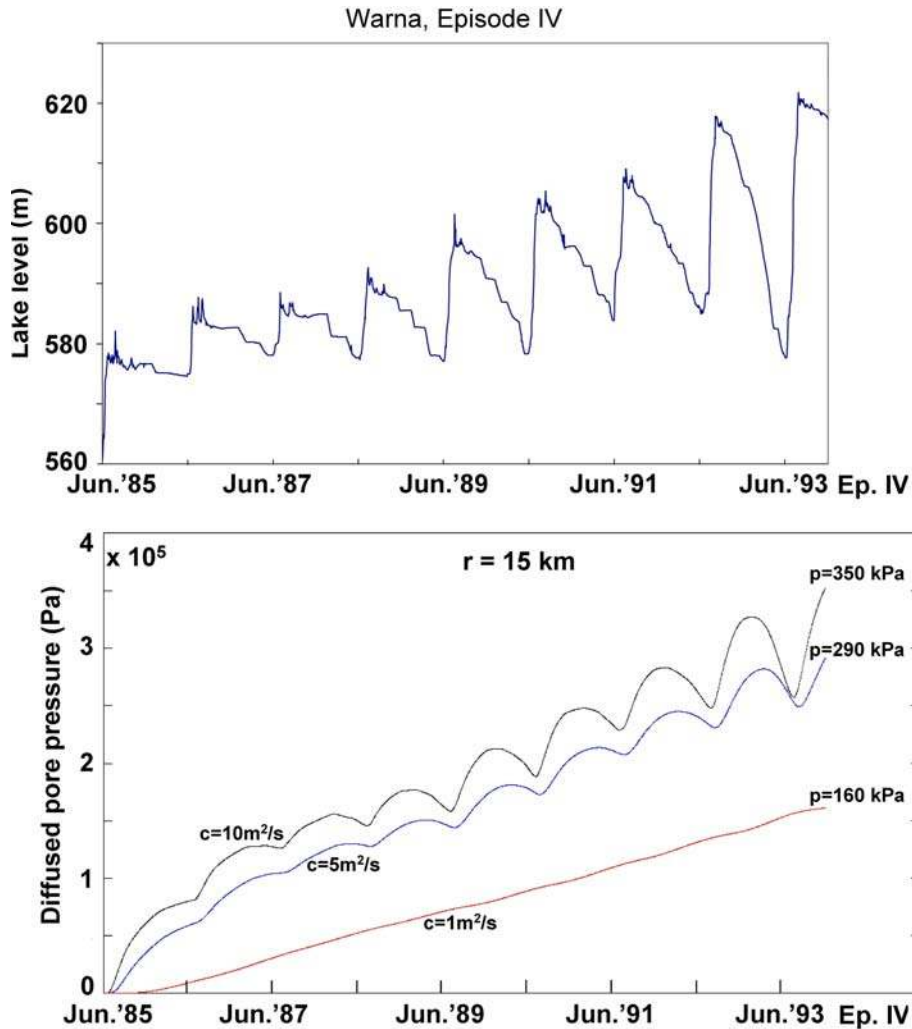


Figure 12

Build-up of diffused pore pressure from the Warna Reservoir at a distance of 15 km from the beginning of impoundment to the beginning of Episode IV (earthquake #14)

### 6.6.2 Excess Pore Pressure Associated with $M \sim 4.0$ Earthquakes in the KWR

For comparison, we calculate the required diffused pore pressure to generate an  $M 4$  earthquake in the Warna and Koyna areas.

Given the scarcity of seismic stations near Koyna in the early years of seismicity (before 1967), earthquakes were poorly located and any attempt to calculate the diffused pore pressures is subject to the error associated with the earthquake location. The first reported  $M 3.8$  earthquake near the Koyna

Reservoir occurred in 1965/11/06 (lat.  $17.4202^\circ$ , long.  $73.7623^\circ$ ) (TALWANI *et al.*, 1996) at a depth of 5 km. Considering the horizontal distance along the main fractures to be  $\sim 2.6$  km, we obtain a hypocentral distance  $r$  of  $\sim 6$  km from the Koyna dam. Assuming the hydraulic diffusivity to be  $c = 0.1 \text{ m}^2/\text{s}$  (TALWANI *et al.*, 2007) and using Eq. (4), pore pressures of the order of  $\sim 160$  kPa are needed to generate a  $M 3.8$  earthquake near Koyna.

The first  $M 4.0$  earthquake generated after the impoundment of the Warna Reservoir occurred on January 6, 1991 near Warna Reservoir at a depth of

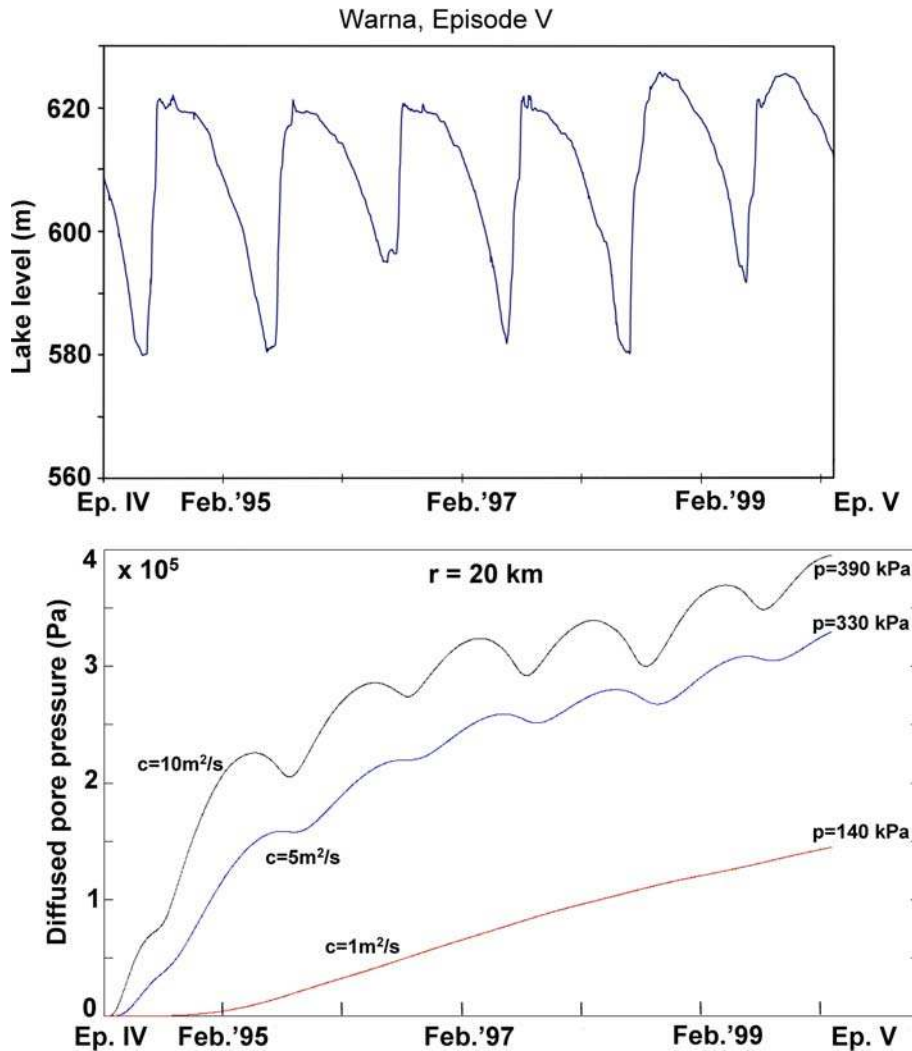


Figure 13

Build-up of diffused pore pressure from the Warna Reservoir at a distance of 20 km assuming complete leakage of pore pressure during Episode IV (earthquakes #14 and #15). Pore pressures are calculated from the end of Episode IV (earthquake #15) to the beginning of Episode V (earthquake #16)

6.3 km (lat. 17.1973°, long. 73.7355°) (TALWANI *et al.*, 1996). Following the argument given earlier, both Koyna and Warna Reservoirs contributed to the diffused pore pressure values and their combined effect caused the earthquake. From the Warna dam, the hypocentral distance is  $\sim 18$  km (the horizontal distance along the main fractures is  $\sim 17$  km, considering that pore pressure flows through the lake, L4, P1 and L5). For a hydraulic diffusivity  $c = 5 \text{ m}^2/\text{s}$  (RASTOGI *et al.*, 1997 obtained  $c = 6.9 \text{ m}^2/\text{s}$  near Warna from July to August 1993, the time at which the seismicity near Warna increased dramatically),

applying Eq. (4) we derive that Warna contributed with diffused pore pressures of the order of  $\sim 180$  kPa. From Koyna dam, the hypocentral distance is about 26 km (assuming that pore pressures flow through KRFZ and L5). For  $c = 5 \text{ m}^2/\text{s}$ , the pore pressure values from Koyna vary from 340 kPa (assuming complete leakage of pore pressure after Episode III) to 420 kPa (without leakage). Therefore, for  $c = 5 \text{ m}^2/\text{s}$ , the first M 4 earthquake associated with the combined effect of Koyna and Warna Reservoirs occurred at diffused pore pressure values of 520–600 kPa.



### 6.6.3 Amplitude of the Annual $dp/dt$ Increase

We compare the excess pore pressure (in  $10^5$  Pa or 100 s of kPa—middle panel) with its daily rate of change ( $dp/dt$ , in  $10^3$  Pa/day or kPa/day—bottom panel) at Koyna (1961–1990) and Warna Reservoirs (1985–2001) for two diffusivity values each, in Figs. 5, 7, 10 and 11, respectively. Note that the calculated excess pore pressure values reflect the cumulative effect of water level fluctuations from  $t_0$  to a given time  $t$ . The calculated  $dp/dt$  values reflect the changes in those cumulative pore pressures, and are not directly related to the lake-level changes in that cycle. We estimated the amplitude of the annual  $dp/dt$  increase,  $A$  (Fig. 7). It is those changes in  $dp/dt$  for a given cycle that we compare with values of  $\Delta H(m)$  and excess diffused pore pressures.

### 6.6.4 Comparison of the Roles of Various Factors Influencing RIS

Next, for various filling cycles, we compare those factors that have been related to RIS in Tables 6 and 7. In deciding the filling cycles for comparison, we chose those that included the occurrence of an episode, or in which the lake level exceeded the previous maximum, i.e., displayed stress memory; or the amplitude of the annual increase in water level ( $\Delta H$ ) exceeded 40 m. As we are interested in the relative values of  $A$ , for various filling cycles, we calculated it assuming  $c = 5 \text{ m}^2/\text{s}$  at a 10 km distance.

For those filling cycles associated with an episode, the excess pore pressures were calculated at the hypocentral locations and times of the given episodes as described in Sect. 6.6.1.

For those filling cycles associated with large  $\Delta H$  and  $A$ , but not accompanied by an episode, we calculated the excess pore pressure at the hypocentral location of the next known episode on December 31 of the given filling cycle (middle of the cycle). For all those filling cycles after Episode II, we calculated these pore pressures assuming both complete leakage of pore pressure following an episode and  $c = 5 \text{ m}^2/\text{s}$  and no leakage with  $c = 10 \text{ m}^2/\text{s}$ . The calculated values of the excess pore pressure define the range of possible values for that particular case. We account

for the effect of both reservoirs for filling cycles after 1985.

We summarize our findings in Table 6. For a given filling cycle (column 1), if the maximum water level exceeds the previous maximum water level from Tables 1a and 1b (column 2) and an episode follows, we consider it as evidence of stress memory (we highlight and label the corresponding cell as ‘yes’). If the annual lake-level increase ( $\Delta H$ , column 3) exceeds 40 m, we consider it anomalous and highlight the corresponding cell. If the amplitude of the annual  $dp/dt$  increase,  $A$  (column 4) is more than 1.80 kPa/day, we consider it anomalous and highlight the corresponding cell. We note the occurrence of an episode (column 5), the estimated minimum excess pore pressure, with complete leakage (column 6) and maximum pore pressure with no leakage (column 7). Next we compare these sets of parameters for the various filling cycles.

From Sect. 6.6.1 and Table 6 we notice that after the first episode which was associated with initial filling, Episodes II and III occurred when  $p_c$  was greater than  $\sim 300$  kPa, whereas the subsequent episodes occurred when  $p_c$  exceeded  $\sim 600$  kPa.

For Episode I the values of  $\Delta H(m)$  and  $A(\text{kPa}/\text{day})$  were not anomalous, but the previous water level was exceeded. The pore pressure increased to  $\sim 560$  kPa and induced a M 6.3 earthquake and its aftershocks. This behavior of the lake level and seismicity is characteristic of RIS associated with initial filling (TALWANI, 1997a). For Episode II, we note that the previous  $H_{\max}$  was exceeded,  $\Delta H$  and  $A$  were anomalously high, and the minimum induced pore pressure (assuming leakage) was  $\sim 400$  kPa. In 1975, although  $\Delta H$  and  $A$  were anomalously high, the minimum excess pore pressure  $\sim 130$  kPa was less than the values obtained for Episodes I, II and III. Only after the 1979–1980 filling cycle did the excess pore pressures at 30 km distance increase enough to cause Episode III. However, Episode III did not occur until the 1980–1981 filling cycle when the lake level exceeded the previous maximum. For the 1992–1993 filling cycle,  $\Delta H$  and  $A$  were anomalously large at Koyna and large excess pore pressures were reached without inducing an episode. Episode IV occurred the following year when  $H_{\max}$  was exceeded at Warna and  $\Delta H$  and  $A$  were anomalously high. For the 1995–

Table 6  
 Comparison of factors contributing to the generation of  $M \geq 5$  earthquakes

Year of filling cycle	Water level exceeds $H_{\max}$ : (evidence of stress memory)	$\Delta H$ (m)	A for $c = 5 \text{ m}^2/\text{s}$ at 10 km distance (kPa/day)		$M \geq 5$	$p_c$ (kPa)	Minimum value with leakage	Maximum value without leakage
			Koyna	Warna				
Koyna								
6/1967–5/1968	Yes	36.11	1.48			–	560	
6/1973–5/1974	Yes	<b>47.09</b>	<b>1.90</b>			Yes, Episode I.	400	
6/1975–5/1976	No	<b>46.18</b>	<b>1.9</b>			Yes, Episode II.	380	
6/1979–5/1980	No	<b>45.41</b>	<b>2.24</b>			No	280	
6/1980–5/1981	Yes	36.96	1.48			Yes, Episode III	420	
A for $c = 5 \text{ m}^2/\text{s}$ at 10 km distance (kPa/day)								
$M \geq 5$								
$p_c$ (kPa)								
Minimum value with leakage								
Maximum value without leakage								
Koyna + Warna								
Water level exceeds $H_{\max}$ : (evidence of stress memory)								
	Koyna	Warna	Koyna	Warna	Koyna	Warna	Minimum value with leakage	Maximum value without leakage
6/1992–5/1993	No	No	<b>43.90</b>	32.60	<b>1.98</b>	1.30	620	780
6/1993–5/1994	No	Yes	36.15	<b>44.11</b>	1.56	<b>1.85</b>	(Eq. 14) 650 (Eq. 15) 660	810
6/1994–5/1995	Yes	Yes	36.00	<b>41.88</b>	1.62	<b>1.90</b>	160	860
6/1995–5/1996	No	No	36.70	<b>40.46</b>	1.64	1.75	310	840
6/1997–5/1998	No	No	23.25	<b>40.15</b>	1.62	1.70	480	880
6/1998–5/1999	No	Yes	30.80	<b>45.04</b>	0.98	<b>1.85</b>	540	900
6/1999–5/2000	No	No	22.40	33.77	0.96	1.40	620	910
6/2000–5/2001	No	Yes	22.50	38.09	1.02	1.60	74?	890

Table 7

*Comparison of filling rates, duration of high water levels and episodes*

Filling cycle	Episode	Koyna		Warna	
		Filling rate (m/day)	Duration water level > 651 m (days)	Filling rate (m/day)	Duration water level > 615 m (days)
1967–1968	I	<b>0.95</b>	<b>173</b>		
1973–1974	II	0.52	142		
1975–1976		0.58	155		
1979–1980		0.89	153		
1980–1981	III	<b>0.76</b>	128		
1992–1993		0.73	120	0.56	52
1993–1994	IV	0.53	<b>168</b>	<b>0.83</b>	148
1994–1995		0.83	143	0.77	165
1995–1996		0.47	127	0.53	158
1997–1998		0.48	163	0.88	146
1998–1999		0.30	168	0.46	205
1999–2000	V	<b>0.82</b>	<b>190</b>	0.34	<b>215</b>
2000–2001	VI	0.23	112	0.36	

1996 and the 1996–1997 filling cycles,  $\Delta H$  was anomalously high and the minimum excess pore pressures were less than 500 kPa and there were no episodes. For the 1998–1999 filling cycle, there was evidence of exceedance of  $H_{\max}$ , large  $A$  and  $\Delta H$  at Warna, and the minimum excess pore pressure was  $\sim 540$  kPa and no earthquake occurred, suggesting that  $p_c$  had not been reached. Episode V occurred during the 1999–2000 filling cycle which showed no evidence of stress memory or anomalously high values for  $\Delta H$  and  $A$ , but for which the excess minimum pore pressure exceeded  $\sim 600$  kPa. For the 2000–2001 filling cycle, the lake level at Warna exceeded the previous maximum and there were no anomalous changes in  $\Delta H$  or  $A$ , and the minimum excess pore pressure exceeded  $\sim 74$  kPa and episode VI followed. This low value of the minimum excess pore pressure suggests that the excess pore pressure may not have completely leaked following Episode V. The exact cause of this earthquake is not understood.

In order to evaluate the role of filling rate,  $dH/dt$ , and the duration of the highest lake level in a filling cycle,  $D$  in days, in inducing RIS (see e.g., GUPTA, 2002) we examined these parameters for the filling cycles examined in Table 6. In some years the lake level begins to rise following the onset of the monsoon in June, and rises to a maximum level and then decreases; we used the dates and heights of the

lowest and the highest water levels to calculate the filling rate,  $dH/dt$ . In other years, the lake level rises following the initial rains, and then decreases and increases again to the highest level in that filling cycle; this usually happens following delayed rains in September. We used the water level attained after the initial filling to calculate  $dH/dt$ , and compare it with other years (Table 7). We counted the number of days,  $D$ , that the water level stayed above elevations of 651 m and 615 m (arbitrarily decided) for the Koyna and Warna Reservoirs. These levels are within 10 m of the maximum levels attained by the reservoirs. As we were interested in the relative roles of these factors for the different filling cycles, we arbitrarily chose  $>0.75$  m/d for the filling rate,  $dH/dt$ , and  $>160$  days for  $D$ , to define as “high” (Table 7).

In Table 7, using data for the two reservoirs, we note that four of the six episodes occurred when the filling rates were high, and that there were four instances of these high filling rates without an episode. Of the seven cycles when  $D$  was high, only three were accompanied by an episode. These observations suggest that high filling rates and longer duration of high water levels are not the primary factors in inducing an episode, but play a supporting role, if the diffused pore pressures have nearly reached threshold levels.

Next we summarize these observations of the comparative role stress memory and anomalous

values of  $\Delta H$ ,  $A$ ,  $dH/dt$  and  $D$  play in the triggering of episodes (Table 8), and discuss them in the next section.

### 7. Discussion and Conclusions

In this paper we have attempted to examine the hydromechanics related to the occurrence of six of the seven episodes of reservoir-induced M 5 events from 1967 to 2005 in the Koyna–Warna region. The seismicity is related to the increase in pore pressures to threshold levels, with the subsequent slippage along the fractures and fault planes identified in the seismotectonic framework. The increase in pore pressure at hypocentral locations large enough to induce the episodes results from changes in surface loads due to lake-level fluctuations. We assumed 1-D pore pressure diffusion from the reservoirs to hypocentral locations through a network of fractures. Using the daily lake level data, from the time of impoundment up to 2000, we calculated a history of diffused pore pressure,  $p$ , and  $dp/dt$  at hypocentral locations. For our calculations, we used a range of values of the hypocentral distance,  $r$ , and the hydraulic diffusivity of the involved fractures,  $c$ , so as to bracket the true conditions. To account for the release of accumulated pore pressures at the time of M 5 events, we calculated the pore pressure increase for the subsequent events by assuming both a complete leakage of pore pressure and a complete absence of leakage at the time to the preceding event, thus further bracketing the real situation.

The episodes occur when the excess pore pressures at the hypocentral locations attained a critical value,  $p_c$ , in accordance with the Coulomb failure criterion. In the Koyna–Warna region, the annual

cycle of reservoir's filling and emptying contributed to the diffusion of fluid pressures to tens of kms. The amplitude of lake-level fluctuations,  $\Delta H$ , and the pre-fluctuation lake levels control these excess pore pressures. For Episodes II and III, the critical excess pore pressures were found to be  $\sim 300$  kPa or more, whereas for the subsequent episodes they were  $\sim 600$  kPa or more. The need for higher pore pressures to induce Episodes IV, V and possibly VI could be because of rheological differences or unfavorable orientation of the fault planes with respect to the regional stress field.

Most of these episodes were associated with normal faulting, which suggested leakage of the excess pore pressures following these events. This assumption was found to be consistent with the observation that 5–7 years elapsed between episodes. Our calculations further support this assumption, that 5–7 years are needed to build-up the excess pore pressures to critical levels.

The role of five factors was examined in the triggering of these episodes after the excess pore pressures had reached critical levels (Table 8). We note that water level exceeded the previous maximum for five of the six episodes. For two cases where the previous  $H_{\max}$  was exceeded, no episode followed in that cycle. Additionally, for the 2003–2004 filling cycle in Warna, the lake level also exceeded the previous  $H_{\max}$  and the M 5 event did not occur until the following filling cycle (Table 1b). This observation suggests that in these cases the excess pore pressure had not reached critical values. We conclude that the exceedance of  $\Delta H$  (stress memory) plays an important but not decisive role in the triggering of these episodes.

For the cases in which  $\Delta H$  exceeded 40 m and  $A$  exceeded 1.80 kPa/day, only two were associated

Table 8

Comparison of triggering factors

	Evidence of stress memory		$\Delta H > 40$ m		$A > 1.8$ kPa/day		Filling rate $> 0.75$ m/day		Duration $> 160$ days	
	Observed		Observed		Observed		Observed		Observed	
	Yes	No	Yes	No	Yes	No	Yes	No	Yes	No
Episode	5	1	2	4	2	4	4	2	3	3
No episode	2		7		5		4		4	

with episodes. There were seven cases when  $\Delta H$  exceeded 40 m and five cases when  $A$  exceeded 1.80 kPa/day and no episode occurred. This suggests that large values of  $\Delta H$  and  $A$  contribute towards the general increase of excess pore pressures but do not induce the episodes. Also for four episodes the filling rate was  $>0.75$  m/d, and there were four occasions during which these high filling rates were not accompanied by an episode. Anomalous durations,  $D$ , were found to occur before three episodes, while on four occasions there were no episodes. Faster filling rates (see e.g., Fig. 5 in TALWANI *et al.*, 2007) and longer durations of  $D$  contribute to higher diffused pore pressures and at earlier times. We estimate that these two factors contribute a few additional kPa of pore pressure in the diffusion process. They are inadequate of themselves to induce the episodes.

The words ‘induced’ and ‘triggered’ have been used interchangeably to describe the seismicity that follows reservoir impoundment or lake-level fluctuations. Our results at KWR allow us to distinguish between them. We suggest that the seismicity results from a two-step process. The impoundment of a reservoir (in the case of initial seismicity), or large annual lake-level fluctuations (in the case of protracted seismicity) over a prolonged period of time, raises the pore pressure by  $\sim 100$  s kPa, which induces the seismicity. Short-term factors (such as the exceedance of the lake level over the previous maxima, large values of  $H$ ,  $\Delta H$ ,  $dH/dt$ ,  $D$ ,  $dp/dt$  and  $A$ ), with pore pressure changes of 10 s kPa or less, are the end products of the induction process—advancing the timing of the earthquakes and triggering them.

We did not have daily lake levels after 2000 to analyze Episode VII in 2005. However, based on the limited data, we note that the  $H_{\max}$  was exceeded in the Warna reservoir in the third filling cycle (September 2003) after Episode VI. These three cycles were inadequate to generate excess pore pressures to induce any event. It was in the next filling cycle that Episode VII occurred in 2005, 4.5 years after episode VI. Episode VII was preceded by high filling rates in both reservoirs (GUPTA *et al.*, 2005; SATYANARAYANA *et al.*, 2005).

We expect that the seismicity will expand to the southwest of the Warna Reservoir, where another M 5 earthquake might occur in the future, and whose

timing may be advanced if the current  $H_{\max}$  is exceeded.

### Acknowledgments

This paper benefited from critical comments by David Simpson, Hari Rajaram and an anonymous reviewer. We appreciate the invitation of the Editor Cezar Trifu to submit this paper, and his patience and input in its publication.

### Appendix I

The following expression (6) relates the increase of pore pressure due to compression (undrained response),  $\Delta p_u(r)$ , at a distance  $r$  from the reservoir and the change in the mean stress  $\Delta\sigma_{kk}/3$  at the given distance  $r$

$$\Delta p_u(r) = -B\Delta\sigma_{kk}/3 \quad (6)$$

where  $B$  is the Skempton’s coefficient.

An estimate of the change in the mean stress can be obtained following the procedure in AHLVIN and ULERY (1962). For a uniform circular load, the authors tabulated the vertical, radial and transverse stresses at a given depth and radial distance  $d$  from its center using elliptic functions. Considering the radius  $R$  of the circular loaded area to be 1.25 km, for an earthquake located at a depth  $z = 4$  km and a radial distance  $\rho = 2$  km (i.e.,  $r = (4^2 + 2^2)^{1/2} \sim 4.5$  km), we obtain  $\sigma_z = -0.09193p$ ,  $\sigma_\rho = -0.009830p$  and  $\sigma_\theta = 0.006325p$ , where  $p$  is the surface contact pressure. Therefore, the mean stress at a distance of 5 km is

$$\Delta\sigma_{kk}/3 \sim -0.032p. \quad (7)$$

The maximum water height in Koyna is  $\sim 70$  m, providing a surface contact pressure  $p$  of about 700 kPa

$$p = \rho gh = 1000 \times 9.8 \times 70 \sim 700 \text{ kPa}$$

and, substituting  $p$  in (7), a mean stress value of about  $-22.4$  kPa.

Assuming  $B \sim 0.7$  for crystalline rocks (TALWANI *et al.*, 1999), substituting in (6) we obtain

$$\Delta p_u(5 \text{ km}) \sim 16 \text{ kPa.}$$

The undrained response is one order of magnitude less than the diffused pore pressure values calculated in this study.

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(Received November 18, 2008, revised March 14, 2009, accepted March 26, 2009)