

Impact-Induced Tensional Failure in Rock

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Planar impact experiments were employed to induce dynamic tensile failure in Bedford limestone. Rock discs were impacted with aluminum and polymethyl methacrylate flyer plates at velocities of 10 to 25 m/s. This resulted in tensile stresses in the range of ~11 to 160 MPa. Tensile stress durations of 0.5 and 1.3 μ s induced microcrack growth which in many experiments were insufficient to cause complete spalling of the samples. Ultrasonic *P* and *S* wave velocities of recovered targets were compared to the velocities prior to impact. Velocity reduction, and by inference microcrack production, occurred in samples subjected to stresses above 35 MPa in the 1.3- μ s PMMA experiments and 60 MPa in the 0.5- μ s aluminum experiments. Apparent fracture toughnesses of 2.4 and 2.5 MPa m^{1/2} are computed for the 1.3- and 0.5- μ s experiments. These are a factor of ~2 to 6 greater than quasi-static determinations. Three-dimensional impact experiments were conducted on 20 cm-sized blocks of Bedford limestone and San Marcos gabbro. Compressional wave velocity deficits up to 50-60% were observed in the vicinity of the crater. These damage levels correspond to O'Connell and Budiansky damage parameters of 0.4 as compared to the unshocked rock. The damage decreases as $\sim r^{-1.5}$ from the crater indicating a dependence on the magnitude and duration of the tensile pulse. Using the observed variation in damage with tensile stress from the one-dimensional experiments, and estimates of the variation of peak dynamic tensile stress and tensile stress duration with distance from an impact on an elastic half-space, the observed dependence of damage with radius in the three-dimensional experiments are theoretically predicted and compare favorably to experimental data.

INTRODUCTION

Impact-induced cracking of rock beneath impact craters and in the rocks surrounding contained explosions have long been recognized both on Earth and the Moon. As early as 1968, Short [1968] reported the systematic crack-induced reduction (by as much as 42%) in the compressional wave velocity (C_p) in rocks surrounding the underground explosion, Hardhat. Similarly, large scale reductions in compressional wave velocity were reported beneath the ~1-km-diameter Barringer (Meteor) impact crater in Arizona [Ackerman *et al.*, 1975], as well as the considerably larger (35 and 20 km diameter) Ries crater in Germany [Pohl *et al.*, 1977] and Haughton dome crater in Canada [Hajnal *et al.*, 1988]. More recently, Ackerman *et al.* [1986] have reported substantial velocity deficits beneath the 1-km-diameter explosion crater (Eniwetok) produced by the 10-Mt surface explosion, Oak. Seismic refraction studies on the Moon, and virtually all the rocks returned from the Moon, indicate that shock-induced fractures reduce the observed seismic velocity from intrinsic values. Simmons *et al.* [1973] pointed out that virtually the entire crust of the Moon, down to the depth of ~25 km, suffers shock-induced crack damage, and the in situ velocity is substantially below the intrinsic values of the rocks present (e.g., gabbros, norites, and pyroxenites). Related to the crack-induced seismic velocity deficits is a series of recognized deficits in the local gravity field beneath large young craters on Earth and the Moon [Dvorak and Phillips, 1977; Ivanov, 1989]. Cracking beneath small-scale laboratory impact craters has been previously described [Hörz, 1969; Maurer and Rinehart, 1960; Moore *et al.*, 1963; Polansky and Ahrens, 1990]. However, the physical mechanisms which produce the different types of fractures have only partially been discussed

previously [e.g., Melosh 1989, chapter 5]. Another related issue which remains unstudied is the effect of large-scale gravity-induced late-stage cratering motions on rock properties. Presumably, rock cracking occurs upon crater rebound that transforms large bowl-shaped transient cavities into flat-floored and multiringed craters [O'Keefe and Ahrens, 1989; also Planetary cratering mechanics, submitted to *Journal of Geophysical Research*, 1992]. In the present paper we analyze data from two rocks, Bedford (Indiana) limestone and San Marcos, (California) gabbro, damaged in one- and three-dimensional impact experiments. These rocks are studied because they are representative of crustal rock types and impact and strength tests for specimens from these localities have been performed previously.

Our goal was to understand the relation of dynamic production of cracks beneath craters, to the concurrent reduction in elastic moduli and quasi-static strength. This relation may be important to inferring the impact or explosion parameters which produce specific observed effects. Theoretical work relating crack density and crack geometry to material modulus reduction [O'Connell and Budiansky, 1974; Walsh, 1965; Zimmerman and King, 1985] and more recent work relating crack density to material strength reduction [Ashby and Sammis, 1990; Horai and Nemat-Nasser, 1986; Sammis and Ashby, 1986] suggest that it will be possible to obtain a complete and testable theory of shock wave damage in rock. Previous work by Hadley [1976] generally confirmed the O'Connell and Budiansky [1974] theory relating modulus reduction to microcrack production in quasi-statically deformed rock. The recognition by Simmons *et al.* [1973] that the seismic velocity reduction within the upper crust of the Moon was due to impact-generated cracking is but one example of the numerous instances in which velocity reduction can be used as a measure of impact-induced damage in rock. This report describes our initial laboratory study of this phenomenon. It is the first quantitative laboratory study of elastic modulus degradation in shocked rock. First, we discuss planar impact (one-dimensional) and then axisymmetric small-scale cratering experiments on rock. Then, we investigate the relationship of the measured crack damage parameters

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observed in the planar impact experiments to the observed rock damage versus distance from the crater in the axisymmetric experiments. The following strategy is employed:

1. Cracking during the complex loading and unloading history in the axisymmetric impact experiments could be a result of both the tensile stress concentration around flaws during the compressive pulse, and the tensile portion of the stress wave. Therefore, to better understand the underlying physics of rock failure, we have performed a series of one-dimensional tensile (spall) experiments on Bedford limestone to characterize the high-strain rate tensile strength properties of this material. A summary of some of these results are recently published [Rubin and Ahrens, 1991]. Our strategy was to shock discs of limestone at controlled stress magnitudes and durations below that required to produce complete spalling and to observe the progression in rock damage. The elastic moduli of the recovered samples were determined by measuring their longitudinal and shear wave velocities. As in the three-dimensional experiments, the seismic velocity deficits can be used to infer the crack densities that were induced as a function of applied stress and stress duration.

2. Experimental studies of crack density and modulus reduction produced by hemispherical shock waves in rock were carried out by impacting blocks of Bedford limestone and San Marcos gabbro with high-velocity projectiles. Longitudinal elastic wave (P wave) velocity measurements were carried out on a large number of oriented cubes cut out of each rock sample. Theoretical estimates of seismic velocity reductions based on microscopic observations of crack density are compared to observed velocity reductions for P waves.

3. We relate the measured seismic velocity reductions in the axisymmetric experiments to the rock damage parameters of O'Connell and Budiansky [1974] and Grady and Kipp [1979, 1987]. The inferred damage parameter is then interpreted in light of estimates of the stress history of the samples using rock fracture properties derived from the planar impact experiments. We also examined the applicability of the theoretical and experimental analysis of the relation of fracture length induced by static indentation into hard brittle media outlined by [Anstis et al., 1981] to the size of the zone of dynamic fragmentation.

DYNAMIC ONE-DIMENSIONAL LOADING AND FRACTURE OF ROCK

The dynamic tensile strength of rock can exceed the quasi-static tensile strength by an order of magnitude [Rinehart, 1965]. By "dynamic tensile strength" we mean the peak transient stress sustained during high strain rate tensile failure. The difference between dynamic and quasi-static (hereafter termed static) tensile failure results from the pristine rock containing abundant inherent flaws (pores, grain boundary cracks, etc.). If a tensile strain (stress) is increased gradually, the largest well-oriented flaw grows at the expense of others (Figure 1a). A characteristic "tensile strength" implies that the material has a characteristic largest flaw size. If the strain (stress) is increased very rapidly, then a single flaw and the zone of reduced stress surrounding that flaw cannot propagate rapidly enough to prevent other smaller or poorly oriented flaws from later being activated (Figure 1b). Because both the crack growth rate and the expansion of the stress-relaxed zone are limited

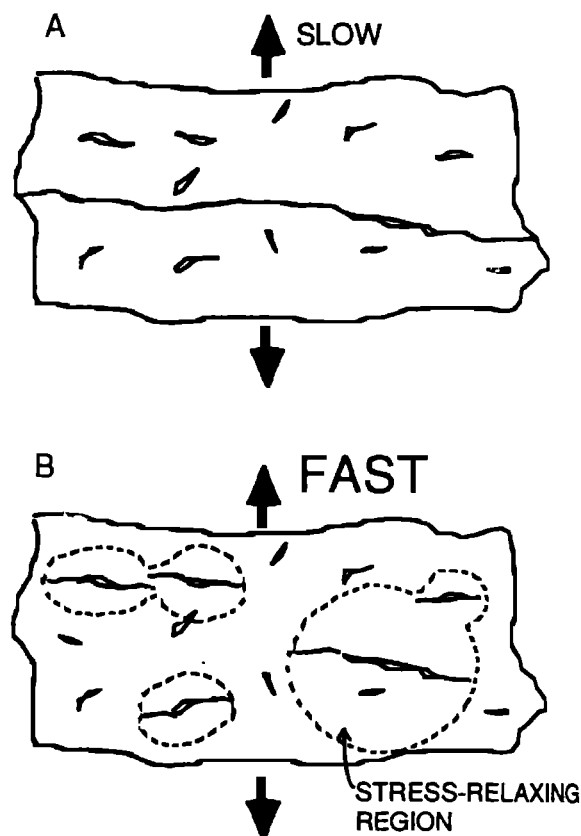


Fig. 1. Schematic diagram of tensile failure of rock under (a) quasi-static and (b) dynamic loads.

by the sound speed of the material, in general, the higher the strain rate, the smaller, at a given strain, the zone of reduced stress surrounding growing microcracks, the more flaws are activated and, ultimately, the greater the peak stress and number of fragments produced.

In current models of dynamic rock fracture [i.e., Grady and Kipp, 1987], the reduction in elastic modulus concurrent with microcrack growth is an essential element in describing the deformation history. For an elastic solid, the stress is equal to the product of the strain and the modulus. In a fragmenting solid, the stress history is determined by the competition between the increasing strain produced by the loading system, and the decreasing modulus produced by crack growth. Models incorporating this concept, combined with estimates of the initial flaw distribution from microscopy, have been used to explain the fragment size distribution and measured stress history within Arkansas novaculite in dynamic spall experiments [Grady and Kipp, 1987; Shockey et al., 1974]. However, there is ample reason to test the models further. For example, although during dynamic fragmentation the stress field is very heterogeneous, with some regions as yet unaffected by crack growth (Figure 1b), the computed stress is some average based on the effect of crack growth on the overall reduction in static modulus of the aggregate. Furthermore, although the experimental results are apparently consistent with static measurements of the rock fracture toughness [Grady and Kipp, 1987], they have yet to be reconciled with measurements of the much lower "apparent" fracture toughness of "subsize" cracks [Ingraffea, 1987], of which inherent flaws are examples.

EXPERIMENTAL PROCEDURE FOR PLANAR
IMPACT EXPERIMENTS

The purpose of the present experiments was to determine the modulus reduction of intact dynamically loaded rock samples at various stages of crack growth. This was accomplished by (1) loading the samples with tensile stress pulses of controlled magnitude and duration insufficient to cause complete spalling, and (2) measuring the subsequent reduction in *P* and *S* wave velocity due to microcracking.

Planar impact experiments used to produce dynamic tensile failure are described by *Cohn and Ahrens* [1981]. Briefly, a lexan projectile with a polymethyl methacrylate (PMMA) or aluminum (Al) flyer plate is accelerated to velocities of 10 to 25 m/s. The flyer plate impacts the target, which then flies free into a recovery tank, where loose rags prevent further damage. The target, a disc of rock 25 mm in diameter and 6.5 to 7.5 mm thick, is polished so that its front and rear surfaces are flat and parallel to ± 0.005 mm. The rock selected for this study, Bedford limestone, has been the subject of numerous rock mechanics investigations (Table 1).

Upon impact, compressive waves propagate forward into the target and back into the flyer plate. Tension is produced when these compressive waves, reflected as release waves from the free surfaces of the target and flyer plate, later meet within the sample. The magnitude of compressive stress is proportional to the impact velocity. For the PMMA experiments, separation occurs between the flyer plate and rock target, and the tension is equal in magnitude to the initial compression. For the Al experiments, a small residual compression is maintained across the flyer plate/rock interface, and the magnitude of tension is reduced by that amount (see below). The duration of the tensile pulse, if failure does not occur, is equal to the time required for the stress wave to traverse the flyer plate twice (1.3 μs for the PMMA and 0.5 μs for the Al experiments). The magnitude of the tensile stresses, σ, quoted in Table 2 were calculated using the values of density ρ and longitudinal wave velocity *C_p* from Table 1.

Following impact, the particle velocity *u_i* within the impactor can be derived from continuity of stress and displacement at the impactor/target interface [*Cohn and Ahrens*, 1981]; the result is

$$u_i = U_p \frac{\rho_i C_i}{\rho_i C_i + \rho_t C_t}$$

Here *U_p* is the projectile velocity, and the subscripts *i* and *t* refer to the impactor and target, respectively. The particle velocity within the impactor with respect to the projectile velocity, *U_i - U_p*, doubles upon initiation of the release wave from the rear of the flyer plate; the particle velocity behind the release wave front is then given by

$$u_i = -U_p \left(1 - \frac{2\rho_i C_i}{\rho_i C_i + \rho_t C_t} \right)$$

This velocity is less than zero (opposite to *U_p*) if the impedance of the impactor is less than that of the target, equal to zero if the two impedances are equal, and greater than zero if the impactor impedance is greater than that of the target. Within the target, on the other hand, relaxation of the stress at the target/flyer interface to zero behind the release wave implies that the target particle velocity returns to zero. This implies that, when the impactor impedance is less than the target impedance, separation of the target and flyer plate occurs. In this case, the subsequent tensile stress within the target is given by the acoustic formula [*Cohn and Ahrens*, 1981]

$$\sigma = U_p \frac{\rho_t C_t \rho_i C_i}{\rho_i C_i + \rho_t C_t} \tag{1a}$$

However, when *u_i* within the impactor is greater than zero following release, then separation does not occur. In this case, there is a residual compression across the interface that can be determined by treating the positive impactor particle velocity behind the release wave front as if it were a (zero pressure) projectile moving with that velocity; that is, by substituting *u_i* behind the release wave for *U_p* in equation (1a). The peak tension later reached within the sample is reduced by this amount, yielding

$$\sigma = U_p \frac{\rho_t C_t \rho_i C_i}{\rho_i C_i + \rho_t C_t} \left(2 - \frac{2\rho_i C_i}{\rho_i C_i + \rho_t C_t} \right) \tag{1b}$$

Equation (1a) is used for the PMMA flyer plate experiments and (1b) for the Al experiments.

TABLE 1. Physical Properties of Experimental Materials

Material	ρ, g/cm ³	<i>C_p</i> , km/s	<i>C_s</i> , km/s	E, GPa	σ _T , MPa
Bedford limestone (dry)	2.42(1)	4.9(1)	2.8(1)	48(1), 24(2), 32(3)	5.4(2), 0.8-1.2(4), 7.7(5)
Bedford limestone (saturated)	2.53(1)	5.1(1)			
PMMA	1.2	2.8			
Aluminum	2.78	6.36			

Sources:

(1) this study, (2) *Nur and Simmons* [1969], (3) *Schmidt* [1976], (4) *Ingraffea et al.* [1984], (5) *Ingraffea and Schmidt*, [1978].

TABLE 2. One-Dimensional Tensile Strain Impact Parameters and Preshot and Postshot Ultrasonic Compressional and Shear Velocities

Shot	Sample	Flyer Plate	Atmosphere	Projectile Velocity, m/s	Tensile Stress, MPa	Preshot		Postshot	
						C_p , km/s	C_s , km/s	C_p , km/s	C_s , km/s
1	BL #2	PMMA	air	24.0	62.4	4.88	2.91	4.34	2.69
2	BL#3	PMMA	air	16.9	43.9	4.38	2.68	4.9	2.92
3	BL#4	PMMA	air	20.6	53.6	4.55	2.76	4.09	2.58
								3.84 ^a	2.46
								3.79 ^a	2.44
4	BL#10	PMMA	air	16.3	42.4	4.25	2.74	4.22	2.68
5	BL#11	PMMA	air	22.5	53.5	4.42	2.81	3.74	2.48
								3.87	2.24
								3.87	2.54
7	BL#5	PMMA	vacuum	24.7	64.2	4.65	2.84	4.26	2.63
8	BL#7	PMMA	vacuum	26.5	58.9			^b	^b
9	BL#8	PMMA	vacuum	24.2	62.9	4.3	2.74	3.33	2.17
10	BL#9	PMMA	vacuum	22.6	58.8	4.3	2.74	4.17	2.58
11	BL101	PMMA	vacuum	12.53	32.6	4.59	2.82	4.67	2.84
12	BL102	PMMA	vacuum	22.88	59.5			^b	^b
13	BL103	PMMA	vacuum	16.61	43.2	4.71	2.77	4.36	2.55
15	BL106	PMMA	vacuum	22.14	57.6	4.70	2.78	4.33	2.43
								4.1	2.46
16	BL107	PMMA	vacuum	18.45	48.0	4.69	2.85	4.42	2.60
18	BL#15	Al	vacuum	24.6	139.5	4.17	2.71	^b	^b
19	BL#16	Al	vacuum	19.8	112.5	4.17	2.73	3.58	2.35
								3.50	2.40
20	BL#17	Al	vacuum	17.8	101.1	4.5	2.83	4.02	2.42
								3.60	2.48
21	BL#18	Al	vacuum	12.6	71.6	4.32	2.82	4.56	2.81
22	BL#19	Al	vacuum	15.0	85.2	4.35	2.84	3.79	2.58
23	BL#21	Al	vacuum	20.9	113.6	4.49	2.90	^c	^c
24	BL#22	Al	vacuum	17.2	97.7	4.62	2.82	4.40	2.67
25	BL#12	Al	air	11.19	63.6	4.39	2.82	4.24	2.73
26	BL#13	Al	air	22.2	126.1	4.34	2.79	3.29	2.20
								3.61	2.31
								3.42	2.06
27	BL#14	Al	air	25.8	146.6	4.37	2.78	3.26	1.88
28	BL#20	Al	air	19.35	109.9	4.41	2.78		
29	BL201	Al	air	19.2	109.0	4.84	2.72	4.41	2.49
								4.63	2.68
								4.38	2.47
30	BL202	Al	air	12.9	73.3	4.65	2.74	4.42	2.55
31	BL203	Al	air	15.56	88.4	4.98	2.72	4.35	2.55
32	BL211	Al	air	10.85	61.6			4.81	2.70
33	BL109	Al	vacuum	22.06	126.3	4.63	2.75	4.06	2.63
						4.63	2.74	3.90	2.50
								4.01	2.69
34	BL110	Al	vacuum	24.45	138.9	4.61	2.77	3.82	2.57
								4.03	2.65
								4.01	2.59
35	BL212	Al	air	16.07	91.3	4.69	2.80	4.72	2.70
						4.86	2.80	4.72	2.73
36	BL209	Al	air	16.20	103.3	4.66	2.77	4.60	2.76
	W ^d					4.70	2.77	4.73	2.76
37	BL207	Al	air	21.89	139.5	4.69	2.80	4.89	2.82
	W ^e					4.84	2.80	4.69	2.72
38	BL210	Al	air	10.85	69.2	4.74			
	W								
39	BL204	Al	air	13.4	85.4	4.69	2.68	4.39	2.55
	W								
40	BL205	Al	air	15.08	96.2	4.82		4.4	2.54
	W								
41	BL206	Al	air	19.34	123.3	4.94	2.72	4.26	2.32
	W ^e								
42	BL208	Al	air	25.80	164.5	4.80		4.64	2.68
	W ^e							4.77	2.67

^aMultiple entries are velocities for different fragments.

^bSample too heavily damaged via spalling to conduct postshot velocity measurements.

^cSample too damaged via radial cracks for measurements.

^dW designates water-saturated sample.

^eAluminum ring placed around sample prior to firing to prevent radial cracking. This ring apparently precompressed sample and inhibited cracking; hence data are not shown in Figures 2 and 4.

Several assumptions are made in computing the magnitude and duration of the tensile pulse. (1) Because rocks are generally much stronger in compression than tension, no damage occurs during the initial compressive pulse. (2) The P wave velocity is constant over the pressure range involved (several tens of megapascals). (3) The rise time of the stress pulse is much shorter than its duration. (4) The strain can be considered to be one-dimensional. (5) Negligible crack growth occurs during later reverberations of the stress waves. These assumptions are discussed further below.

In order to determine the effect of pore water on dynamic fracture behavior, four shots were performed on water-saturated samples. The samples were evacuated overnight, and water was siphoned into the container before the samples were exposed to atmosphere. The front and rear surfaces of the targets were covered with a cellophane tape <0.02 mm thick to prevent evaporation. The perimeters of the targets were likewise covered, except for a several mm gap at the top where a large drop of water was held by surface tension as they were mounted and shot. Approximately 5-7 min were required to mount, align, and shoot the targets. Evaporative weight loss within the sample in this time was determined to be less than 1% of the total water content without the buffering water droplet. The sound speed measurements on these saturated samples made prior to and following impact were performed dry. Four shots were also performed on dry, taped targets, which demonstrated no detectable effect due to the tape alone.

The Bedford limestone specimens used have a typical grain size of 0.5 mm and a porosity of ~ 9.9% (Table 1). Ultrasonic velocities (reproducible to < 2%) were measured using the travel-time method with 1 MHz PZT transducers. The dry velocities are higher than those of Nur and Simmons [1969], and the difference between the wet and dry velocity is less. This suggests a lower pore volume in the form of high aspect ratio cracks [Toksoz et al., 1976]. Table 1 also gives previous determinations of the static tensile strength σ , fracture toughness K_{Ic} , and Young's modulus E.

RESULTS FOR PLANE IMPACT EXPERIMENTS

The results of the ultrasonic velocity measurements are shown in Table 2 and Figure 2. Both longitudinal and shear wave velocities in the direction perpendicular to the impact surface were measured, and compared to the preshot velocities. Two typical samples are shown in Figure 3. Important points are as follows:

1. Comparable levels of rock damage occur at considerably lower stresses in the longer-duration experiments. The onset of microcrack growth, as determined by a detectable reduction in sound velocity, occurs at computed stresses of ~ 60 and ~35 MPa in the 0.5- and 1.3- μ s experiments. Complete spalling occurs above computed stresses of ~135 and 60 MPa in the 0.5- and 1.3- μ s experiments.

2. Macroscopic radial cracks were produced above computed stresses of ~100 and 45 MPa in the 0.5- and 1.3- μ s experiments, indicating that strain was not completely one-dimensional as assumed. Such radial cracks commonly plague planar-impact recovery experiments [Shockey et al., 1974].

3. Comparable velocity reductions occur for both longitudinal and shear waves.

4. The results for the four water-saturated targets are indistinguishable from those for the dry targets. (The experiments which used aluminum constraining rings are not considered in this comparison.)

In order to relate the velocity deficits observed in the one-dimensional experiments to three-dimensional impact experiments, it is useful to utilize the damage parameters, D_p and D_s (Figure 4) which are defined as

$$D_p = 1 - (C_p/C_{p0})^2 \tag{2a}$$

$$D_s = 1 - (C_s/C_{s0})^2 \tag{2b}$$

where C_p and C_s are longitudinal and shear wave velocity, and the subscript zero indicates the undamaged value. As is evident in both Figures 2 and 4, for a given tensile stress, a range of damage is demonstrated by different samples.

Interpretation of the Velocity Reduction Data

The P wave velocity of the starting material was 75% of that reported by Christensen [1982] for an aggregate of calcite crystals. Assuming a random distribution of penny-shaped cracks, this implies an initial crack density, ϵ , of ~ 0.2, where crack density is expressed as

$$\epsilon = N \langle a^3 \rangle \tag{3}$$

where N is the number of cracks per unit volume and $\langle a^3 \rangle$ is the average of the cube of the crack radii [O'Connell and Budiansky, 1974]. A 2% reduction in P wave velocity corresponds to an increase in crack density of 0.016, or about a 3% increase in the radii of the largest cracks present, so this growth represents the minimum that could be detected by the present method.

Onset of Tensile Cracking

Following Grady and Kipp [1987], we use a simple model for the time-dependent stress intensity factor at the tips of inherent flaws to compute an apparent fracture toughness appropriate for the dynamic loading conditions of the experiments. The assumption is that crack growth occurs when the crack tip stress intensity factor K_I equals the rock fracture toughness K_{Ic} . Under a static applied tension σ , for an isolated penny-shaped crack of radius a,

$$K_I = 2\sigma\sqrt{a/\pi} \tag{4a}$$

Under dynamic loading conditions the form of the equation remains the same, but for early time, the "effective" crack size grows as $C_s t$:

$$K_I = 2\sigma\sqrt{C_s t/\pi}, \quad C_s t < a. \tag{4b}$$

These considerations imply that for suddenly applied loads of a prescribed duration t, there is some critical tension σ above which $K_I > K_{Ic}$, given sufficiently large cracks (of radius a > $C_s t$). Substituting into (4b) the stress at the onset of microcracking for σ , the duration of the tensile pulse for t, and the value of C_s for Bedford limestone, we compute K_{Ic} (K_I at the onset of microcracking) to be 2.4 and 2.5 MPa $m^{1/2}$ for the 1.3- and 0.5- μ s experiments, respectively. This is 2 to 3 times the 0.8 to 1.2 MPa $m^{1/2}$ reported by Ingraffea [1987] for Indiana limestone under static loading. The flaws activated are those with radius a > $C_s t$, or 3.6 and 1.4 mm for the 1.3- and 0.5- μ s experiments. Although a flaw diameter of 2.8 mm is smaller than the largest grains in Indiana limestone (3 mm according to

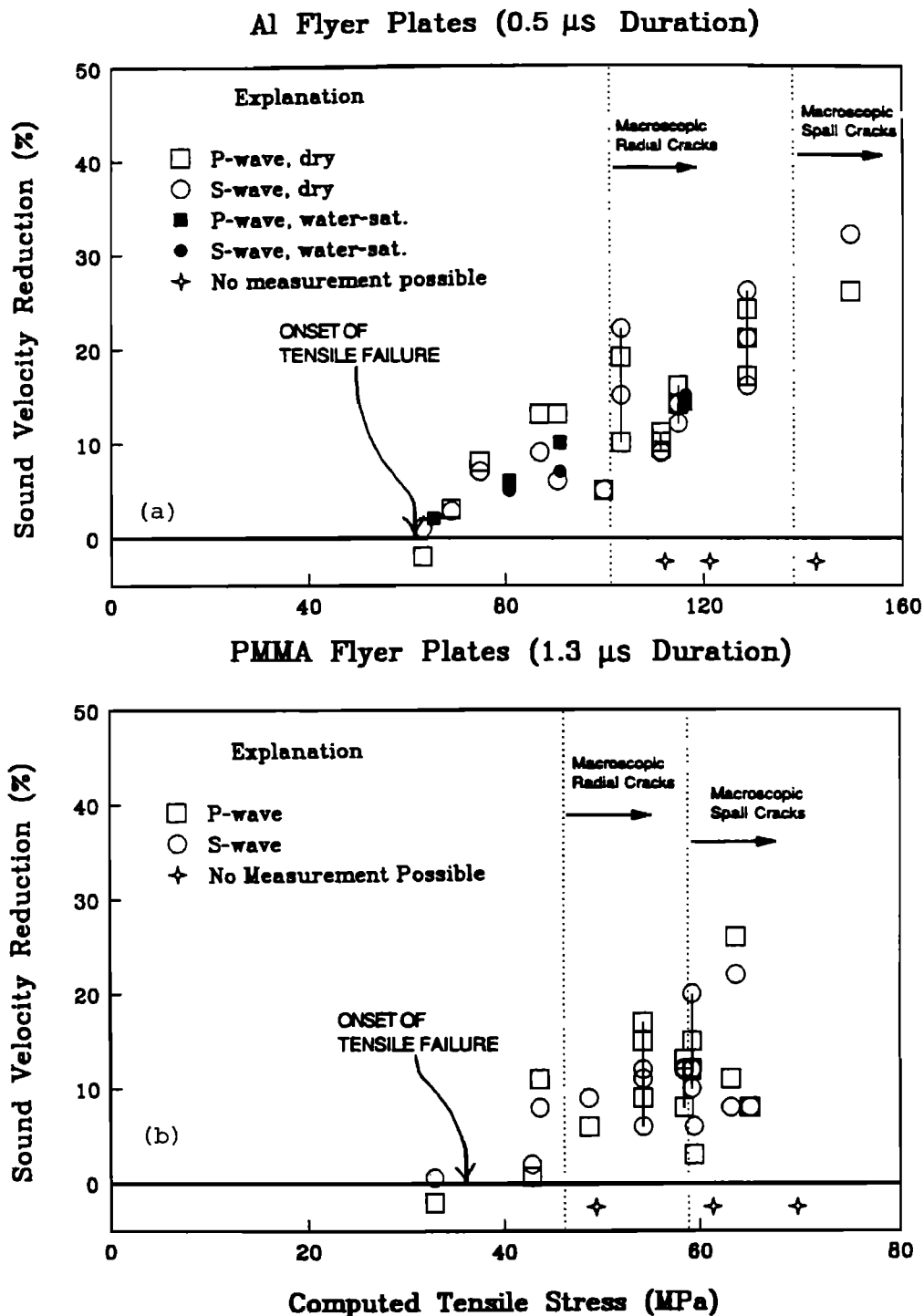


Fig. 2. Velocity measurements for (a) 0.5 μ s and (b) 1.3 μ s experiments. Dashed lines indicate pressures above which macroscopic radial and impact-face-parallel (spall) cracks occurred. Vertical bars connect measurements made on different pieces of targets broken by radial cracks.

Ingraffea and Schmidt [1978]), 7.2 mm is clearly much larger than the largest grains present.

Grady and Kipp [1987] interpret dynamic spall data and the size of the largest flaws observed in the samples for several rocks as consistent with crack growth from initial flaws at values of K_I equal to the statically determined fracture toughness. We identify two possible ways to relate the fracture toughness and initial flaw size from the present data to the static fracture toughness and grain size of Bedford limestone:

1. The computed magnitude and/or duration of the tensile loads could be overestimates. Grady and Kipp [1987] found typical strain rates in plate impact experiments to be $10^4/s$. For the current Young's modulus, this would produce stresses of 60 MPa in 0.125 μ s. However, wave scattering due to the large porosity and grain size (1/5 of the tensile pulse wavelength for the 0.5- μ s experiments) could produce further spreading of the wave front and attenuation of the peak stress. This possibility could be tested by obtaining particle velocity profiles at the back of the target [i.e., Grady and

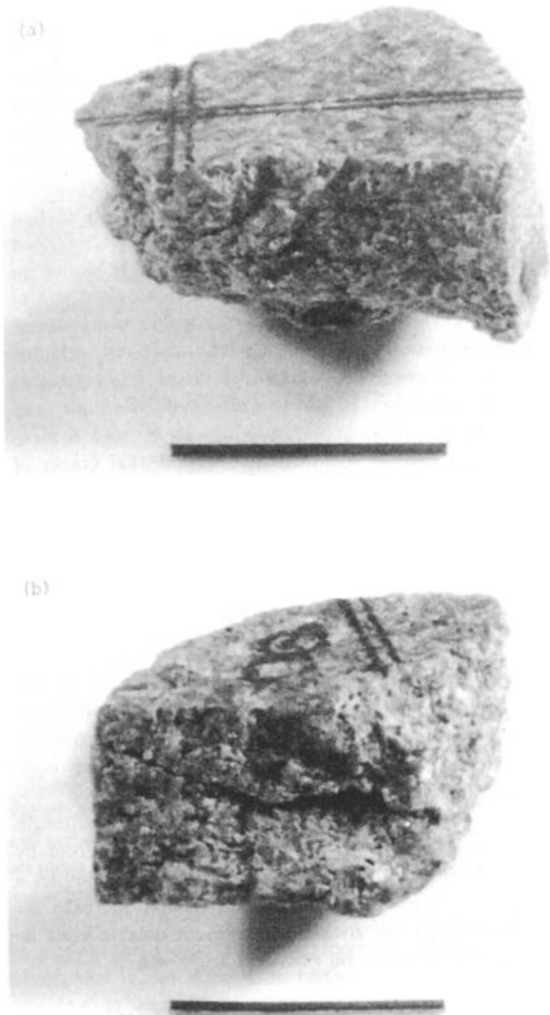


Fig. 3. Recovered fragments (a) and (b) of sample 8 impacted at 24 m/s with a PMMA impactor. Horizontal cracks are incipient spall cracks. Wire in photo is 0.5 mm in diameter and 1.0 cm long. The measured velocity reduction was 23 and 21% for P and S waves.

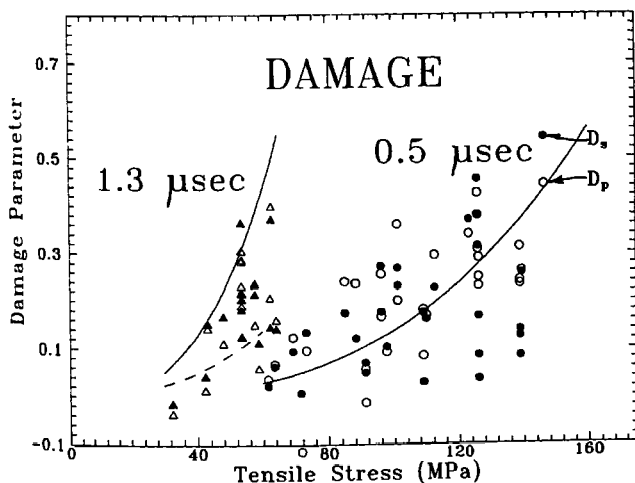


Fig. 4. Damage parameters D_p and D_s versus dynamic tensile stress. Curves indicating maximum damage at a given tensile stress for 0.5- and 1.3- μ s duration experiments are given by $D = 1.37 \times 10^{-7} \sigma^3$ and $D = 2 \times 10^{-6} \sigma^3$ where σ is in megapascals. Note that a dependence with stress $\propto \sigma^3$ is indicated by data. The dashed curve has a $\sigma^{2.5}$ dependence.

Kipp, 1979]. We note that Grady and Kipp [1987] obtained ~20 MPa as the dynamic tensile strength at a nominal strain rate of 10^4 /sec in Indiana limestone, which is close to that observed here (~30 MPa) for the onset of cracking at a nominal stress duration of 1.3 μ s.

2. The interaction of several closely spaced flaws could be critical in the initiation of tensile failure. Under static loading conditions, the stress intensity factors at the tips of closely spaced cracks can be much larger than for the same size isolated cracks. Under dynamic loading conditions, the time required for the stress intensity factors to reach their static values would be much longer for interacting cracks than for isolated cracks. Thus at the lowest dynamic loads producing failure, the time to failure could be larger than $a_l C_s$ where a_l is the radius of the largest flaw. Microcrack interaction could play an essential role in both dynamic and static rock failure.

We believe that the discrepancy between the dynamic and static data remains even for those cases cited by Grady and Kipp [1987] where the stress history has been determined more directly. The reason is that for initial crack lengths below some critical value, the "apparent" fracture toughness determined from static experiments has been found to decrease with crack length (Figure 5). The critical half length is about 40 mm for Indiana limestone, and for half lengths of 5 mm the apparent fracture toughness is only 0.5 MPa $m^{1/2}$ [Ingraffea and Schmidt, 1978]. A static tensile strength of 6 MPa would require an apparent fracture toughness as low as 0.26 MPa $m^{1/2}$, if the fractures initiated from isolated flaws the size of the largest grains present. A similar relationship between crack size and apparent fracture toughness has been found for Westerly granite, a low-porosity crystalline rock [Ingraffea et al., 1984].

The size dependence of the apparent fracture toughness has been explained qualitatively as resulting when the zone of inelastic deformation at the crack tip becomes large with

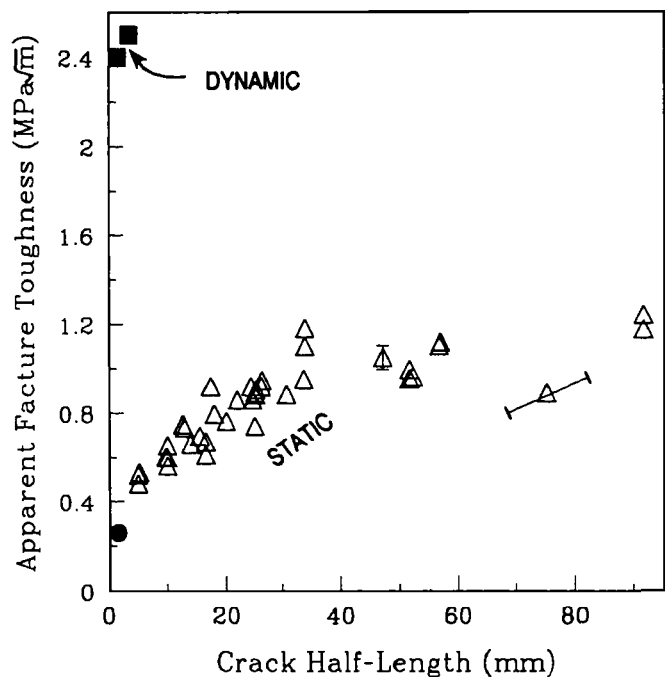


Fig. 5. Apparent quasi-static fracture toughness as a function of crack size in Bedford limestone (modified from [Ingraffea, 1987]). Bars give standard deviation of tests. Solid circle is from static tensile tests (tensile strength 6 MPa; isolated flaws of radius 1.5 mm), squares are present data (plotted at computed minimum crack length).

respect to the crack length. Under such conditions linear elastic fracture mechanics (LEFM) is inadequate for describing crack growth [Ingraffea, 1987]. Thus, although the dynamic data are apparently consistent with simple models assuming LEFM and values of K_{Ic} appropriate for large cracks [Grady and Kipp, 1987], the same models do not adequately explain the static tensile strength of rock. Microcrack interaction therefore appears to be required to reconcile the dynamic and static fracture toughness data. Under static conditions, crack interactions lower the tensile stress required for failure, relative to the isolated crack case. Under dynamic loading conditions, at relatively low stress levels, interaction would still be required for failure, but would require longer initiation times than for the isolated crack case. At high dynamic stress levels, interaction might not be required, but failure would occur at higher values of K_{Ic} than predicted, based on the static tensile strength assuming isolated cracks the size of the largest flaws observed.

Increasing Rock Damage with Stress Level

At higher tensile stress σ , K_I reaches K_{Ic} at earlier times t_c . Rewriting (4b), we obtain

$$t_c = \pi K_{Ic}^2 / (4C_s \sigma^2) \quad (5)$$

for the time to growth. Therefore for larger σ , cracks will have a longer time in which to grow prior to the cessation of tension. In addition, crack growth at earlier times implies more cracks of radius $a > C_s t_c$, so more cracks will be activated. Both these factors promote increasing crack density and ultimately complete spalling. Assuming an initial crack density of 0.2, the measured 30% reduction in sound velocity prior to complete spalling corresponds approximately to an increase in crack density to 0.4 [O'Connell and Budiansky, 1974]. At crack densities above about 0.5 the O'Connell and Budiansky model predicts that the material quickly disaggregates with increasing damage, providing a plausible explanation for our detected maximum velocity reduction. Other models, however, predict non-zero moduli for all finite crack densities [e.g. Zimmerman and King, 1985]. Within the interval 0.2 to 0.4 the velocity reduction increases nearly linearly with crack density, so we interpret the data as indicating increases in crack density of 0.003/MPa and 0.007/MPa in the 0.5- and 1.3- μ s experiments, respectively.

The ultimate spall strength of 60 MPa determined for the 1.3 μ s experiments is approximately 10 times the static tensile strength reported by Schmidt [1976], consistent with the ratio of dynamic and static strengths of other rocks. However, the amount by which the computed tensile stress exceeds the actual stress is expected to increase with impact velocity, as crack growth initiates and begins to relax the tension earlier.

Radial Crack Growth and Subequal Reductions in Wave Velocities P and S

The radial cracks observed to form above threshold stress levels for each of the two series of experiments indicate that deformation was not restricted to uniaxial strain. The radial cracks may result from failure first occurring at the sample edge during the initial compression. The Hugoniot elastic limit, or dynamic compressive strength under conditions of uniaxial strain, is about 400 MPa for Bedford limestone, well

above the stresses achieved here [Ahrens and Gregson, 1964]. However, near the sample edges the rock is essentially unconfined, which lowers the compressive strength. A second possible source for the cracks is that bending moments result from the perimeter of the flyer plate being supported by the lexan projectile.

Aligned cracks are expected to produce a greater reduction in P than S wave velocity, for a propagation direction perpendicular to the crack planes. If all cracks formed parallel to the target impact surface, a 20% reduction in longitudinal velocity would be associated with only a ~ 5 to 7% reduction in shear velocity, for reasonable crack aspect ratios [Anderson et al., 1974]. For randomly oriented cracks, a 20% reduction in longitudinal velocity is associated with an 18% reduction in shear velocity [O'Connell and Budiansky, 1974]. Subequal reductions in P and S wave velocities in the current experiments suggest that cracks of diverse orientations have been produced, as opposed to the dominantly face-parallel spall cracks expected. Thus both radial and face-parallel cracks are apparently contributing to the measured velocity reductions.

Effect of Water Saturation

Water is known to play an important role in the compressive failure of rock under static loading conditions. It can also reduce the tensile strength by providing chemically reactive species to facilitate molecular bond breaking at crack tips.

The effect of pure water on the dynamic tensile strength of rock is unknown. It could steepen the wave front by reducing scattering, introduce viscous dissipation, impart a viscous suction to opening crack walls, and increase the dynamic modulus of the rock. The present data indicate no significant effect of water on microcrack growth.

EXPERIMENTAL PROCEDURE FOR AXISYMMETRIC CRATERING EXPERIMENTS

Samples of Bedford limestone from the same aliquot as the one-dimensional sample, and San Marcos gabbro, similar to that employed in previous studies [Lange et al., 1984; Polanskey and Ahrens, 1990], were acquired. The San Marcos gabbro specimen is a 20-cm cube, with a density of 2.867 g/cm³ and a very low initial crack density [Lange et al., 1984; Wong and Biegel, 1985]. It was impacted by a 3.2-mm-diameter steel projectile (mass 0.143 g) at a speed of 5.2 km/s. The resulting crater is approximately 10 cm across and 1.0 cm deep. The upper 5 cm of the specimen was cut in half (Figure 6) and a 1-cm-thick section was cut from one of the halves. The half of this section corresponding to the left half of Figure 6 was cut into oriented ~1-cm cubes for P wave velocity measurements. The impactor mass and velocity were chosen so as to produce a large crater compared to the size of the 1 cm cubes but small enough that large edge effects would not be produced by the target boundaries.

The shocked Bedford limestone specimen is a block, 20 cm on a side and 12 cm deep. The large rock sample has a bulk density of 2.62 g/cm³, an average grain size of approximately 0.5 mm, and a pore porosity of approximately 0.12. Cracks, as opposed to pores, were not observed with an optical microscope. It was impacted by a 5.6-mm-diameter copper-jacketed lead bullet (mass 3.2 g) at a speed of 1.2 km/s. The upper 6.8 cm of the sample was cut in half in order to prepare oriented cubes as for the gabbro.

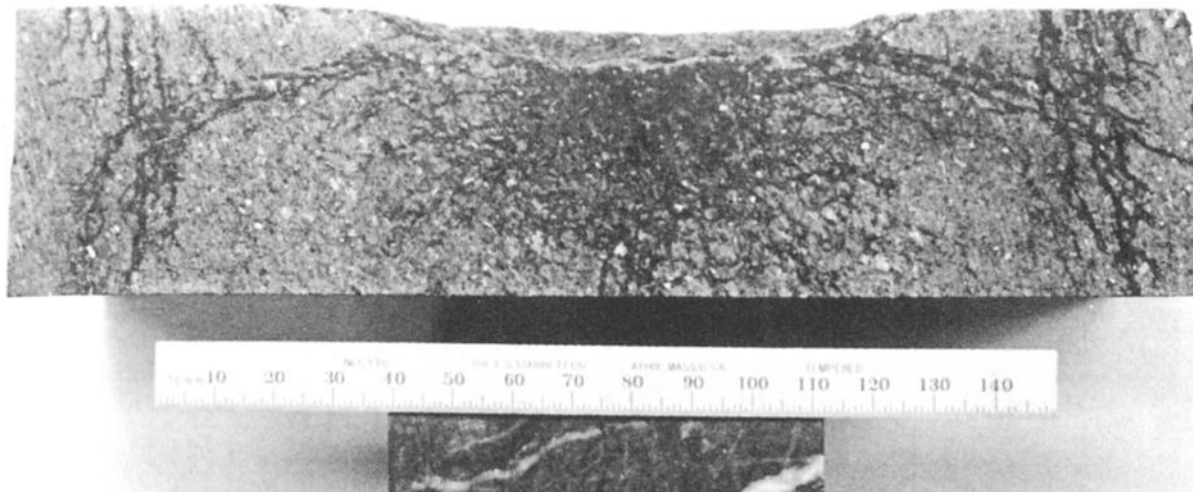


Fig. 6a. Cross section of shocked San Marcos gabbro cut through a crater diameter. Projectile was fired vertically downward upon upper surface (at approximately the 80 mm mark). Measurements were made on the facing half of the sample corresponding to the left half of the figure. Cracks are highlighted by water-soluble saw coolant. The crack density near the impact site is great enough that individual cracks cannot be distinguished in this photograph.

The ~1-cm cubes from both samples were dried in an evacuated oven at 100°C until further weight loss over a 12-hour period was less than 10^{-4} g, and seismic velocities were measured at 0.4 MHz. The P wave velocity C_p of the unshocked gabbro and limestone were determined to be approximately 6.4 ± 0.2 and 3.9 ± 0.1 km/s, respectively. We assume the 1-cm³ samples are representative of the rock in bulk; that is, we believe no further crack damage was done upon sawing and in preparing the samples for ultrasonic measurements. The detailed velocity data are presented in Table 3. Although the velocity anisotropy of the unshocked samples was not determined, the differences in velocity in different directions is expected to be small relative to the velocity reductions seen in the data. This is confirmed by the directional velocity measurements far from the axis of impact, where the sound speed approaches the unshocked value, in Figures 9 and 11. The difference between the average velocity in the three principal directions is less than the scatter in velocities for each direction.

RESULTS FOR AXISYMMETRIC EXPERIMENTS

Compressional Wave Velocity Measurements

San Marcos Gabbro. Velocity measurements for aliquots of gabbro were measured in orthogonal directions as defined in Figure 7. Local seismic anisotropy was observed in the upper left part of the specimen, where major vertical fractures associated with the specimen edge can be observed (Figure 6). Such fractures are expected to reduce the crack-perpendicular (x) velocity much more than the crack-parallel (z, y) velocities [Anderson *et al.*, 1974], which is the observed result (Figure 8). The reduced z velocities at the 1.5 cm depth are most likely due to the subhorizontal cracks associated with reflections from the free surface (Figures 6 and 7a).

Elsewhere, the anisotropy is less pronounced. Figure 8 illustrates the P wave velocity in the $x, y,$ and z directions at 2.5 cm depth within the sample. Within the central portion

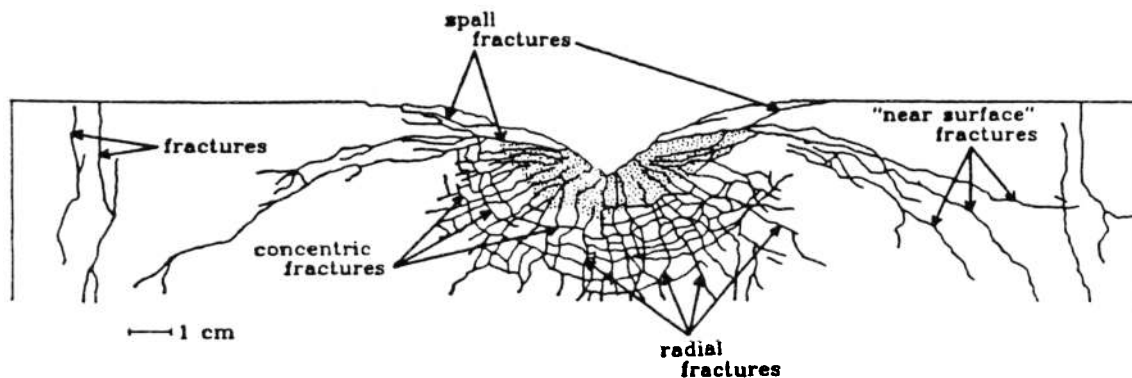


Fig. 6b. Cross section of the target from shot 840904 illustrating the classification of internal fractures. Note that all fractures have been drawn with the same line thickness despite actual variations in the target. The shaded area immediately below the crater indicates a highly fractured region [Polanskey and Ahrens, 1990].

TABLE 3a. Compressional Wave Velocity Beneath Impact Craters in Bedford Limestone

	Radius from Crater Center, cm								
	0-1	1-2	2-3	3-4	4-5	5-6	6-7	7-8	
Depth, cm					0.5-1.5		0-1		
					4.1(a)	4.3	4.0	4.0	
					4.0(b)	4.2	4.1	3.5	
					4.0(c)	4.0	4.0	4.0	
Depth, cm				1-2	1.5-2.5		1-2		
				3.6	3.8	3.8	3.9	3.8	
				3.4	3.4	4.0	3.7	3.5	
				3.3	3.5	3.8	3.4	3.6	
Depth, cm			2-3		2.5-3.0		2-3		
	1.6	2.1	3.3	3.8	3.6	3.8	3.7	3.5	
	1.6	2.0	3.3	3.6	3.6	3.6	3.4	3.6	
	1.7	1.9	3.1	3.0	3.3	3.7	3.5	3.4	
Depth, cm					3-4				
	2.5	3.0	3.6	3.8	3.8	3.9	3.8	3.9	
	2.8	3.0	3.5	3.9	3.9	3.7	3.8	3.9	
	2.9	3.0	3.5	3.9	4.0	4.0	3.9	3.9	
Depth, cm					4-5				
	3.0	3.4	3.5	3.6	3.6	3.8	3.8	3.7	
	3.2	3.4	3.5	3.7	3.9	3.8	3.8	3.8	
	3.4	3.6	3.6	3.6	3.8	3.9	3.8	3.8	
Depth, cm					5-6				
	3.3	3.8	3.8	4.0	3.8	3.9	4.0	3.6	
	3.5	3.9	3.9	3.9	3.8	3.9	4.1	3.9	
	3.7	3.8	3.9	3.8	4.0	4.1	4.0	3.9	

(a), (b), and (c) refer to compressional wave velocity along directions defined in Figure 5.

TABLE 3b. Compressional Wave Velocity Beneath Impact Craters in San Marcos Gabbro

	Radius from Crater Center, cm									
	0-1	1-2	2-3	3-4	4-5	5-6	6-7	7-8	8-9	9-10
Depth, cm						0.5-1.5			0-1	
						6.2(a)	6.8	5.9	4.7	5.8
						5.8(b)	6.2	6.4	6.5	6.6
						6.0(c)	6.4	6.4	6.0	6.4
Depth, cm			1-2		1.5-2.0			1-2		
		4.9	5.7	5.8	6.0	6.4	6.3	6.6	4.8	6.3
		4.1	4.8	6.0	5.4	5.8	6.0	6.1	5.6	5.7
		3.0	4.9	5.7	5.9	6.0	6.1	6.5	6.1	6.5
Depth, cm					2-3					
	4.5	4.8	5.7	5.8	6.1	6.5	6.7	6.3	4.8	6.5
	3.8	4.5	4.8	5.5	6.2	6.2	6.3	6.3	6.1	6.4
	4.2	4.9	5.0	5.4	6.2	6.0	6.5	6.6	6.6	6.6
Depth, cm					3-4					
	4.8	5.1	5.4	5.9	6.2	6.5	6.5	6.1	6.0	5.9
	5.0	5.0	5.7	6.2	6.1	6.6	6.5	6.2	6.1	6.2
	4.9	5.0	5.3	5.8	5.9	6.3	6.3	6.4	6.3	6.6
Depth, cm					4-5					
	5.8	5.6	5.8	5.7	6.1	6.1	6.2	6.0	5.9	6.1
	5.5	6.0	6.0	6.3	6.2	6.6	6.4	6.3	6.4	6.5
	5.4	5.6	5.5	5.7	5.9	6.3	6.7	6.2	6.2	6.7

See Table 3a footnote.

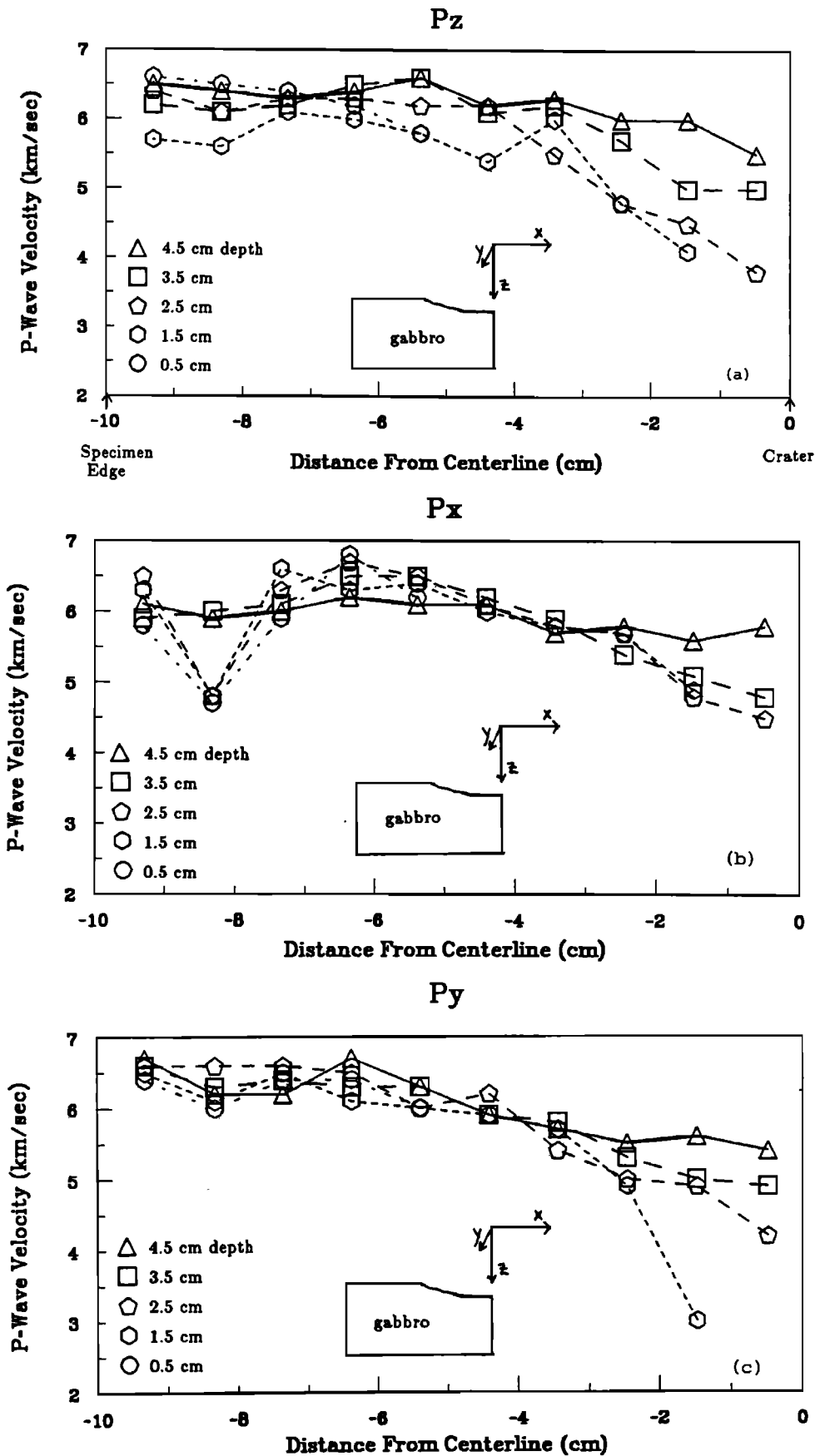


Fig. 7. P wave velocities for gabbro as a function of distance from the z axis (projectile path), at indicated depths within the sample. Sample velocities farthest below the impact site are generally the highest. (a) Pz, velocity in z direction; (b) Px, velocity in x direction; (c) Py, velocity in y direction (see sketch).

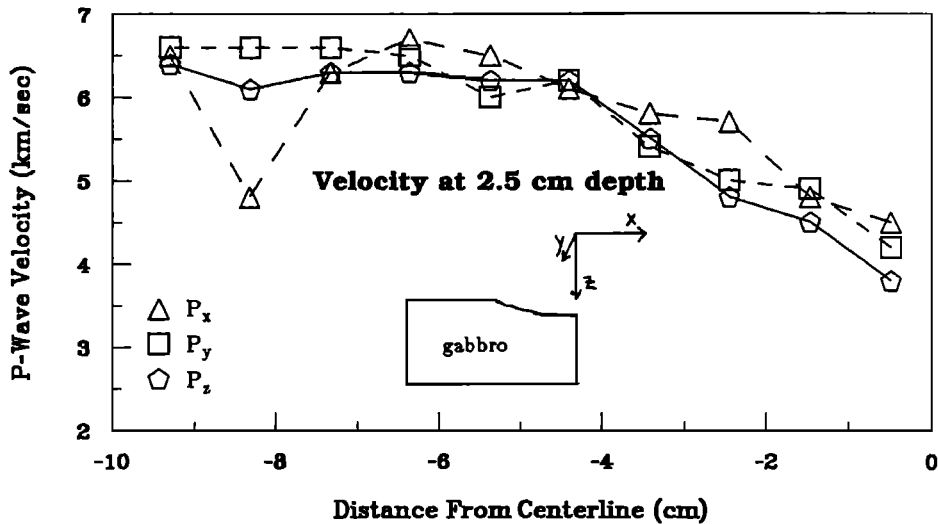


Fig. 8. P wave velocities for gabbro in x, y, and z directions at 2.5 cm depth below the surface. The pronounced anisotropy near the sample edge is due to the oriented macroscopic cracks visible in Figure 6. It also appears that the z direction is slowest of the three beneath the crater, but reaches its far-field velocity at lesser distances than the x and y directions.

of the crater the vertical (z) velocity is slightly but consistently less than both horizontal (x, y) velocities. This is consistent with the large subhorizontal cracks somewhat concentric with the crater floor (Figure 6). In addition, from Figures 7 and 8 it appears that with the exception of the sample located at a depth of 1.5 cm, the vertical (z) P wave velocity approaches unshocked values at lesser radial distances than do the horizontal (x,y) velocities (approximately 4.5 cm as opposed to 6.5 cm). The 1.5-cm-depth layer includes the large sub-horizontal cracks outside the crater region in Figure 6.

Figure 9 shows all the velocity measurements for gabbro versus radial distance from the assumed effective center (located at 1/2 the crater depth) of the approximately hemispherical stress wave emanating from the impact. The seismic velocity increases smoothly to its unshocked value at a radial distance of approximately one crater radius. The average velocity in the vicinity of the crater floor is approximately 0.65 times the unshocked velocity.

Bedford Limestone. The velocity measurements on limestone (Figures 10 and 11) resemble those made on the gabbro. The seismic velocity, occurring just beneath the crater floor, is reduced to values slightly less than half the unshocked value. The maximum velocity reduction is slightly greater for the limestone (0.59 times the unshocked value) than for the gabbro (0.53 times the unshocked value). These velocity reductions are some 50% greater than those observed in the one-dimensional experiments described previously in this paper. Although the loading history of a given zone of rock differs between the uniaxial and cratering experiments, the greater velocity reductions are probably the result of the greater damage which can be inflicted on rock which is internal to a massive block on account of the intrinsic inertial confinement of three-dimensional geometry. Seismic anisotropy within the limestone, if present, is much less than within the gabbro. This is particularly evident nearest the crater center, where the scatter in velocity for the limestone (Figure 11) is strikingly less than for the gabbro

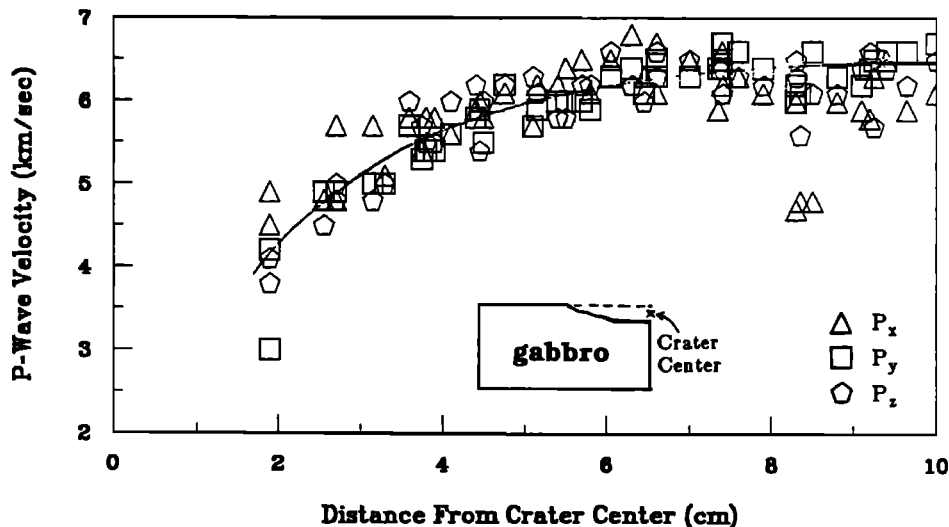


Fig. 9. Plot of all velocity measurements for gabbro as a function of distance from the crater center. Curve is eyeball fit to data. P_x, P_y, and P_z refer to wave propagation along the direction as defined in Figure.

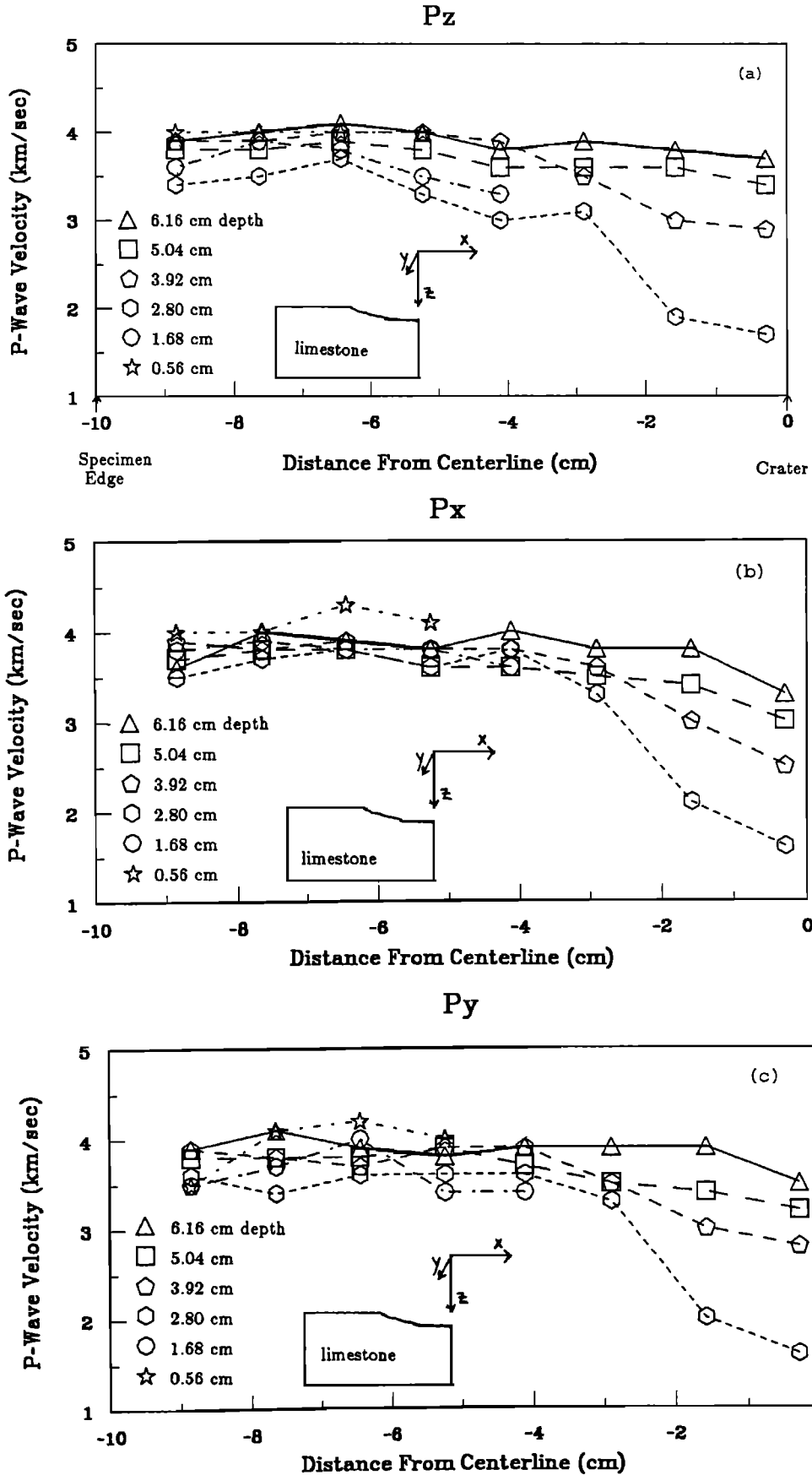


Fig. 10. P wave velocities for Bedford limestone as a function of distance from the z axis (projectile path), at indicated depths within the sample. Sample velocities furthest below the impact site are generally the highest. (a) P_x , velocity in z direction; (b) P_x , velocity in x direction; (c) P_y , velocity in y direction (see sketch).

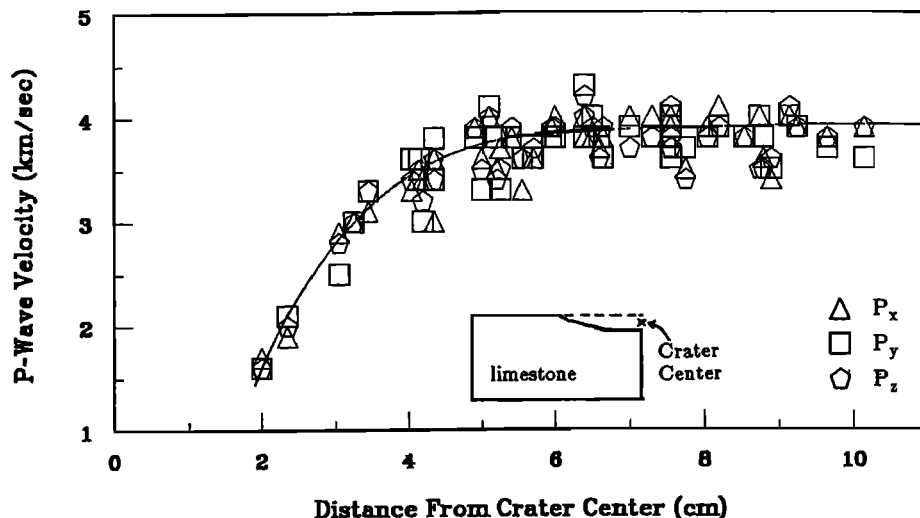


Fig. 11. Plot of all velocity measurements for Bedford limestone as a function of distance from the crater center. Peak stress levels in rock vary from 900 MPa at 2 cm to 200 MPa at 5 cm.

(Figure 9). The compressional wave velocities increase relatively smoothly with distance from the crater center (Figure 11). The average of the velocity reductions at distances less than approximately one crater radius is significantly greater within the limestone than within the gabbro. The seismic velocity approaches its unshocked value at a slightly lesser distance, relative to the crater radius, in the limestone than in the gabbro.

Correlation with Crack Density

Comparison of Figure 6 with Figures 9 and 11 indicates that the zones of greatest velocity reduction correspond closely to the zones of greatest crack concentration.

Microscopic observation of the 1-cm cubes of gabbro at 25x indicates cracks ranging from grain boundary cracks and cleavage cracks internal to plagioclase and amphibole (0.1-1 mm) to cracks which apparently traverse the entire cube.

Cleavage cracks and grain-crossing cracks are restricted largely to highly shocked low-velocity samples near the impact. Both crack density and maximum crack length increase as seismic velocity decreases. Thus samples with relatively high velocities far from the impact have about 1 crack of average length 0.5 mm per mm of traverse, while samples close to the impact have as many as 2.3 cracks of average length 1.2 mm per mm of traverse.

Existing theories relating seismic velocity reduction to crack density assume either that the cracks are randomly oriented [e.g., O'Connell and Budiansky, 1974], or uniformly aligned [e.g., Anderson et al., 1974]. The cracks within the cubes cut from the shocked gabbro have preferred but non-uniform orientations and are therefore an intermediate case to those modeled theoretically. The crack density parameter in these studies is $\epsilon = N \langle a^3 \rangle$, where N is the number of cracks per unit volume and $\langle a^3 \rangle$ is the average of the cube of the characteristic linear dimension (half length) of the crack

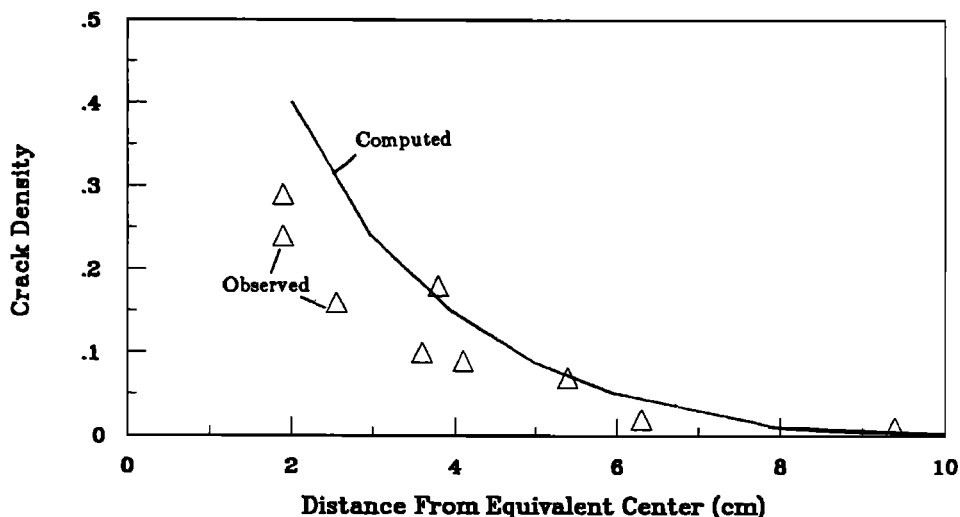


Fig. 12. Plot of maximum observational estimate of ϵ for each gabbro sample measured (triangles), as a function of distance from the crater center. Also shown is the computed plot of ϵ versus distance, using the theory of O'Connell and Budiansky [1974] and the fit to the velocity data from Figure 9 (solid line).

[O'Connell and Budiansky, 1974, equation (4)]. Because of the a^3 term the influence on velocity is heavily weighted toward the largest cracks present. O'Connell and Budiansky [1974] relate ϵ to the observed crack trace density statistically by

$$\epsilon = \frac{3}{4\pi} M \langle l^2 \rangle, \tag{6}$$

where M is the number of line segments per unit area and l is the observed segment length. Optical estimates of ϵ were made on nine cubes at various distances from the crater center. Independent estimates were obtained for each of the x , y , and z directions, based on the number and lengths of cracks with length greater than 0.4 mm crossed by oriented traverses. Estimates of ϵ ranged from 0.01 near the specimen edge to 0.28 just beneath the crater. The range of directional values of ϵ within a single sample were typically considerably greater than the observed range of seismic velocity measurements for the sample and the largest value of ϵ is plotted in Figure 12.

Using the O'Connell and Budiansky (random orientation) theory, ϵ is computed from

$$\frac{\bar{K}}{K} = 1 - \frac{16}{9} \left(\frac{1 - \bar{\nu}^2}{1 - 2\bar{\nu}} \right) \epsilon \tag{7}$$

$$\frac{\bar{\nu}}{\nu} = 1 - \frac{16}{9} \epsilon \tag{8}$$

$$\frac{\bar{C}_p}{C_p} = \left(\frac{(1 - \bar{\nu})(1 + \nu) \bar{K}}{(1 + \bar{\nu})(1 - \nu) K} \right)^{1/2} \tag{9}$$

where K is bulk modulus, ν is Poisson's ratio, and bars denote effective values appropriate for the cracked body. The values of ϵ that would reproduce the fit to the velocity data in Figure 9 are shown in Figure 12, together with the values determined optically. An initial crack density ϵ of 0.0 (for practical purposes equal to the lowest observed value of 0.01) was assumed. The computed values of ϵ are up to a few tens of percent greater than the observed values. The computed value of ϵ producing the maximum P wave velocity reduction of 50% is 0.4, as opposed to the observed value of 0.28. This discrepancy could be due in part to the neglect of cracks less than 0.4 mm in length. The theory of O'Connell and Budiansky [1974], which assumes random crack orientation, cannot be used in this simplified fashion to describe the directional properties of the velocity measurements in detail. Further microscope observation, shear wave velocity measurements, and consideration of aligned-crack models are necessary to determine if refinements of the theoretical models relating crack density to modulus reduction in shocked rock are required.

Observation of the crack density in the limestone at a magnification of 50X indicates that in the closest region, about 2 cm distant, nearly all the grains are severely fractured and shattered. A short distance further about half the grains are fractured, with from 1 to about 10 cracks each. By about 3 cm distant, only about 5% of the grains contain visible fractures, although the velocity reduction is still pronounced. These observations indicate that the initial stage of inelastic failure involves fracture along grain boundaries, which could explain the apparent lack of anisotropy of the velocity measurements.

APPLICATION OF LINEAR ELASTIC FRACTURE MECHANICS TO FRACTURE DAMAGE IN THREE DIMENSIONS

Mechanism of Shock Damage

Detailed studies of crack patterns within impact craters in rock [e.g. Polansky and Ahrens, 1990], indicate that some three families of cracks are produced upon impact onto a half space of brittle media such as rock. These three crack families are (1) radial, (2) concentric, and (3) spall fractures (Figure 13). There exist two families of radial fracture (spoke and conical) which are sketched in Figure 14. Field's [1971] study suggests that radial crack formation initiates immediately behind the outgoing stress wave, whereas the concentric cracks are initiated at later times and appear to be related to the tensile phase of the stress pulse, associated with sudden release of the impulsive load applied at the surface [Selberg, 1952; Shibuya and Nakahara, 1968]. The spall fractures parallel to the impact surface are believed to result from the interaction of the compressive stress pulse with the free surface [Melosh, 1984; Swain and Hager, 1980], as are the vertical cracks near the edges of the sample. Shibuya and Nakahara [1968, Figures 5 and 6] demonstrate that the hoop stresses $\sigma_{\theta\theta}$ and $\sigma_{\phi\phi}$, which are responsible for the radial fractures, reach their peak tensile value behind the outgoing compressive wave front, immediately prior to the arrival of the shear wave. In the following we assume that the bulk of the damage is produced by both types of radial cracks, whose formation is more straightforwardly relatable to calculated stress wave profiles.

Such radial cracks which grow are perpendicular to the direction of peak tension, and thus normal to the quasi-spherical compressive wave front. We first test the hypothesis that these cracks begin to grow at the time t_c given by (5). At a time $t > t_c$ after the onset of a tensile stress pulse, cracks of initial length l_0 will increase in length by $\Delta l = C_s(t - t_c)$, assuming they grow at the shear wave velocity. Substituting (5) for t_c , we have

$$\Delta l = C_s t - \frac{\pi}{4} \frac{K_c^2}{\sigma^2} \tag{10}$$

The relation between the crack damage parameter ϵ of O'Connell and Budiansky [1974] and the cracked compressional wave velocity C_p can be approximated by

$$1 - \frac{C_p}{C_{p0}} = \alpha_p (\epsilon - \epsilon_0) \tag{11}$$

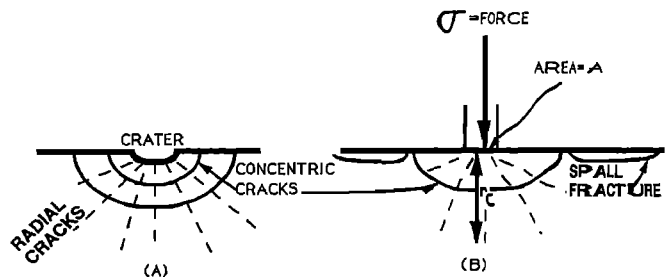


Fig. 13. Diagrammatic cross-section showing three crack families, radial, concentric, and spall fractures: (a) geometric definition and (b) definition of parameters of Anstis et al. [1981] radial spoke fracture model.

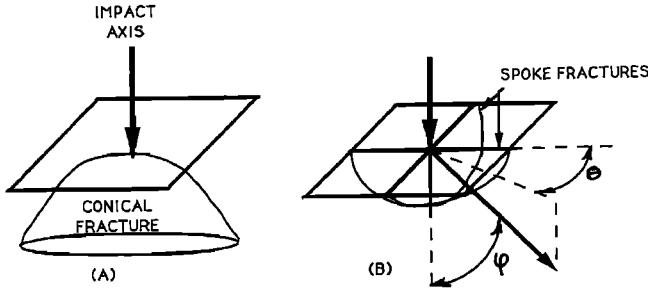


Fig. 14. Two subclasses of radial fractures from impact: (a) conical fracture and (b) spoke fracture.

where α_p is a constant given approximately by 1.3 over the range $0 \leq \epsilon \leq 0.4$ [see O'Connell and Budiansky, 1974, Figure 6]. Since

$$\epsilon/\epsilon_0 = \langle l^3 \rangle / \langle l_0^3 \rangle, \quad (12)$$

assuming all existing cracks are of similar length and grow by Δl , we find upon linearizing (equation (12)), and substituting into (11), that

$$1 - \frac{C_p}{C_{p0}} \cong \alpha_p \epsilon_0 \left[(1 + 3\Delta l / l_0) - 1 \right] \quad (13)$$

Upon substituting (10) into (13), we obtain

$$1 - (C_p / C_{p0}) = 3 \alpha_p \epsilon_0 \left[C_s t - \pi K_c^2 / (4 \sigma^2) \right] / l_0 \quad (14)$$

The first term in braces in (14) predicts the behavior of velocity deficit with time, at constant tensile stress, whereas the second term yields a prediction of how velocity deficit varies with tensile stress amplitude, which we can relate to radial distance.

We can employ the results of the one-dimensional experiments to compare with the predictions of the "C_st" term. For example, we note that the velocity deficit is predicted to be linear with time, at constant tensile stress. Using Figure 2, we note that for 0.5- and 1.3-μs duration tensile stress pulses with amplitudes of 70 MPa, velocity deficits 3 and 30% are observed, respectively. The first term in braces would predict a 8% velocity deficit at 1.3 μs from the 0.5 μs value, whereas a 30% deficit is observed. Thus, the first term underestimates the effect of time duration of applied tensile stress. The second term overestimates a dependence of velocity deficit on tensile stress ($\propto \sigma^{-2}$), whereas a dependence ($\propto \sigma$) is observed.

Parameterization of Impact Damage

In this section we relate the damage parameter D, observed as a function of distance in the axi-symmetric experiments, to computations of D as a function of tensile stress magnitude and duration. Because we find that damage as detected via both longitudinal D_p and shear D_s wave velocity deficits are indistinguishable in the present experiments (Figures 2 and 4), we simply employ the variable D. An expression for dD/dt can be based on theories of the nucleation and growth of cracks which have an initial Weibull distribution [Grady, 1980; Grady and Kipp, 1987]. A Weibull distribution specifies that the number of cracks per unit volume n activated at or below a tensile strain Σ are given by

$$n = k \Sigma^m \quad (15)$$

where k and m are empirical constants. Grady and Kipp [1987] find

$$\frac{dD}{dt} = (36\pi)^{1/3} \left[k \left(\frac{\sigma}{M(1-D)} \right)^m \right]^{1/3}$$

$$C_g D^{2/3} (1-D)^{(1-m/3)} \quad (16)$$

respectively. Here M is the modulus of the wave (either longitudinal or shear), and C_g is the crack propagation velocity which we assume here is given by C_s.

Upon separating variables,

$$\frac{(1-D)^{(m/3)-1}}{D^{2/3}} dD = (36\pi)^{1/3} \left[k (\sigma/M)^m \right]^{1/3} C_g dt \quad (17)$$

Approximating the left hand side of (17) by $\{1 - [(m/3) - 1]D\} / D^{2/3}$ yields, upon also dropping terms of order D^{4/3} relative to terms of order D^{1/3}:

$$D \cong \frac{4}{3} \pi k (\sigma/M)^m C_g^3 t^3 \quad (18)$$

We shall relate (18) and the experimental results to observations of damage from impact-induced quasi-hemispherical shock waves in gabbro and limestone. The maximum damage D for a constant duration of tensile stress application, as measured by either P or S wave velocity deficits, can empirically (e.g., Figure 4) be described by a relation of the form

$$D = A_n \sigma^m, \quad n = 0.5 \text{ and } 1.3 \quad (19)$$

where for the solid curves shown in Figure 4, m = 3 and A_{0.5} = 2 x 10⁻⁶ MPa⁻³ and A_{1.3} = 1.37 x 10⁻⁷ MPa⁻³, respectively, for the 0.5- and 1.3-μs duration experiments. The parameters of the solid curves of Figure 4 are constrained by the data. For example, the dashed curve through the 1.3-μs data has the parameters, A_{1.3} = 2 x 10⁻⁶ MPa^{-2.5} and m=2.5.

Thus, from Figure 4, at constant t, we obtain m = 3. Moreover, we see that m = 2.5 does not fit the 1.3-μs duration data. The solid curve fits shown in Figure 4 are a compromise. Here, m = 3 is an upper bound to the 1.3-μs data, whereas it is slightly lower than what would give an optimum fit to the 0.5-μs data. This compares to m = 9.1 obtained by Lange et al. [1984] for impact spall of the present San Marcos gabbro, and m = 8 obtained by Grady and Kipp [1987] in fragmentation studies on oil shale. The peak dynamic compressional (and the following tensional) stress from an impact on a half-space is expected to approach a r⁻² dependence [Melosh, 1989]. However, very close to the impact the stress wave decay is less rapid [Ahrens and O'Keefe, 1977]. We therefore assume that in the regime of interest $\sigma \propto r^{-1.5}$. Moreover, numerical calculations of Shibuya and Nakahara [1968] found the duration of the tensile hoop stress associated with the longitudinal wave t was proportional to radius, i.e., t ∝ r.

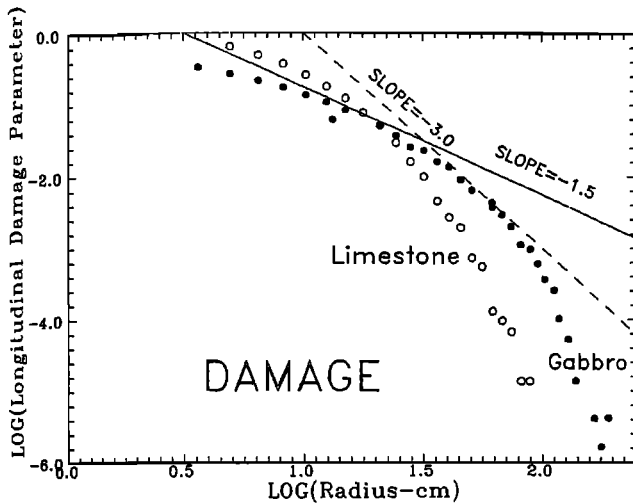


Fig. 15. Log, damage (longitudinal wave) parameter versus log, radius in centimeters, for impact experiments on blocks of Bedford limestone and San Marcos gabbro.

Therefore, we predict for the limestone from (18), for the damage associated with the outgoing longitudinal wave that

$$D \propto (r^{-1.5})m r^3 \quad (20)$$

or

$$D \propto r^{-1.5} \quad (21)$$

In Figure 15, it can be seen that this predicted decay of D agrees well with the initial slope of limestone data. Undoubtedly, other dynamic stresses (in addition to $\sigma_{\theta\theta}$ and $\sigma_{\phi\phi}$) affect the crack damage parameter. However, radial cracking appears to result from the tensile stress associated with the elastic longitudinal wave and the decay of this stress is well described. We note that if we assume a $\sigma \propto r^{-2}$ dependence, rather than $r^{-1.5}$, equation (20) then predicts that $D \propto r^{-3}$. Notably, this more rapid stress decay fits the data of Figure 15 further from impact point in the more nearly elastic regime.

We note that if the $m = 9.1$ value for gabbro is employed in (20), then a dependence of $D \propto r^{-10.5}$ is predicted. This is a much stronger (and less credible) dependence on r than obtained using $m=3$. It may be that values of "m" from fragmentation experiments such as those of *Langeet et al.* [1984] simply do not apply.

Finally, the extent of radial cracking beneath the crater is also approximately described by cracking in the "spoke" mode as discussed in the appendix.

CONCLUSIONS

Samples of Bedford limestone were subjected to planar impacts in order to produce tensile failure under conditions of nearly one-dimensional strain to obtain the fracture toughness under dynamic loading conditions. Modern theories of the damage mechanics of brittle solids are able to relate the fracture toughness of rock (measured under tensile stress) to compressive failure [e.g., *Ashby and Sammis*, 1990; *Isida and Nemat-Nasser*, 1987]. Impact velocities were chosen to cover a range of stresses at and below those required to produce complete spalling, and the ultrasonic velocities of the recovered samples were measured to assess the degree of microcracking. The experiments were of 0.5- and 1.3- μ s duration. Major conclusions are as follows:

1. The onset of microcracking occurs at lower stresses for the longer-duration experiments; approximately 35 MPa as opposed to 60 MPa. Complete spalling also occurs at lower stresses for the longer duration experiments; approximately 60 MPa as opposed to 140 MPa.

2. Estimates of the rock fracture toughness based on the stress at the onset of microcracking yield consistent results: 2.4 MPa $m^{1/2}$ for the 1.3- μ s experiments and 2.5 MPa $m^{1/2}$ for the 0.5- μ s duration experiments. These values are somewhat higher than the quasi-static values determined for porous limestone, which range from 0.8 to 1.2 MPa $m^{1/2}$.

3. Estimates of the initial flaw size activated during failure are 6.8 mm for the 1.2- μ s duration experiments and 2.8 mm for the 0.5- μ s duration experiments. In comparison, the largest grains in Bedford limestone are approximately 2 mm in length. One possible explanation for the relatively large computed size of the initial flaws is that in porous rock several closely spaced flaws must interact to produce crack growth at relatively low stresses. This could be an important mechanism in controlling dynamic failure in porous material.

4. Three-dimensional laboratory impact experiments were performed on crystalline (San Marcos gabbro) and porous (Bedford limestone) rocks. The targets were approximately 20 cm on a side and the resulting craters were approximately 5 cm in radius and 1 cm deep. The seismic velocities of oriented 1-cm cubes were measured from the sectioned targets and related to induced crack densities via the theory of *O'Connell and Budiansky* [1974] and optical microscope observations. We compare the results from the porous and crystalline tests: (1) In both cases the impacts resulted in a seismic velocity (elastic modulus) reduction within the target that decayed essentially monotonically with distance from the impact site. (2) The average velocity reduction within the most heavily shocked region was greater within the limestone (approximately 0.4 times the unshocked value) than the gabbro (approximately 0.65 times the unshocked value), despite the fact that the peak pressure was greater within the gabbro. The seismic velocity approaches its unshocked value at lesser radial distance within the shocked limestone than within the shocked gabbro, suggesting greater attenuation within the porous limestone.

5. The radius of significant fracture damage beneath the impact craters was found to be describable by the quasi-static theoretical relation of radial cracking to fracture toughness of *Anstis et al.* [1981] (see the appendix). Using measured values of fracture toughness of 2.45 and 5.23 MPa $m^{1/2}$ for Bedford limestone and San Marcos gabbro, inferred impact stresses agreed to within ~50% of that inferred from conventional shock impedance calculations.

6. If nucleation and growth of cracks from an initial Weibull distribution is assumed, the radial dependence of the crack damage parameter in Bedford limestone is predicted to be $D \propto r^{-1.5}$ on the basis of the time and stress dependence of the *Grady and Kipp* [1987] damage model, and the present one-dimensional tensile stress experiments. This is in approximate agreement with the observed variation of D in the three-dimensional experiments on limestone.

APPENDIX: EXTENT OF FRACTURE FROM QUASI-STATIC CONSIDERATIONS

The extent of fracture from impacts into solid brittle media can be related to the fracture toughness of the target. *Anstis et al.* [1981] developed a semiempirical theory for which it was demonstrated that the radius of the radial

(spoke) fracture, r_c (Figure 14b) beneath quasi-static indentation into a brittle medium half-space is related to the mode I fracture toughness via

$$K_C = \sigma A / (\beta_0 r_c^{3/2}) \quad (A1)$$

where σ and A are the compressive force and area of an indenter acting on a half-space and β_0 is an empirical (dimensionless) constant. Using indentation data assembled by Anstis et al. for some 13 ceramics and glasses, we find that $\beta_0 = 16.2 \pm 5.1$. We now use the measured values of r_c , A , and K_C in equation (10) to calculate the impact stress, σ , and compare it to the impact stresses calculated from the conventional shock wave equation of state [Ahrens, 1986]. We have already obtained values of r_c and A from knowledge of the distance that the sound speed is degraded in the target and the projectile diameter given above. We note that from Figures 9 and 11, $r_c \cong 5$ cm for both impact into gabbro and limestone. We have determined K_C for Bedford limestone as $2.45 \text{ MPa m}^{1/2}$. For San Marcos gabbro, we employ equation (4b) and the previously measured spall strength of Lange et al. [1984] of $147 \pm 9 \text{ MPa}$ for a tensile stress duration of $1.07 \mu\text{s}$ to obtain a fracture toughness of $5.23 \text{ MPa m}^{1/2}$. (The latter is an upper limit as here K_{IC} is the stress intensity for the onset of crack growth.) Substituting into (A1) yields impact stresses of 18 and 118 GPa for the limestone and gabbro experiments. One-dimensional impedance match solutions using calcite [Kalashnikov et al., 1973] and the Centerville diabase [McQueen et al., 1967] equations of state with lead and steel yield impact pressures of 11 and 88 GPa. Considering the approximations made, the agreement is good and we conclude that (A1) yields a good approximation to the linear extent of shock damage.

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