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# Local site effects and incremental damage of buildings during the 2016 Central Italy earthquake sequence

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## ABSTRACT

The Central Italy earthquake sequence initiated on 24 August 2016 with a **moment magnitude** M6.1 event followed by a M5.9 and a M6.5 earthquake, that caused significant damage and loss of life in the town of Amatrice and other nearby villages and hamlets. The significance of this sequence led to a major international reconnaissance effort to thoroughly examine the effects of this disaster. Specifically, this paper presents evidences of strong local site effects (i.e., amplification of seismic waves due to stratigraphic and topographic effects that leads to damage concentration in certain areas). It also examines the damage patterns observed along the entire sequence of events in association with the spatial distribution of ground motion intensity with emphasis on the clearly distinct performance of reinforced concrete and masonry structures under multiple excitations. The paper concludes with a critical assessment of

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23 past retrofit measures efficiency and a series of lessons learned as per the behavior of  
24 structures to a sequence of strong earthquake events.

## 25 INTRODUCTION

26 Earthquake engineering has a strong theoretical foundation but is also an empirically  
27 driven discipline. As a result, post-earthquake reconnaissance efforts provide essential  
28 knowledge and help to improve our understanding of seismic events and their effects on the  
29 natural and built environment. Post-earthquake reconnaissance reports date back to several  
30 centuries ago. A pioneering example is the report by Sarconi dated back to 1784 on the  
31 seismic sequence of the year before in Calabria (Italy), in which several illustrations  
32 documenting the observed damage and particularly the diffuse liquefaction phenomena were  
33 presented.

34 The 2016 Central Italy seismic sequence caused significant damage and loss of human  
35 life with 299 casualties. Three main events occurred between August and October 2016: a  
36 **M6.1** on 24 August, a **M5.9** on 26 October, and a **M6.5** on 30 October. Remarkably, the  
37 event characterized by the largest magnitude earthquake (**M6.5**, 30 October) occurred when  
38 many villages were entirely abandoned following previous events. As a result, although it  
39 caused disruption in several villages over a large area, it did not cause any casualty.

40 After the **M6.1** event, a joint Italy-UK-USA team conducted a reconnaissance effort  
41 under the auspices of the Geotechnical Extreme Events Reconnaissance (GEER) association  
42 funded by the U.S. National Science Foundation (NSF), followed by a second reconnaissance  
43 mission in October to collect additional data on the cumulative damage of the building stock,  
44 earthquake-induced landslides/rockfalls and surface faulting features. GEER (2016; 2017)  
45 summarize main findings of both reconnaissance missions. This paper focuses on the  
46 observed damage to buildings, its spatial correlation in relation to the intensity of ground  
47 motion, including site effects, and the influence of multiple earthquake excitations on the  
48 extent and nature of the damage patterns observed for different structural systems. To serve  
49 this purpose, the paper is organized into three main parts as described below.

50 First, field mission organization, coordination, and activities are presented with emphasis  
51 on the methodologies and tools employed. Next, a study of the geological and topographic  
52 conditions of the surveyed municipalities and hamlets is presented with the aid of the analysis  
53 of a limited number of single station ambient vibration measurements (Horizontal-to-Vertical

54 Spectral Ratio method). Detailed site-response analyses are out of scope for the present study  
55 as they are currently in progress within the framework of the seismic microzonation studies  
56 that can be found elsewhere (CentroMS, 2016), however, evidences of local site  
57 amplification are described within the paper if observed during the surveys.

58 For three selected towns and villages, namely Accumoli, Amatrice, and Norcia, that were  
59 inspected both after the 24 August and the October events, a comparative assessment of  
60 quick visual inspections of their entire building portfolio is presented. Where available, a  
61 further comparison is made between on-site visual inspections made by the GEER team and  
62 the rapid assessment of damage released after each event by means of satellite data  
63 (Copernicus, 2016). The paper concludes with the lessons learned in terms of the effect of  
64 local soil and site conditions as well as of the cumulative damage caused by the sequence of  
65 the earthquake events.

## 66 RECONNAISSANCE APPROACH AND METHODOLOGY FOR DATA 67 COLLECTION

68 To better coordinate the GEER field missions, activities were designed to maximize use  
69 of resources and data as they gradually became available. The approach was to combine  
70 conventional field reconnaissance activities with advanced imaging and damage detection  
71 techniques enabled by information and communications technologies (ICT) and geomatics. A  
72 similar multi-scale reconnaissance approach has been implemented by the GEER team to  
73 document landslides (Franke et al., 201x – this issue). The steps followed during our  
74 reconnaissance effort are described below and illustrated in Figure 1:

75 *Initial planning of the field mission paths:* Identification of areas most significantly  
76 affected by earthquake-related damage, utilizing available post-event rapid-assessments of  
77 damage distribution based on satellite images, released after the earthquake event  
78 (Copernicus, 2016; Center for seismic microzonation and its applications – CentroMS, 2016;  
79 Advanced rapid imaging and analysis, ARIA, 2016a). Path optimization was based on: (1)  
80 Google Maps information regarding the accessibility of roads and (2) feedback from other  
81 GEER groups and local engineers that had visited the area previously.

82 *Use of unmanned aerial vehicles, UAVs (drones):* to map areas of affected residential  
83 buildings, churches, bridges, landslides and geotechnical systems.

84 *Conventional inspection:* on-ground, structure-by-structure visual inspection of buildings and  
85 other infrastructures in the selected areas.

86 *Database & GIS:* Creation of an *ad-hoc* developed Microsoft Access Database for filling-in  
87 the Italian quick inspection form, according to the AeDES guidelines (Baggio, 2007) for  
88 post-earthquake assessment of 1313 buildings consistently documented after the 24 August  
89 and the October events. Database fields include classification of the structural system,  
90 material, soil conditions, damage at a member level between slight (D1), moderate (D2-D3),  
91 and very heavy (D4-D5) damage levels and an automated procedure to assign a global  
92 damage index for each building based on a weighted average of individual element failures.  
93 Conventional hard copy forms were also filled-in for redundancy purposes.

94 *Back-tracking & Documentation:* A unique ID was assigned to each building along with the  
95 coordinates associated with a waypoint (path tracked with handheld GPS) for easy back-  
96 verification of position to each building. Storage of the geo-tagged photos taken on-site in the  
97 database matched with complementary pre-earthquake photos retrieved by Google Street  
98 View

99 *GIS:* Development GIS shapefiles containing the surveyed buildings footprints and the  
100 associated data from the database to visualize the spatial distribution of structural damage.

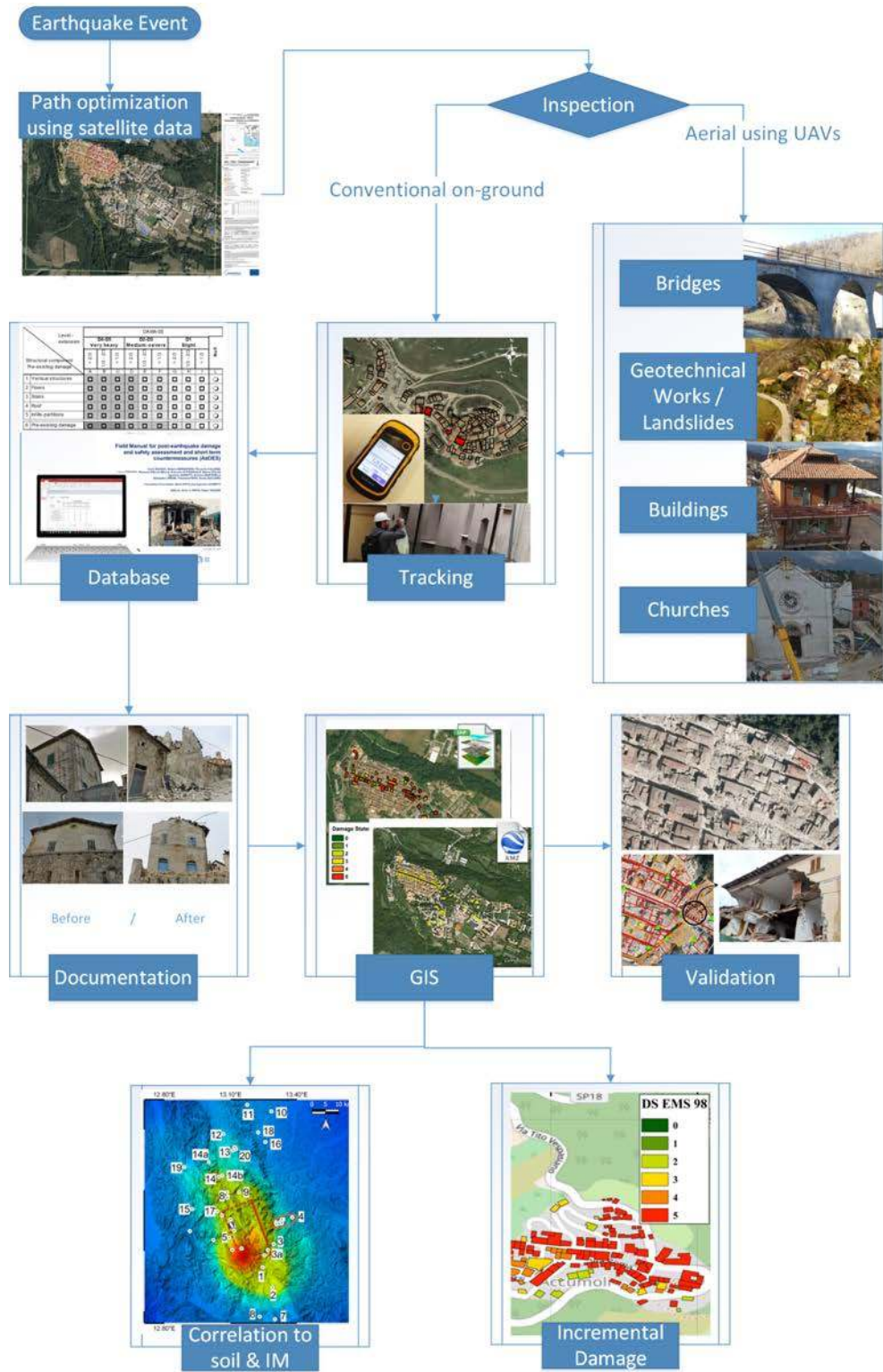
101 *Manual completion:* Population of the missing data for approximately 20% of the buildings  
102 for which detailed on-site visual inspection was not feasible due to accessibility issues, based  
103 on the existing photos, pre-quake and satellite images, drone footage (Sextos, 2016), and  
104 engineering judgment.

105 *Validation of satellite-based quick damage assessment:* Database validation to ensure that the  
106 observed damage was solely the result of earthquake excitation and not of any post-  
107 earthquake intervention (i.e. post-earthquake controlled-demolitions), through comparison  
108 with of the observed damage with Copernicus images that were taken closer to the event.

109 *Effect of multiple earthquake events:* Quantification of the damage evolution after multiple  
110 seismic events for different structural systems, i.e., reinforced concrete and masonry  
111 buildings.

112 *Correlation to ground motion intensity measures (IMs) and site effects recognition:*  
113 Correlation, where possible, of the observed damage with mapped geological information  
114 and preliminary analysis of the influence of site effect on structural damage patterns utilizing

115 rapid non-invasive *in-situ* investigation based on single station ambient vibration  
 116 measurements (HVSR method).



117

118

**Figure 1.** Overview of the reconnaissance strategy and organization.

## 119 SEISMIC SITE EFFECTS ON DAMAGE PATTERNS

120 Seismic site effects are usually associated with: (a) local ground response (also referred to  
121 as stratigraphic effect), (b) topographic amplification/deamplification, or (c) basin/edge  
122 effects. These phenomena are widely recognized in the literature (Roesset, 1970; Sanchez-  
123 Sesma, 1987; Seed et al., 1988; Frankel and Vidale, 1992; Olsen and Schuster, 1995).

124 Local ground response (i.e., stratigraphic effect) is mainly due to seismic wave  
125 propagation within near-surface soil deposits, where significant variations in amplitude,  
126 frequency content, and duration occur (e.g., Faccioli et al. 2002, Pagliaroli et al. 2011) as a  
127 result of stratigraphic and buried morphology features. Similarly, amplification of seismic  
128 waves due to topographic irregularities is an important cause of damage localization during  
129 seismic events (e.g., Bard and Riepl-Thomas 2000) as documented by several studies in Italy  
130 (Brambati et al. 1980, Siro 1982, Rovelli et al. 1998, Marsan et al. 2000, Paolucci 2002) and  
131 worldwide.

132 According to the Italian building code (Ministry of Infrastructure, 2008; hereafter NTC  
133 2008), these effects on ground motion are accounted for by multiplying the reference ground  
134 motion at the site with a deterministic amplification factor. The latter is derived from  
135 simplified classification parameters that are related respectively to: the averaged shear wave  
136 velocity of the upper 30m ( $V_{s,30}$ ), as per Eurocode 8 (CEN 2004, clause 3.1.2); shape of the  
137 site and slope inclination for topographic effects. This procedure is usually referred to as  
138 hybrid approach (Cramer, 2003). However, the combination of probabilistic hazard models  
139 with deterministic amplification factors, produce results that are biased in terms of medians  
140 and ground motion variabilities and do not preserve the target hazard level in the modified  
141 ground motion level (Gallipoli et al. 2013, Stewart et al. 2014, Stewart et al. 2017).  
142 Furthermore, comparisons between the hybrid approach and a more robust non-ergodic  
143 procedure (in which the effects of site amplifications are included within the hazard  
144 calculation) show that the former method tends to underestimate ground-shaking levels (i.e.,  
145 Goulet and Stewart 2009, Zimmaro et al., 2017).

146 To evaluate the spatial distribution of ground motion intensity measures during the  
147 studied sequence of earthquake events, Zimmaro et al. (201x, this issue) applied a Kriging  
148 procedure to within-event residuals (i.e. the difference between recorded and estimated  
149 ground motions using global ground motion models, for a specific earthquake event) for  
150 uniform reference site-conditions of  $V_{s,30}=580$  m/s (considered site class B according to NTC

151 2008) that were deemed representative of this region. The first step of this approach is to  
152 calculate within-event residuals at all recording station sites, using the average of the  
153 following Italy-adjusted global ground motion models: Boore et al. (2014), Campbell and  
154 Bozorgnia (2014), and Chiou and Youngs (2014). Then, the spatial distribution of a given  
155 intensity measure is estimated using the Jayaram and Baker (2009) global correlation model  
156 (i.e. a semi-variogram that describes the spatial variability of a given ground motion intensity  
157 measure throughout the area). All source-to-site distance were calculated using trimmed  
158 finite fault models presented in Galadini et al. (201x, this issue). The Italy-specific regional  
159 adjustment adopted in these models is needed to capture a relatively steep ground motion  
160 attenuation with distance observed in Italian events (e.g. Stewart et al., 2012). The  
161 effectiveness of the adoption of global models with region-specific adjustments for ground  
162 motion characterization studies in Italy, has been recently illustrated by Zimmaro and Stewart  
163 (2017). Further details on the approach used to estimate the ground motion are provided in  
164 GEER (2017) and Zimmaro et al. (201x, this issue). Following this approach, ground motion  
165 intensity estimations for the three main shocks were obtained for a grid of sites in the  
166 epicentral area, as well as for hamlets, towns, and cities for which co-located recording  
167 instruments were not available (i.e. where no recording stations were available or they did not  
168 record the events).

169 Figure 2 shows the spatial distribution of peak ground acceleration (PGA) for the three  
170 main shocks. In Table S1, a summary of PGA values for visited locations along with a  
171 detailed analysis of site-specific geological conditions is also provided. Main municipalities  
172 and hamlets covered in this paper are labeled in Figure 2, with a sequence number consistent  
173 with those reported in Table S1. It is important to note that the contour map showing spatial  
174 distribution of PGA shown in Figure 2 and the PGA values at selected locations summarized  
175 in Table S1, do not properly account for local effects since uniform generic site conditions  
176 were assumed for the entire area. Furthermore, each damage level value in Table S1  
177 represents an average damage level in the villages, while intra-village damage patterns are  
178 discussed in a subsequent section.

179 The estimated values of PGA at each inspected village are compared in Table S1 with the  
180 average damage level documented during the reconnaissance. The damage was classified on  
181 the basis of visual inspections of buildings following the scheme provided by the Department  
182 of Civil Protection (DPC) in Italy for post-earthquake reconnaissance purposes. As shown in



183 Table 1, the damage scale ranges from D0 which denotes “no observed damage” to D5 that  
 184 corresponds to collapse (EMS 98, Grunthal, 1998; Bray and Stewart, 2000). Moreover,  
 185 synthetic descriptions of topographic features of each visited municipality are reported in  
 186 Table S1.

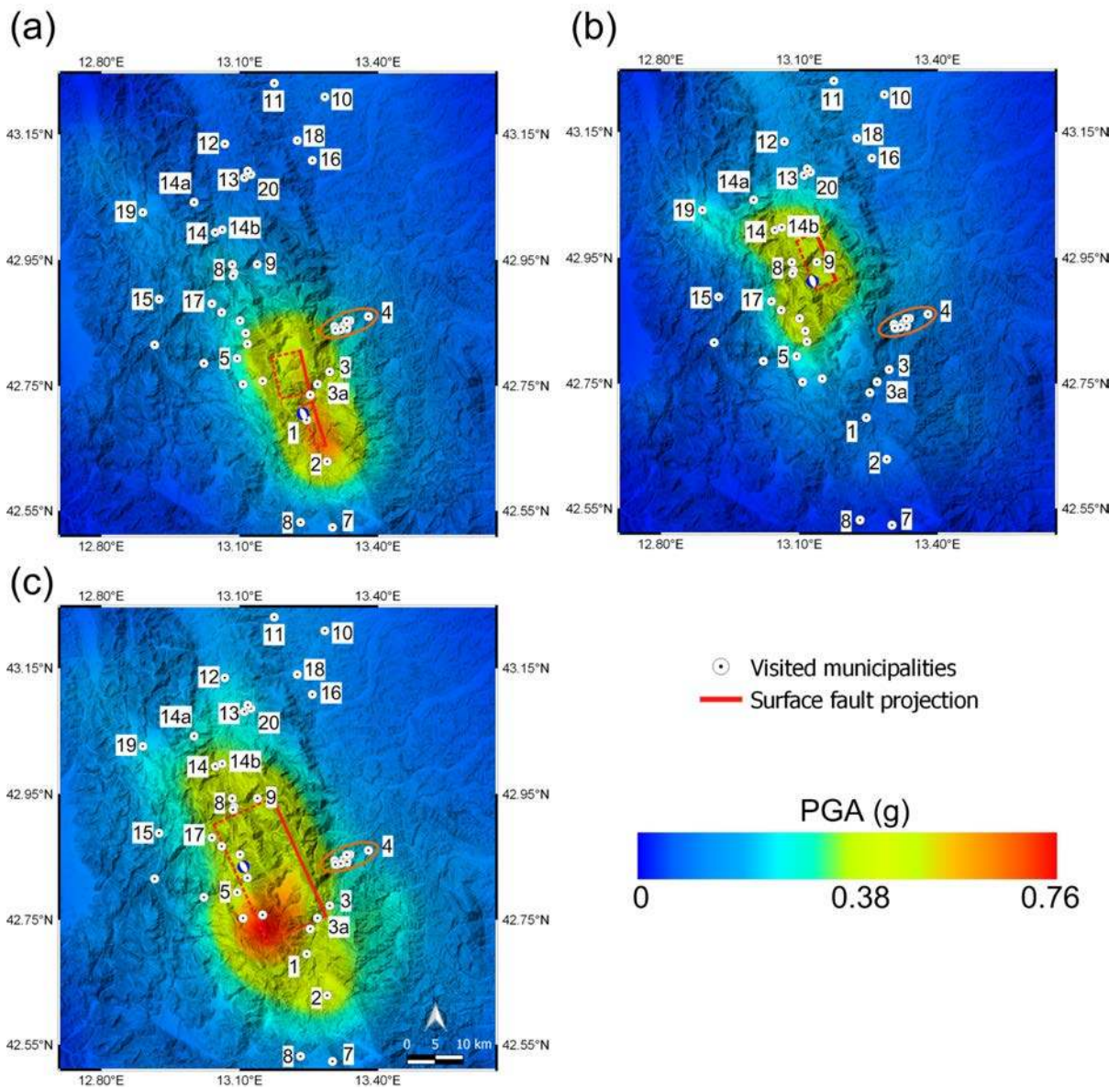
187 In the following section, selected examples of local site effects at several locations are  
 188 shown. The main goal is to identify if structures that can be considered homogeneous and  
 189 therefore equally vulnerable (i.e., same age, structural system, etc.) have been affected in  
 190 different manner by the specific site conditions with respect the final observed damage.  
 191 Therefore, the following observations are intended to highlight only the effects of ground  
 192 motion spatial variability across villages due to specific stratigraphic and topographic  
 193 configurations. Incremental structural damage assessment after different shocks is presented  
 194 later.

### 195 Montegallo

196 Montegallo is a village composed of 23 small hamlets spread over a large area. It is  
 197 characterized by an altitude varying significantly from the hamlet of Uscerno (i.e., 494m  
 198 A.S.L.) to the highest peak of Colleluce at 1023m.

199 **Table 1.** Definition of damage classification (adapted from Bray and Stewart, 2000).

| Damage Level | Description   | Tag Color    |
|--------------|---|--------------|
| D0           | No Damage   | Dark Green   |
| D1           | Cracking of non-structural elements, such as dry walls, brick or stucco external cladding   | Medium Green |
| D2           | Major damage to the non-structural elements, such as collapse of a whole masonry infill wall; minor damage to load-bearing elements | Light Green  |
| D3           | Significant damage to loading-bearing elements, but no collapse   | Yellow       |
| D4           | Partial structural collapse (individual floor or portion of building)   | Orange       |
| D5           | Full collapse   | Red          |



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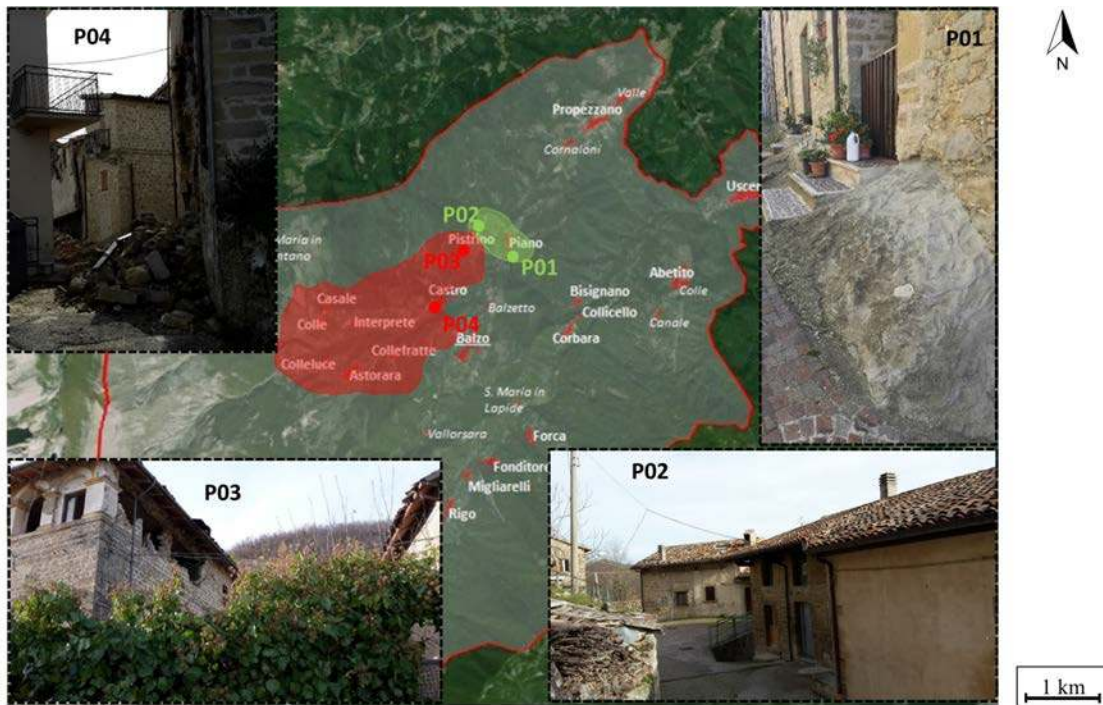
201 **Figure 2.** Location of visited municipalities and hamlets, epicenter locations (moment tensors), and  
 202 spatial distribution of PGA for the: (a) 24 August **M6.1**, (b) 26 October **M5.9**, and (c) 30 October  
 203 **M6.5** earthquakes. **Numbers in Figure 2 are those presented in Table S1.**

204 The geology of Montegallo is characterized by eluvial-colluvial deposits consisting of  
 205 silty sand and mixtures of silt and sand, as well as alluvial terraced deposits (Figure S1). The  
 206 bedrock is a turbiditic succession known as Laga Flysch mainly composed of arenaceous and  
 207 arenaceous-pelitic lithofacies. However, specific geologic-topographic characteristics widely  
 208 vary across the area, leading to a significant heterogeneity in damage patterns even for  
 209 buildings with apparently similar structural type and vulnerability.

210 An evidence for ground shaking variability is the undamaged hamlet of Piano in the NNE  
 211 area of Montegallo. Despite examples of poorly constructed masonry buildings, there was no

212 sign of evident damage at the end of the seismic sequence. For Piano, it is expected the  
213 absence of stratigraphic amplification given the visible outcropping rock in this area (Figure  
214 3-P01). A second example is a slight damage (i.e., D0-D2) observed in the hamlet of Pistrino  
215 di Sotto (Figure 3-P02), which is less than 500m away from Piano, on the opposite side of the  
216 NNE hill. It is also arguable that Pistrino di Sotto is resting on shallow bedrock conditions.  
217 These geologic conditions, combined with the relatively high natural frequency of the site,  
218 likely did not produce significant amplification of the ground motion. On the contrary, the  
219 adjacent hamlet, Pistrino di Sopra (Figure 3-P03), presented a significant level of damage,  
220 most likely associated with the presence of a soft cover of elluvial-colluvial deposits. These  
221 conditions are typical of the area, as shown in Figure S1.

222 Other Montegallos hamlets, such as Astorara, Castro, and Colleluce in the southwestern  
223 part of the area at a distance of 1.5 to 2.5km from Piano, located on quaternary deposits  
224 resting on rock, experienced high levels of damage and several cases of total collapse (D5).  
225 For example, Figure 3-P04 shows a street in Castro that was blocked by the debris of a  
226 damaged building. Given the proximity between Castro (highly damaged) and Piano  
227 (practically undamaged), and the very similar structural systems and construction standards,  
228 it is probable that Castro experienced stronger ground motions than Piano, due to significant  
229 topographic amplification. A view of the 3D model obtained with a drone survey over the  
230 entire area can also be found in BYU-PRISM (2016). It shows the typical crest configuration  
231 of the zone, leading to possible 2D topographical effects.

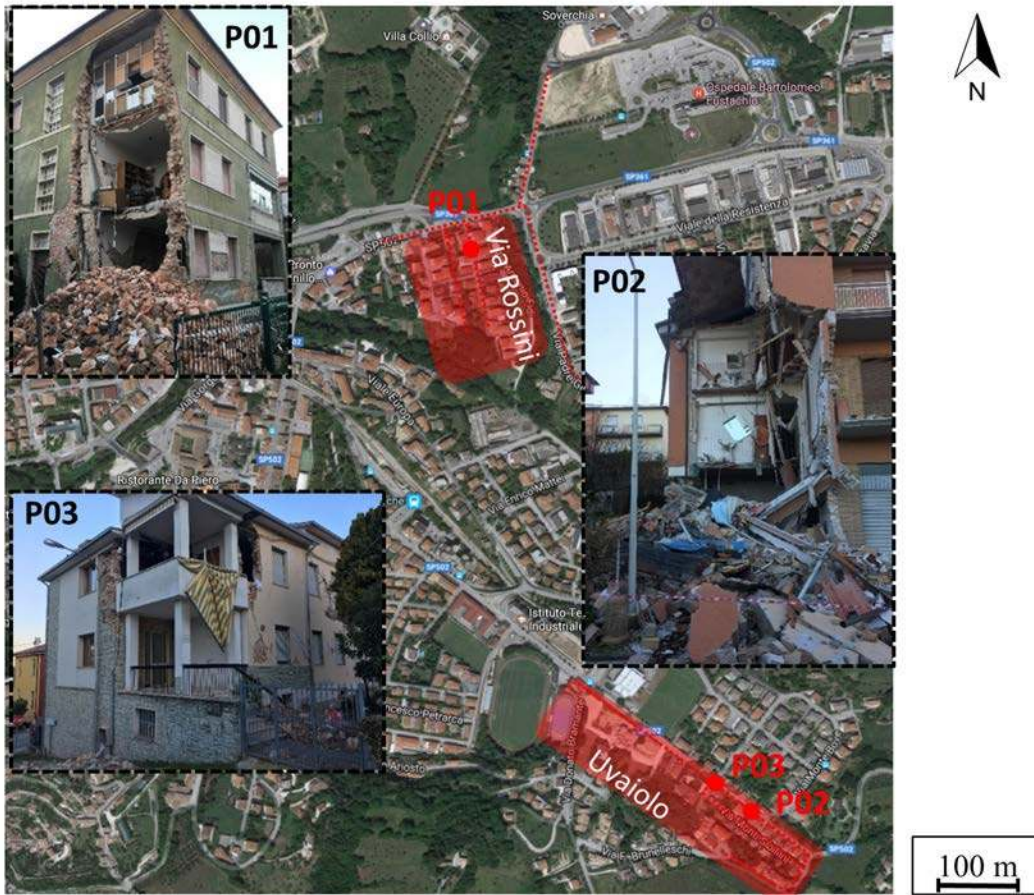


232

233 **Figure 3.** Spatial distribution of building damage across the municipality of Montegallo.

234 **San Severino Marche**

235 Other examples of local site effects were identified in some areas of San Severino Marche  
 236 (number 11 in Figure 2). San Severino Marche is a town in the Province of Macerata, in the  
 237 Marche region, located about 50 kilometers south-west of Ancona and about 25 kilometers  
 238 south-west of Macerata. It has about 12,000 inhabitants, and it comprises more than 40  
 239 hamlets. Unlike Montegallo, San Severino has districts where most of the buildings are of  
 240 reinforced concrete, built in the 1960s and the 1970s. Within San Severino Marche, two  
 241 neighborhoods along Via Mazzini and Via Rossini attracted most of the GEER  
 242 reconnaissance team attention due to the evident and quite localized damage observed  
 243 (Figure 4). Via Mazzini is located uphill while buildings along Via Rossini are constructed  
 244 on the ancient riverbed of the Potenza River. It is deemed that stratigraphic amplification is  
 245 likely to have taken place due to the presence of soft shallow sediments resulted from the  
 246 river artificial channeling operations. Similar damage patterns and site effects have been  
 247 observed in Tolentino (number 10 in Figure 2), as described in GEER (2017).



**Figure 4.** Characteristic building damage within the town of San Severino Marche.

248  
249

250 **Fiume**

251 Fiume is a hamlet in the province of Macerata (Marche region) and is approximately 4  
 252 kilometers away from the town of Pieve Torina. An extract from the 1:10.000 geological map  
 253 is given in Figure S2. The geologic bedrock of the area of interest is characterized by Scaglia  
 254 Cinerea, a grey marly limestone (SCC). The western part of the hamlet of Fiume is built on  
 255 Holocene travertine, travertine plaques and calcium carbonate-encrusted (MUSf1), i.e.,  
 256 materials that are typically tender and crumbly. On the contrary, the Eastern part of the  
 257 village is built on softer deposits constituted by Holocene eluvial-colluvial deposits  
 258 (MUSb2), recent alluvial deposits, mainly made of silts and sandy clay intercalated with marl  
 259 and limestone (MUSb) and debris flow deposits, mainly limestone debris and gravels with a  
 260 silty-sandy matrix (MUSa).

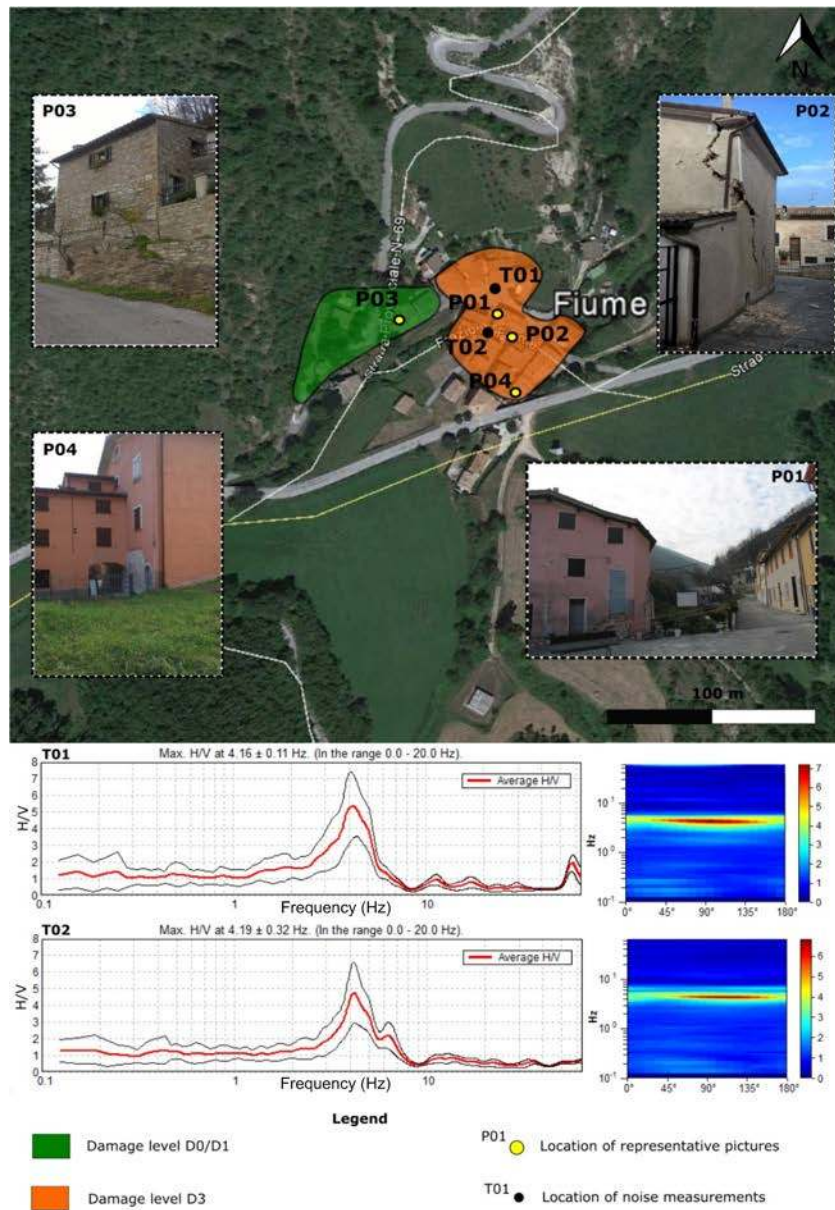
261 The Fiume building stock consists mainly of low-rise unreinforced masonry structures,  
 262 some of which retrofitted to some extent. Locations and pictures of representative structures  
 263 inspected in Fiume are reported in Figure 5 illustrating the severe and extensive damage.  
 264 Notably, the degree of damage to buildings was highly variable across the village. The

265 eastern part of the hamlet, founded on colluvial and alluvial deposits resting on bedrock,  
266 suffered high levels of damage (D3) as shown in reference pictures P01-P02-P04, whereas  
267 the western part, built on travertine rock, had only negligible damage (D0/D1, P03).

268 Two noise measurements (T01-T02 in Figure 5) were performed in the damaged zone  
269 (east side of the hamlet) during the GEER mission. A portable Tromino tomograph was  
270 employed and the total duration of each measurement was approximately 15 minutes.  
271 Horizontal-to-Vertical (H/V) spectral ratios were computed by using the geometrical mean of  
272 horizontal components. In addition, H/V ratios were computed by rotating the horizontal  
273 component between  $0^\circ$  and  $180^\circ$  (directional or polar HVSR), in order to investigate  
274 preferential directions of site amplification (i.e., the polarization of ground motion). Both  
275 H/V and polar H/V are reported in Figure 5 showing a large H/V peak around 4 Hz, which  
276 shows significant stiffness contrast between the upper soil layers and the underlying bedrock,  
277 i.e. a typical proxy of local site amplification.

## 278 **Visso**

279 Located in a valley 607m A.S.L. and surrounded by mountains of the National Park of  
280 Monti Sibillini, Visso is a municipality in the Marche region with a population of 1,100  
281 people living in 13 hamlets covering a wide area of approximately 100km<sup>2</sup>. The geological  
282 setting of the area is shown in Figure S4. The outcropping formations belong to the  
283 Cretaceous Miocene basinal succession made of, from bottom to top, Scaglia Rossa Fmt  
284 (SAA), Scaglia Variegata Fmt (VAS) and Scaglia Cinerea Fmt (SCC), Bisciaro Fmt (BIS).  
285 They are organized in a monoclinial architecture striking from NNW-SSE to N-S, and dipping  
286 to W with low-to-moderate angles and crossed by normal fault systems, mainly striking NW-  
287 SE. From a morphological viewpoint, Visso is located in a depressed area of the Sibillini  
288 Mountains, driven by quaternary normal faults, where the basinal successions are covered by  
289 quaternary alluvial and eluvio-colluvial sediments, and widespread slope deposits. The  
290 thickness of the covering layer varies from few meters to 40m, reached below the more  
291 recent urbanized area of Visso (Figure S4).



292

293 **Figure 5.** Damage zonation within the village of Fiume (up). Location and results of noise  
 294 measurements in terms of H/V spectral ratio (bottom left) and polar plot (bottom right).

295 Most of the buildings in Visso are unreinforced masonry structures, while a limited  
 296 number of reinforced concrete buildings is also present. These structures are mainly 2 to 3  
 297 stories, mostly built before the 1920s. The damage distribution, detected during the GEER  
 298 site-inspection after the M6.5 30 October event, is superimposed on the geological map in  
 299 Figure S4. As expected, buildings with most damage were 2 to 3 stories, unreinforced  
 300 masonry structures (sometimes recently retrofitted), mainly located in the historical center  
 301 (red line in Figure 10). Site amplification effects likely occurred, since most damage (level

302 D3-D4) was concentrated in the buildings founded on the quaternary continental deposits,  
303 while minor damage (level D1-D2) occurred in the portion founded on the SCC rock.

304 As anticipated, better performance (D2-D3) was detected for the reinforced concrete  
305 structures outside the historical center, despite their placement on the quaternary deposits, an  
306 observation that is in line with the detailed building-by-building inspection of other towns  
307 described in the following sections.

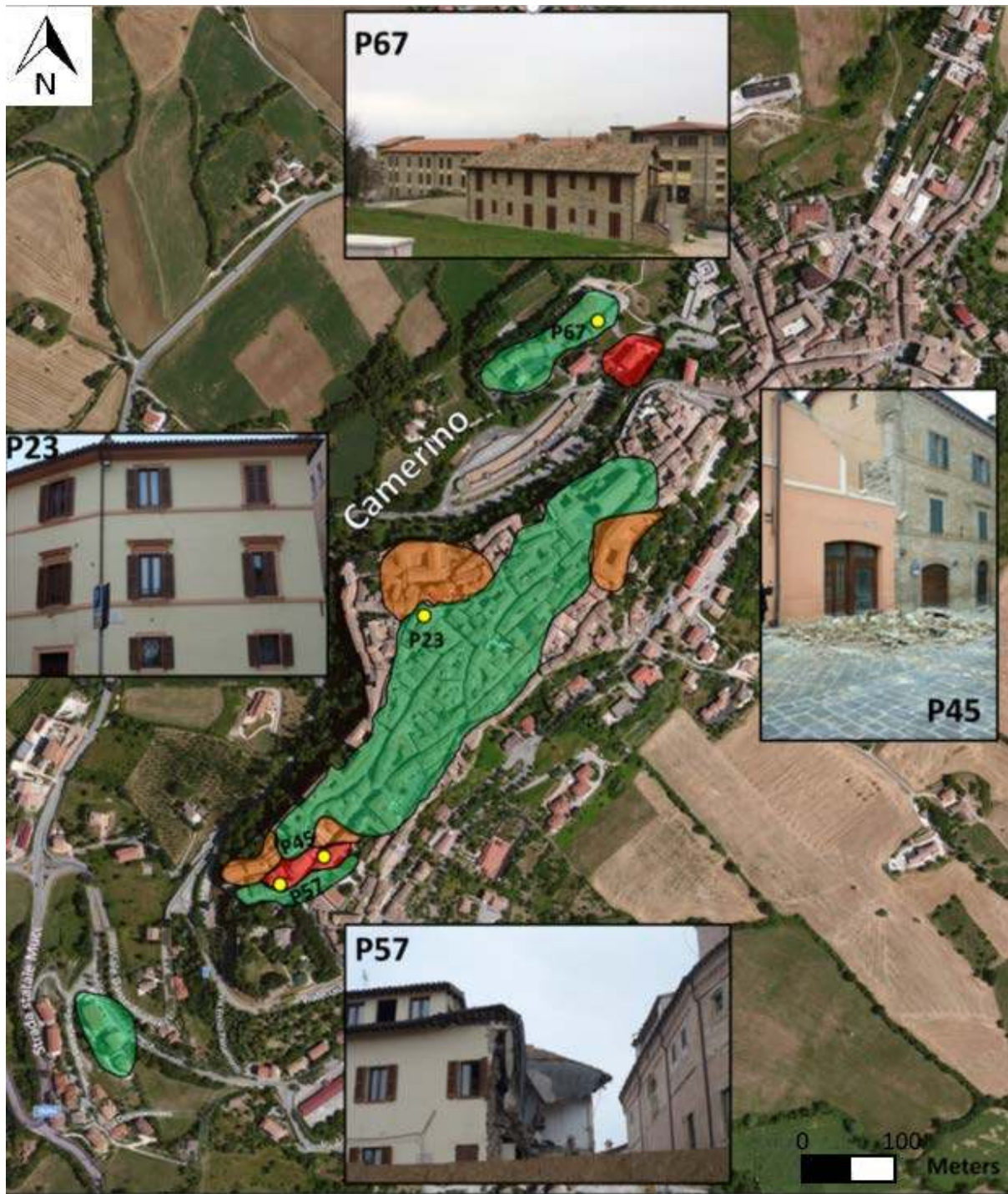
### 308 **Camerino**

309 Camerino is a village with 43 hamlets of about 6,986 inhabitants, located in the province  
310 of Macerata. The reconnaissance activity focused on the historic center where almost 50  
311 buildings were inspected.

312 The bedrock in the area consists of a typical alternation of arenaceous and pelithic-  
313 arenaceous lithofacies (ALS), sometimes with clayey-calcareous marl (COS), called “Scaglia  
314 cinerea” and “Schlier”. The above formations are locally covered by eluvio-colluvial soils  
315 (ML in Figure S5), made of silt or low-plasticity clay, or alluvial soil (GM) in the valley. The  
316 historic center is placed on the above layered arenaceous formation (GRS) referred to as  
317 “Formazione delle Arenarie di Camerino” (blue zones) (Figure S5). Where the bedrock is  
318 covered by thin layers of eluvio-colluvial soils (ML), ground motion amplification may be  
319 expected due to the high impedance contrast.

320 Figure 6 depicts the damage distribution across the main village, as inspected after the 30  
321 October event. Relatively low damage (D0 or D1) were observed within the inner part of the  
322 ridge characterized by local bedrock (GRS) outcrops. Higher damage levels (D2-D3) were  
323 observed for many of the low rise (2-3 stories) unreinforced masonry buildings, even if some  
324 of them were partially retrofitted. The damage is mainly localized on the hillside, where  
325 potential topographic amplifications and permanent deformation (due to slope instability)  
326 may be occurred. The highest damage level (D4) was observed at the SW side of the historic  
327 center and at the bottom of the Camerino hill, where several masonry structures collapsed.  
328 The observed damage distribution pattern in Camerino is consistent with site effects that  
329 could be inferred from the geological map shown in Figure S5. Strong amplification of  
330 earthquake ground motions is highly probable given the thin soft layers of eluvio-colluvial  
331 soils (ML) overlying the bedrock.





332  
333  
334

**Figure 6.** Damage zonation within the historic center of Camerino with pictures of the representative structures inspected.

335

## INCREMENTAL STRUCTURAL DAMAGE

336 An effort was also made to study the performance and incremental damage of different  
337 structural systems under the entire sequence of the August and October events. To this aim,  
338 an almost complete building-by-building inspection was performed, after the first and the  
339 third mainshocks, in three municipalities: Accumoli, Norcia, and Amatrice.

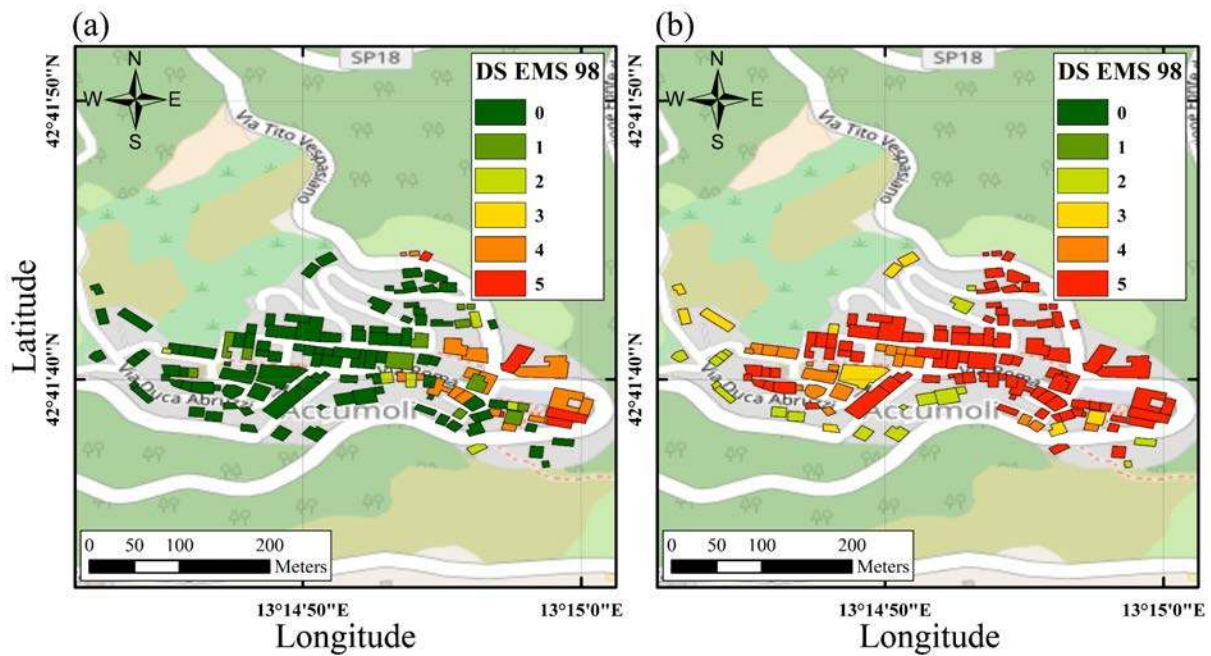
### 340 **Accumoli**

#### 341 *Soil conditions and building stock*

342 Accumoli is a small municipality in the Lazio region composed of seventeen hamlets  
343 covering an area of about 87.3 km<sup>2</sup>, with a population of about 670 inhabitants. The main  
344 village, which was one of the main targets of the surveys, is located on a steep slope of a  
345 ridge elongated in the direction WNW-ESE, with an altitude spanning between 810 and 890  
346 meters above the sea level. According to the 1:500,000 Italian geological map (Ministry of  
347 the environment, 2014), the geological bedrock is made of sedimentary lithology units  
348 composed of sandstones and clay lithofacies of the late Miocene. The vast majority of the  
349 entire building portfolio is composed of masonry residential buildings, with just a few  
350 reinforced concrete buildings. Approximately 8% of buildings are one-story, 42% are two-  
351 story, 43% three-story, and the remaining 7% are four-story or higher. According to the latest  
352 2011 census survey (ISTAT, 2011), 23%, 68% and 9% of the buildings were identified in an  
353 optimum, good, or acceptable conservation status, respectively. Most of these buildings  
354 (59%) were constructed before 1919, 32% between 1919 and 1945, 6% between 1946 and  
355 1960, 1% between 1961-70, 1% between 1971-80, and finally 2% between 1981-90.

#### 356 *Incremental damage observed*

357 Figure 7 illustrates the structural damage levels observed during the two surveys, after the  
358 24 August (left) and the October events (right). After the August 24th event, the most severe  
359 damage was observed at the eastern side of the village, while the vast majority of the building  
360 stock retained its structural integrity null or with minor damage (D0-D1). However, at the  
361 end of the seismic sequence, Accumoli was almost completely destroyed. Few buildings, in  
362 the south end of the village survived the sequence of events with limited damage (D2).



363

364 **Figure 7.** Damage levels in the main village of Accumoli (a) after the first earthquake and (b) at the  
 365 end of the entire sequence.

366 The evolution of structural damage **during the earthquake sequence** is clearly reflected in  
 367 the observed damage: 72% of the buildings experienced zero (DS0) and 8% minor damage  
 368 (DS1) after the first earthquake, while not a single building was found intact or with minor  
 369 damage after the seismic sequence. Large damage states were in contrast more populated (4%  
 370 to 13% for DS2, 0% to 7% for DS3, 12% to 14% for DS4, and a major shift from 4% to 65%  
 371 for DS5).

372 Figures 8a and 8b show an aerial view of the east part of the village during the first and  
 373 the second surveys, respectively, including the local church and the police station, which  
 374 eventually collapsed because of multiple earthquake excitations. **Figures 9, 10 and 11**  
 375 illustrate characteristic cases of minor-to-moderate shear and out-of-plane damage after the  
 376 August event that **led** to abrupt collapse because of the earthquake sequence. Age of  
 377 construction, high spectral accelerations for periods lower than 0.3s (which match the natural  
 378 periods of low-rise buildings) and the variation of spectral polarization across several events  
 379 were likely the main contributors to the observed catastrophic damage patterns. Given the  
 380 location of Accumoli, topographic effects may also have contributed to the observed damage.



381

382 **Figure 8.** Aerial photos of the east side of Accumoli after (a) the first earthquake and (b) the entire  
383 earthquake sequence.



384

385

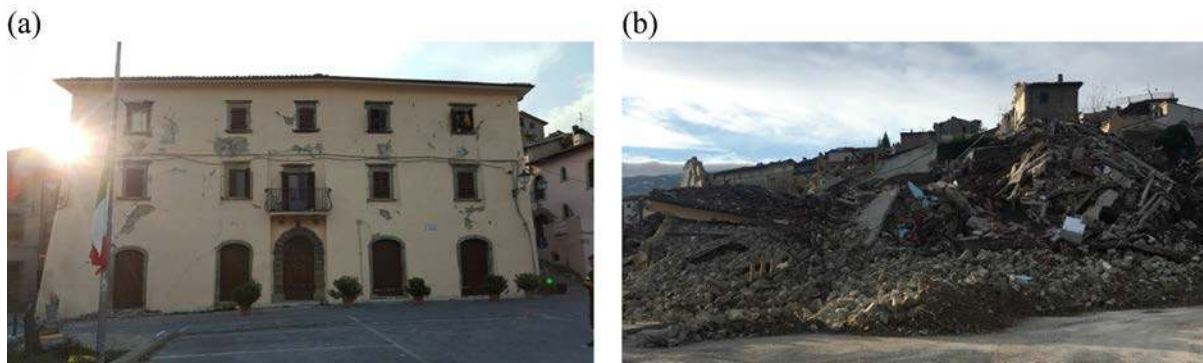
**Figure 9.** The local church: (a) after the first earthquake; and (b) after the entire sequence.



386

387

**Figure 10.** Masonry residential building: (a) after the first earthquake; and (b) after the sequence.



388

389 **Figure 11.** The town hall: (a) after the first earthquake; and (b) after the entire sequence.

390 **Amatrice**

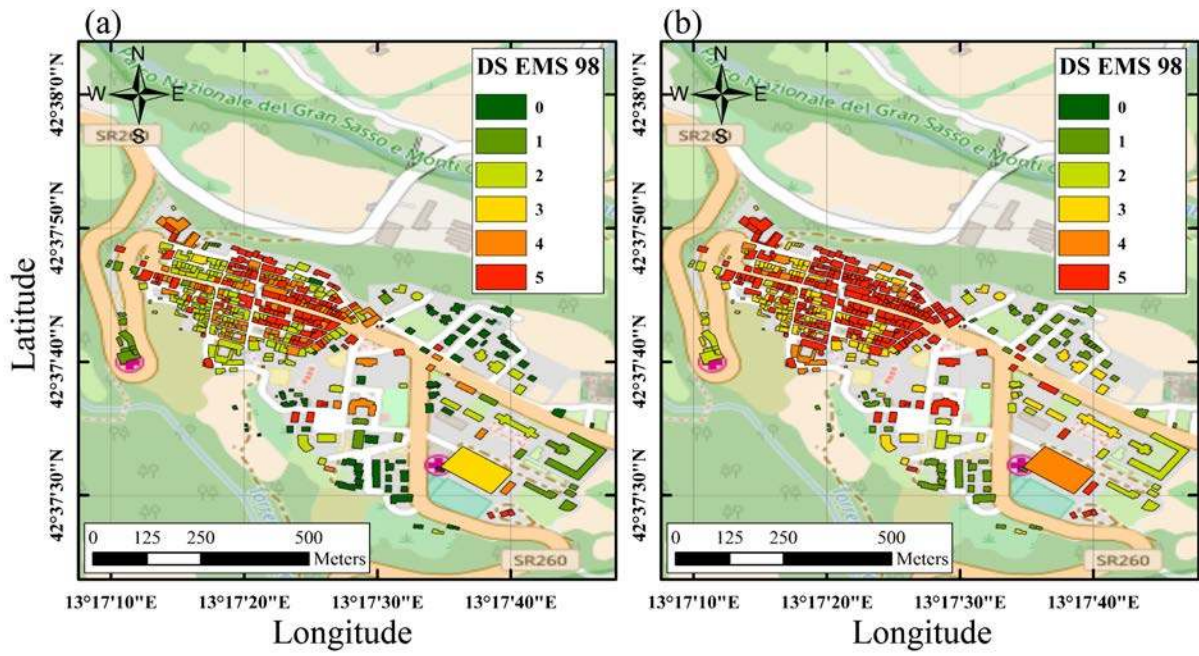
391 *Soil conditions and building stock*

392 Amatrice is a municipality in the Lazio region. It is composed of forty-nine hamlets  
393 covering an area of about 174.4 km<sup>2</sup>, with a population of about 2,630 inhabitants. The town  
394 is located on the edge of a hill, with an altitude spanning between 925 and 950 meters. The  
395 soil conditions in the area of Amatrice consist of sedimentary lithology units, sandstones and  
396 clay lithofacies of the late Miocene. The total number of the buildings inspected over the two  
397 field missions was 491, 77% of which were masonry structures for residential purposes. The  
398 remaining 11% and 13% are made of reinforced concrete and other structural typologies (i.e.,  
399 steel, timber, etc.), respectively. Most of the buildings are two stories (48%), while 41% are  
400 three-story, 8% one story and the remaining 5% four-stories or higher. According to the latest  
401 2011 census survey (ISTAT, 2011), the 29%, 53%, 14%, and the 3% of the buildings were  
402 assessed having an optimum, good, acceptable, and unacceptable conservation status,  
403 respectively. The distribution of the building age is as follows: 22% were built before 1919,  
404 24% in between 1919-1945, 13% between 1946-60, 23% between 1961-70, 11% between  
405 1971-80, 4% between 1981-90, 3% between 1990 and 2000, and only 1% after 2005. Hence  
406 only about 4% of the entire stock was designed complying with modern seismic codes.

407 *Incremental damage observed*

408 Figure 12 shows the structural damage levels observed during the two surveys. The 24  
409 August event caused severe damage to the south-east part of the historical city center along  
410 the main avenue (Corso Umberto I). As observed in the case of Accumoli, many buildings  
411 that were still standing after the first event with only a small residual capacity to additional  
412 horizontal actions, fully collapsed because of the subsequent September and October events.  
413 The **shifting** of damage states between the aftermath of the first event and the end of the

414 entire sequence is reflected in the following inspection results clearly indicating a major shift  
 415 to most critical damage states: intact buildings (D0) were reduced from 30% to 18%,  
 416 buildings with minor damage (D1) were increased from 5% to 10%, moderate damage (D2)  
 417 was reduced from 24% to 6%, D3 increased from 1% to 21%, D4 decreased from 17% to 3%,  
 418 and collapsed buildings (D5) had a significant increase from 23% to 42%.



419  
 420 **Figure 12.** Damage levels observed in the center of Amatrice (a) after the 24 August earthquake  
 421 (during the first survey), and (b) after the entire sequence (during the second survey).

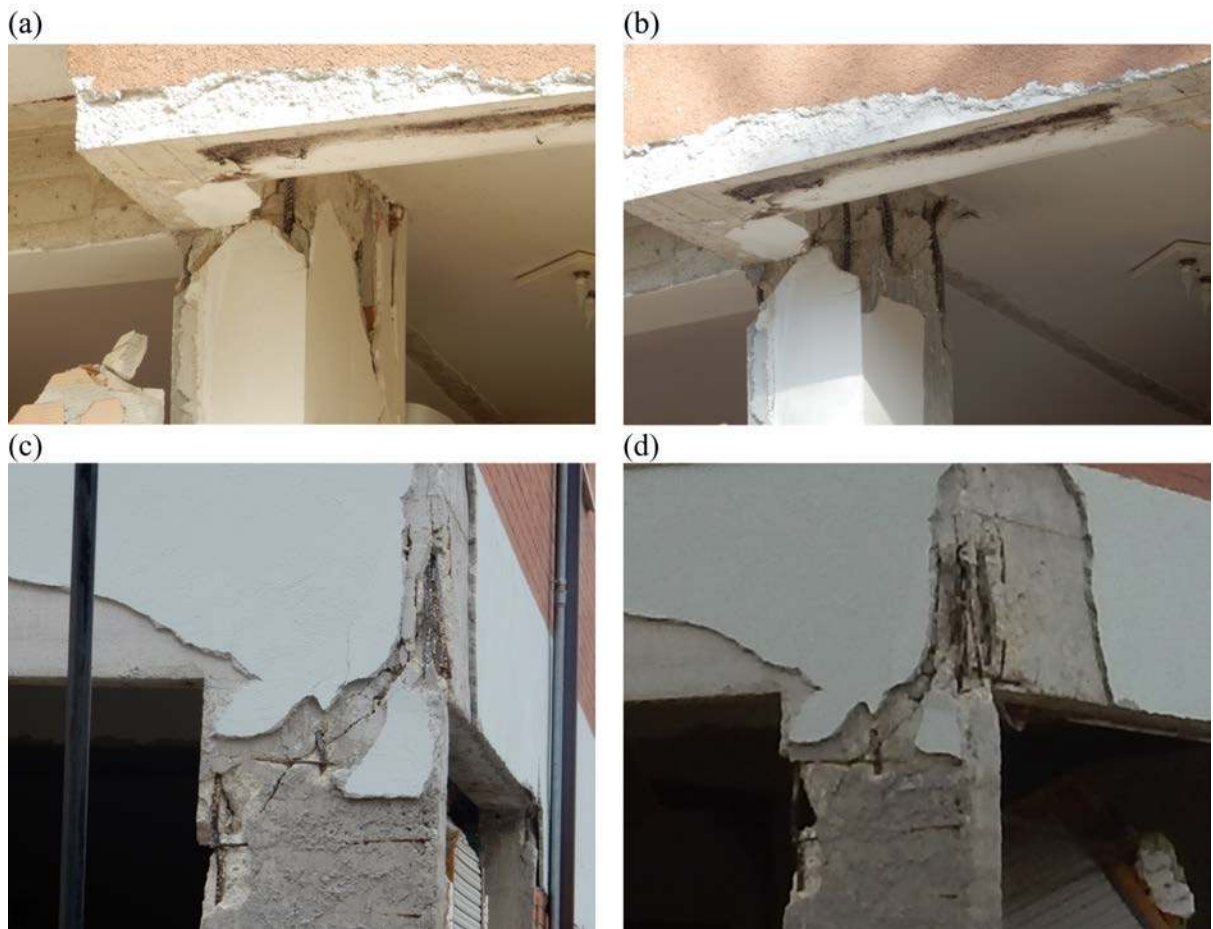
422 Even though the statistical sample of the reinforced concrete buildings was not adequate  
 423 to quantify how damage accumulates for different structural systems under multiple  
 424 earthquakes, an effort was made to compare characteristic cases at least qualitatively. An  
 425 example of a reinforced concrete building is illustrated in Figure 13. The partial out-of-plane  
 426 collapse of an external infill panel after the first event was followed by complete failure at the  
 427 end of the entire seismic sequence. A closer inspection of the top right beam-column joint  
 428 further reveals shear damage that was magnified, though not considerably, under multiple  
 429 excitations, i.e. the reinforced concrete structure retained some of its capacity thus avoiding  
 430 collapse. A similar example is shown in Figure 14. Cyclic degradation, concrete spalling and  
 431 minor longitudinal rebar buckling were indeed observed in the absence of adequate  
 432 transverse reinforcement, however, **global** damage state remained constantly moderate  
 433 despite the multiple earthquake events. In some cases, damage accumulation was more  
 434 significant, as for instance, in the building depicted in Figure 15, where minor damage after

435 the 24 August event propagated to the major out-of-plane failure of the majority of its infill  
436 panels, plastic hinge formations at the end of the exposed column and a degree of residual  
437 drift. However, the collapse was prevented. To the Authors' best knowledge, only one  
438 reinforced concrete building in Amatrice that was damaged by the 24 August earthquake  
439 eventually collapsed in the aftermath of the 26 October event. This structure was a seven-  
440 story building with external red curtain walls. More details about the performance and the  
441 exact location of this building are discussed in GEER (2017). An interesting case of a multi-  
442 story building that survived the multiple seismic excitations within Amatrice's historical  
443 center, is a steel structure (Figure 16) built in the early 90's following the 1996 Italian  
444 seismic code (Ministry of Public Works, 1996).



445

446 **Figure 13.** Reinforced concrete residential building (a,c) after the 24 August earthquake and (b,d)  
447 after the entire sequence. (a,b) External infill failure and (c,d) shear failure at the column top.



448

449 **Figure 14.** Beam-column joints. Concrete spalling and local bar buckling due to lack of transversal  
 450 reinforcement after the 24 August event (a,c) and after the earthquake sequence (b,d).



451

452 **Figure 15.** Irregular in plan reinforced concrete residential building. (a) limited damage after the 24  
 453 August earthquake and (b) considerable non-structural damage at ground level, failure of the infill  
 454 panels and residual drift.





455

456 **Figure 16.** Steel residential building. (a,c) Limited damage after the 24 August earthquake and (b,d)  
 457 extensive damage of the infill panels at ground level with evident residual drift after the entire  
 458 sequence.

459 **Such a steel structure** consists of a basement, a ground floor, and two upper stories  
 460 alongside a shorter top story that serves as a penthouse. After the 24 August event, the  
 461 damage was mainly confined to the infill panels, with only small local flange instabilities  
 462 observed at the top of two front columns of the ground floor. At the end of the entire seismic  
 463 sequence, the building experienced permanent deformation along its longer direction, as  
 464 shown in Figure 16. Such permanent deformation was localized at the second level of the  
 465 building with a visible residual inter-story drift due to the relative positions of infills and  
 466 openings. Preliminary finite element analyses of the building confirmed that the fundamental  
 467 period of the structure is approximately equal to 0.75 sec. This was an uncoupled  
 468 translational mode along the long side, which was mainly attributed to the orientation of the  
 469 steel columns with their strong axes aligned with the short side of the building. Naturally,  
 470 residual drift developed along the longitudinal (weak) axis. Evolution of structural damage is

471 also clearly seen in several characteristic masonry structures, such as the church of  
472 Sant’Agostino (Figure 17, top), the local police (“Carabinieri”) station (Fig. 17, middle) and  
473 typical residential buildings (Fig. 17 bottom and Fig. 18).



474

475 **Figure 17.** Incremental structural damage of the church of Sant’Agostino (top), the local police  
476 station (middle) and one of the several masonry buildings collapsed after (a) the event of 24 August  
477 earthquake and (b) the entire sequence.



478

479 **Figure 18.** Residential masonry residential building after the 24 August earthquake (a, b) and (c) after  
480 the entire sequence. Shear failure of the ground floor bearing wall leads to soft story collapse at the  
481 end of the third event.

482 Several general conclusions can be drawn from the damage analysis in Amatrice.  
483 Notwithstanding the clear evolution of local damage modes of reinforced concrete structures  
484 under multiple earthquake excitations, they did not experience the disproportional damage  
485 increase observed in masonry buildings. In most cases, reinforced concrete buildings showed  
486 adequate ductility and their global damage remained approximately within the same damage  
487 state that was reported in the survey that followed the first earthquake. On the contrary,  
488 masonry buildings suffered, on average, significant damage accumulation during the  
489 sequence of seismic events due to their low residual capacity and the brittle nature of their  
490 out-of-plane and shear failure modes. This led to quickly shifting from low-to-moderate  
491 Damage States (DS1-DS3) to complete collapse (DS5) and demonstrated the **need** for careful  
492 inspection to reliably assess their residual capacity to withstand horizontal forces during  
493 **future shocks.** **The elevated level of damage for masonry buildings is mainly caused by the**  
494 **poor quality of masonry, the lack of connections between walls and the poor connection**  
495 **between external walls and floors, as also observed by Fiorentino et al. (2017).**

496 **Norcia**

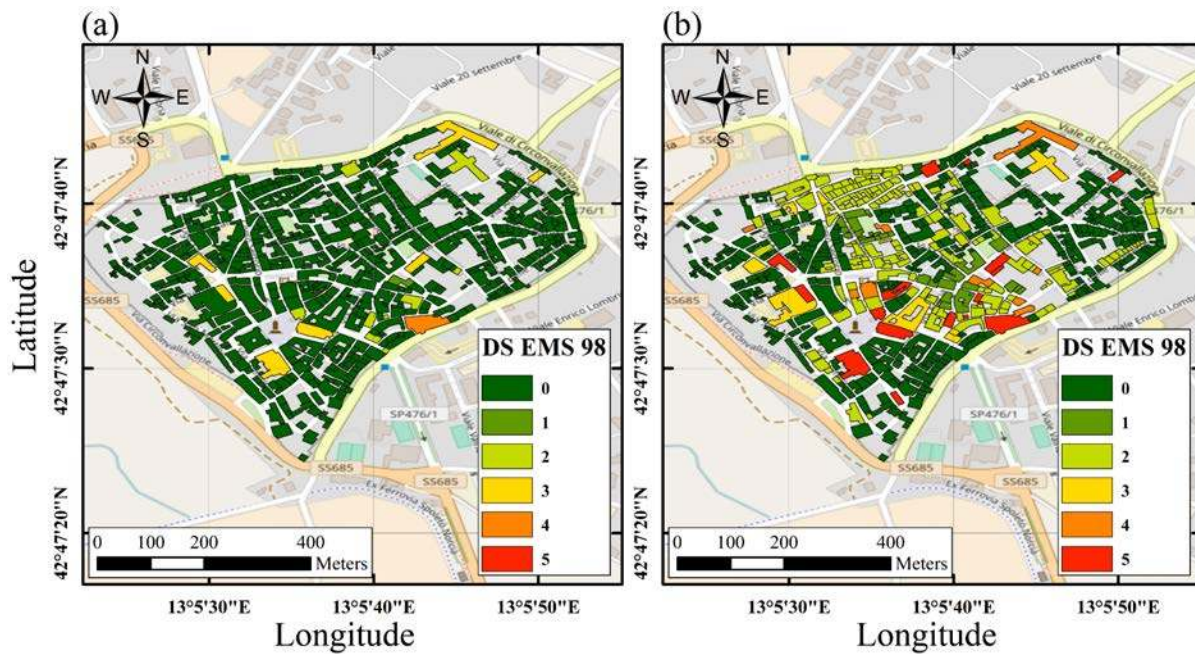
497 *Soil conditions and building stock*

498 Norcia is a municipality located on the border between the regions of Umbria, Marche,  
499 and Lazio. It is composed of 27 hamlets covering an area of about 274 km<sup>2</sup>, with a population  
500 of about 4,940 inhabitants. Its core is located within the historical walls, with an altitude  
501 spanning between 590 and 630 m. The bedrock is made of sedimentary lithology units  
502 composed of unconsolidated colluvial, terraced alluvial, fluviolacustrine and fluviglacial  
503 deposits of Pleistocene. The total number of buildings inspected in the surveyed area is 680,  
504 98% of which are masonry residential structures. The remaining 2% is equally distributed  
505 among the reinforced concrete and other structural typologies such as steel and timber. A  
506 mere 12% of these buildings have one-story, 74% two-stories, 13% three-stories, and the  
507 remaining 1% four-stories or more. According to the last 2011 census survey (ISTAT, 2011),  
508 the 44%, 53%, and the 3% of the buildings were assessed as of optimum, good, and  
509 acceptable conservation status, respectively, a fact that reflects the overall better quality of  
510 construction compared to Accumoli and Amatrice. The majority (67%) of the buildings were  
511 built before 1919, 3% in the time period between 1946 and 1960, 3% between 1961-70, 21%  
512 between 1971-80, 4% between 1981-90, and 1% between 1990-2000.

513 *Incremental damage observed*

514 Figure 19 shows the structural damage levels observed during the two inspection  
515 campaigns. Following the 24 August earthquake, only a small number of buildings  
516 experienced medium or severe damage, located mainly in the historical center of the town.  
517 This good performance can be primarily attributed to two reasons. First, after the 1859  
518 earthquake, the reconstruction of Norcia was based on a set of new practical rules of thumb  
519 prescribing a minimum wall thickness, the use of buttresses, the reduction of building height,  
520 the use of vaults only at ground floor and the mandatory presence of good wall-to-wall  
521 connections. The increased wall thickness is still visible in many structures, and in several  
522 buildings, the wall thickness varies linearly along the height of the first story. Secondly, a  
523 series of repair and strengthening works followed the 1997 Umbria-Marche event, **which**  
524 **improved** the capacity of sub-standard buildings. Such retrofits are generally not visible from  
525 outside, but confining ring-beams and cross-ties can be traced externally in many cases.  
526 Despite the **adequate** structural response of the buildings in Norcia during the 24 August  
527 event, a sharp increase of damage, yet not as disproportional as in the case of Amatrice, was

528 observed at the end of the seismic sequence, mainly in heritage construction such as churches  
 529 and monasteries. The following variation of cumulative damage was reflected in the  
 530 statistical distribution of the different damage states: intact buildings (DS0) were reduced  
 531 from 97% after the first earthquake to 67%, which was a substantial change in structural  
 532 behavior. Minor damage (DS1) also increased at the end of the entire sequence to 4% from  
 533 almost 0% after the first event. The same applies to moderate damage (DS2), it increased  
 534 from 1% to 24%, previously, and to DS5 increased from 0% to 3% in the first event, DS3 and  
 535 DS4 remaining practically constant.



536  
 537 **Figure 19.** Damage distribution in the historical center of Norcia (a) after the 24 August event and (b)  
 538 at the end of the entire seismic sequence.

539 Figure 20 (top) shows one of the churches that was slightly damaged by the M6.1 24  
 540 August seismic event but collapsed following the M6.5 30 October event. Many historical  
 541 churches in Norcia experienced similar damage evolution, as shown for instance in Figure 20  
 542 (middle), where the out-of-plane failure of a historic monastery and the partial loss of support  
 543 of the roof is depicted. Notably, the wall failure was concentrated at a level higher to that of  
 544 the seismic retrofit, thus highlighting that the retrofit shall not be only localized on the  
 545 ground level but also take into consideration the reduced axial load and weak diaphragm  
 546 action of the masonry walls at the higher level. Figure 20 (bottom) shows two masonry  
 547 residential buildings with irregular masonry construction that experienced only minor  
 548 cracking during the first earthquake, but significant out-of-plane and in-plane wall failure  
 549 under subsequent events.



550

551 **Figure 20.** Seismic damage observed in characteristic masonry buildings (a) after the 24 August  
 552 earthquake and (b) at the end of the entire seismic sequence.

553

554

555 **ON-SITE DAMAGE ASSESSMENT VERSUS NASA JPL ARIA DAMAGE PROXY MAPS**

556 Following major natural disasters, the Advanced Rapid Imaging and Analysis (ARIA)  
 557 project (ARIA, 2016a) typically publishes rapid post-disaster deformation maps. These maps  
 558 are produced comparing interferometric synthetic-aperture radar (SAR) coherence maps from

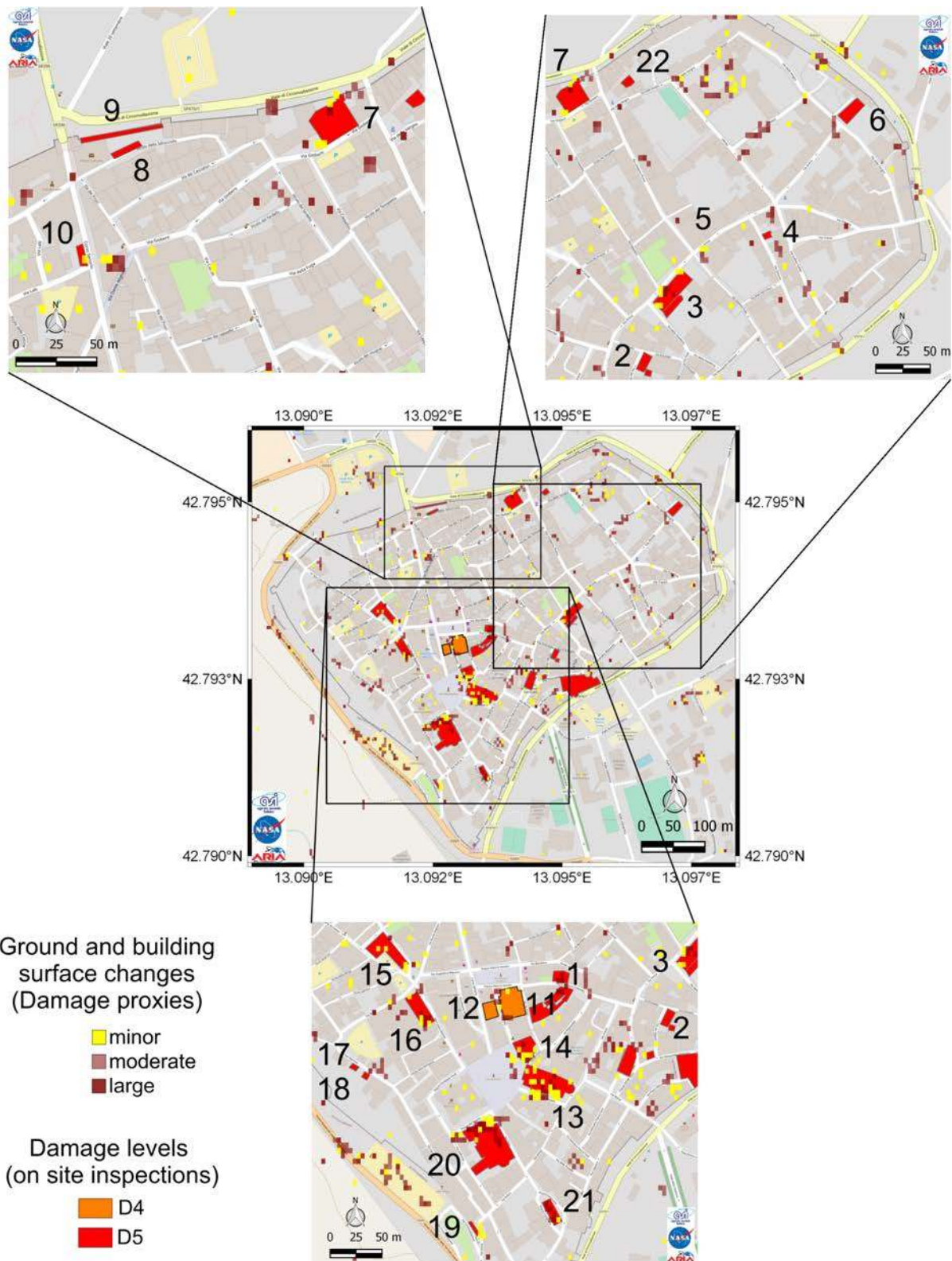
559 before and after an extreme event (e.g., Fielding et al., 2005; Yun et al., 2011). They are  
560 usually referred to as damage proxy maps (DPMs). In the aftermath of the **M6.5** 30 October  
561 event, the ARIA team published a damage proxy map (ARIA, 2016b) for the historical center  
562 of Norcia. This DPM covers an area of 6.2-by-6.2 miles (10-by-10 kilometers), and it has  
563 been derived using the Italian Space Agency's COSMO-SkyMed Spotlight synthetic aperture  
564 radar (SAR) data acquired from an ascending orbit.

565 The effectiveness of the DPMs was tested for the rapid evaluation of earthquake-induced  
566 landslides and rockfalls after the 2015 **M7.8** Gorkha Earthquake. In particular, Yun et al.  
567 (2015) showed that the extent of several observed earthquake-related instability phenomena  
568 in the Himalayas were well captured by the DPMs. Franke et al. (201x, this issue), also  
569 analyzed the effectiveness of DPMs after the **M6.1** 24 August central Italy earthquake for  
570 evaluating the spatial distribution of seismically-induced landslides and rockfalls.

571 The resolution of the DPM published following the **M6.1** 24 August event was too low to  
572 enable comparisons to our field observations of building damage. The DPM published  
573 following the **M6.5** 30 October event was centered on the historical center of Norcia. Given  
574 that this DPM had a relatively limited spatial extent but a high-resolution, detailed structure-  
575 by-structure comparisons of ARIA maps versus field observations were then possible. An  
576 effort was therefore made to investigate the degree of correlation between the DPM rapid  
577 imaging prediction and the actual assessment made by the members of the field mission on  
578 site.

579 Figure 21 shows the DPM produced for the historical center of Norcia after the **M6.5** 30  
580 October event, that is, the end of the earthquake sequence, superimposed with 22 structures  
581 that were classified visually as completely collapsed (D5), and selected D4 structures.

582 By comparing the locations of these mapped structures and the damage zones from ARIA  
583 imaging, a good agreement was observed. In particular, for all structures with an assigned  
584 damage level of collapse (D5), the DPM accurately showed a concentration of red and dark  
585 red zones, representing areas in which substantial deformations occurred.



586

587 **Figure 21.** Damage proxy map of Norcia, along with the identification numbers of all structures with  
 588 assigned damage level D5 and selected structures with assigned damage level D4, from field  
 589 inspections and available high-quality on-site information and photos.

590



591 This is further documented in Figure S6, which depicts representative pictures taken  
592 during the on-site inspection that followed the 30 October, M6.5 earthquake event. The  
593 extent and nature of damage to each spotted building, as illustrated in Figure S6, matches  
594 well the ARIA imaging prediction highlighting the usefulness of rapid aerial assessment of  
595 seismic damage during the post-earthquake recovery period.

## 596 **LESSONS LEARNED AND CONCLUSIONS**

597 The 2016 Central Italy seismic sequence caused significant damage and loss of life. Three  
598 main events occurred between August and October 2016: (a) M6.1 24 August, (b) M5.9 26  
599 October, and (c) M6.5 30 October. This paper presents the observations of two GEER field  
600 missions in the affected area with the aim to evaluate the influence of local site effects on the  
601 observed damage patterns of buildings and assess their structural performance after multiple  
602 seismic events. The first objective required an evaluation of geological and topographic  
603 conditions as well as ambient vibration measurements, where possible (H/V spectral ratios).  
604 The second objective required an extensive, building-by-building visual inspection campaign  
605 in the region and a comparative analysis of the observed damage patterns after the first main  
606 shock (M6.1, 24 August) and at the end of the October sequence of events.

607 In this process, our approach was to combine traditional reconnaissance methods (careful  
608 surveys by a team of experts on the ground) with advanced imaging and damage detection  
609 routines enabled by information and communications technologies (ICT) and geomatics  
610 approaches as well as aerial visualization with the aid of UAVs. In a number of cases, the  
611 damage was not detectable by satellite-based assessment alone, pointing to the importance of  
612 traditional on-site inspection complementing other advanced methods. For the historical  
613 center of Norcia, the damage zones from ARIA imaging (DPMs), however, compared well  
614 with damage maps obtained from on-ground surveys.

615 In general, the damage patterns in various municipalities and hamlets indicated a strong  
616 evidence of local site effects. Amplification of seismic waves due to stratigraphic effects in  
617 the near-surface soil deposits and due to topographic effects **was the main contributor** of  
618 structural damage concentration among portfolios of buildings with otherwise similar  
619 vulnerability. In addition to local site effects, the age of construction, the high-frequency  
620 content of the motions, and the variation of spectral polarization across several events further  
621 contributed to severe damage in several villages.

622 Another interesting observation was that the vast majority of the buildings showed a  
623 clear evolution of damage after multiple earthquake excitations irrespectively of their  
624 structural system. However, the degree of damage accumulation under repeated ground  
625 motions was different. For instance, reinforced concrete buildings did not experience  
626 disproportional damage under multiple events. These structures generally showed adequate  
627 ductility, and their damage at a systems level remained approximately constant after the first  
628 earthquake until the end of the sequence. Masonry structures, on the other hand, suffered  
629 significant damage during the first event and quite often experienced an abrupt collapse in a  
630 successive earthquake because of the rapidly reducing residual capacity and their brittle  
631 nature. Therefore, as shown in all three towns thoroughly examined (Accumoli, Amatrice,  
632 and Norcia), they quickly shifted from low to moderate damage states (D1-D2) to major  
633 damage (D4) and even collapse (D5) after the sequence of seismic events.

634 Local retrofit with steel ties at the corners of the upper story prevented further damage  
635 and collapse in a number of cases, particularly in Norcia where several structures had been  
636 strengthened in the last two decades. Local interventions limited on the ground level alone,  
637 however, were shown to be unsuccessful. The reduced axial load and weak diaphragm action  
638 of the masonry walls at higher levels also need to be considered during retrofit to prevent  
639 damage accumulation and possible collapse. Even though the three cases studied (Accumoli,  
640 Amatrice, and Norcia) are not directly comparable as they were exposed to different levels of  
641 ground shaking over the earthquake sequence, the overall assessment is that reinforced  
642 masonry performed significantly better than the unreinforced one and that simple measures  
643 such as ties and buttresses may be proven crucial to prevent structural collapse.

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652 extreme events.

653

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