

Magnitude and Frequency of Floods in Alabama

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ABBREVIATION AND CONVERSION FACTORS

Factors for converting inch-pound units to International System of Units (SI) and abbreviation of units.

Multiply	By	To obtain
inch (in)	25.4	millimeter (mm)
foot (ft)	0.3048	meter (m)
mile (mi)	1.609	kilometer (km)
square mile (mi^2)	2.590	square kilometer (km^2)
cubic foot per second (ft^3/s)	0.02832	cubic meter per second (m^3/s)
cubic foot per second per square mile [$(\text{ft}^3/\text{s})/\text{mi}^2$]	0.01993	cubic meter per second per square kilometer [$(\text{m}^3/\text{s})/\text{km}^2$]

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ABSTRACT

Equations have been developed by regression analyses for estimating floods for 2-, 5-, 10-, 25-, 50-, and 100-year recurrence intervals on natural streams with drainage areas up to 1,500 square miles in Alabama. Seven different hydrologic areas have been delineated. Separate equations using drainage area as the independent variable apply to five of the hydrologic areas. One hydrologic area cannot be regionalized and another area uses both drainage area and a storage factor in the equations. Flood runoff in one hydrologic area was found to be two to four times greater than other areas due to an impervious plain developed mainly on chalk and marl. Another hydrologic area has very low flood runoff due to a high infiltration rate that transmits water to the ground-water reservoir.

Equations for estimating flood magnitude and frequency of urban streams (taken from a previous report) that use drainage area and percentage of impervious cover as independent variables also are given. From a comparison of the rural areal equations and the urban equations it was found that in the hydrologic area with the high flood runoff (described above), the rural equations sometimes estimated higher flood magnitudes than the urban equations. Therefore, in this hydrologic area the rural equations are recommended for both rural and urban streams.

Rivers with drainage areas greater than 1,500 square miles cannot be regionalized. Estimating methods for these rivers are presented graphically.

Relation of maximum floods of record to drainage areas in Alabama are given for a guide in estimating potential maximum floods. This relation estimates maximum floods about two times larger than relations used previously.

INTRODUCTION

Estimates of magnitude and frequency of floods are needed for planning and economic design of highways, railroads, and other structures near streams. Flood information is also needed for flood-plain management and development, and for determining flood insurance rates. The purpose of this report is to provide methods of estimating the magnitude and frequency of floods for streams in Alabama with drainage areas of 1 to 22,000 mi².

Peak-discharge records of 10 or more years through September 1981 from gaging stations in Alabama and surrounding states were available for use in this report. Statistical analyses of these records were performed using procedures and standards described in Water Resources Council Bulletin 17B (1981) to estimate flood magnitudes for selected frequencies. These flood magnitudes were combined

with flood data derived from the small streams project (Olin and Bingham, 1977), and regression analyses were performed to derive equations for estimating flood magnitudes in un-gaged natural flow rural streams. Procedures are also given to determine flood magnitudes for streams with large drainage areas that could not be regionalized. The method and equations for estimating flood magnitudes for selected frequencies in urban streams is also presented (taken from previous report by Olin and Bingham, 1982). Maximum observed floods versus drainage area also are presented.

This report was prepared by the U.S. Geological Survey in cooperation with the Alabama Highway Department and is based on flood data collected through 1981 as a part of cooperative programs with the Alabama Highway Department, the Geological Survey of Alabama, and other State and Federal agencies. Flood data for some streams in the Mobile River basin were furnished by the U.S. Army Corps of Engineers, Mobile District.

Magnitude and frequency of floods in Alabama have been described by Peirce (1954), Speer and Gamble (1964), Gamble (1965), Barnes and Golden (1966), and Hains (1973). Magnitude and frequency of floods for small drainage area rural streams have been described by Olin and Bingham (1977), and for urban streams by Olin and Bingham (1982).

DESCRIPTION OF THE AREA

Topography

The topography in northern Alabama is rugged with land surface elevations generally ranging from 200 to 2,400 feet. The topography in the remainder of the State to the south is less rugged with elevations generally ranging from sea level to 1,000 feet. Land slopes in the south, with limited exceptions, are far more gentle than those in the north.

Climate

Alabama has a humid temperate climate. Subfreezing temperatures usually last less than 24 hours. Precipitation is predominantly in the form of rain. The mean annual rainfall varies from about 49 inches in the north to 68 inches in the south (Supplementary Data I). March and July are the wettest months, with most major floods occurring in March. Rains

producing floods in Alabama are associated with three types of storms—broad cyclonic disturbances, tropical hurricanes, and summer thunderstorms. Cyclonic disturbances occur every year, commonly between November and April, and bring steady rains over large areas. Tropical hurricanes generally occur between July and November and are less frequent than cyclonic disturbances. Hurricanes often bring torrential rains from the Gulf of Mexico. Summer thunderstorms are often intense but affect relatively small areas.

Factors Influencing Floods

Climatic and land factors influence floods. The climatic factors are the amount, intensity, and areal distribution of rainfall. The land factors are slope of land surface, elevation, composition, land use, land cover, and the general shape and orientation of the basin and its stream channels. Land factors influence the amount and time distribution of runoff. Where the land slopes steeply, overland runoff will reach the stream rapidly and peak flow will occur soon after the rainfall. Where the land slope is flatter, overland runoff will be slower and the peak will occur later after the rainfall. The peak flow and volume of floods are dependent on the amount and intensity of rainfall. The mean annual rainfall and the 2-year 24-hour rainfall (Supplementary Data I) in Alabama generally decrease northward; however, the increase in land surface slope (as indicated by the main channel slope) from south to north tends to offset some of the effect of the lesser amount and intensity of rainfall.

FLOOD DATA

The earliest peak-stage records in Alabama were collected in 1814. Systematic collection of flood records was begun by the U.S. Geological Survey at about the turn of the twentieth century. The data collection network was expanded in 1958 to include 37 rural stream stations with drainage areas less than 15 mi². In 1974, 23 stations on streams draining urban areas were added.

Peak flow data for 234 rural gaging stations and 23 urban stations were available for this report. The 23 urban stations were used to define regional urban equations (see Olin and Bingham, 1982) which are presented

in the section titled "Flood Magnitude and Frequency for Urban Streams." Peak flow data for 17 of the 234 rural gaging stations could not be used for flood frequency computations because the streams are regulated by man. Location and basin characteristics are given in Supplementary Data I; station locations are shown on plate 1.

Because of the volume of flood data available for numerous stations, the data are not presented here. An example of what is available, including location descriptions and annual flood peaks and stages, is given in Supplementary Data II. This information can be acquired from the District Chief, U.S. Geological Survey, 520 19th Avenue, Tuscaloosa, Alabama 35401.

FLOOD FREQUENCY ANALYSIS

The relation of annual flood-peak flows to their probability of occurrence, or recurrence interval, is referred to as a flood-frequency relation. Recurrence interval is the average interval of time between exceedances of the indicated flood magnitude and is equal to the reciprocal of the annual probability of exceedance. For example, a flood having a 10-year recurrence interval has a 1 in 10 chance, on the average, of occurring in any given year. However, the fact that a flood of a certain magnitude occurs in one year does not reduce the probability of a flood as great or greater occurring within the same year or during the next year.

Flood-frequency analyses were computed for 217 rural stations with 10 or more years of annual peak records by fitting the logarithms of observed annual peaks to a log-Pearson Type III distribution as described by the U.S. Water Resources Council (1981). Results of the analyses are given in Supplementary Data I. The log-Pearson Type III distribution is defined by three statistical parameters.

The equation and parameters are:

$$\log Q_p = \bar{x} + K_p S \quad \text{where}$$

\bar{x} = the mean of the logarithms of the annual peaks,

K_p = a function of the exceedance probability and the skew coefficient of the logarithms of the annual peaks,

S = the standard deviation of the logarithms of the annual peaks, and

Q_p = the annual peak discharge having probability p of being exceeded in any year.

The reliability of flood-frequency relations estimated from flood peaks is primarily dependent upon the length of observed record. To improve the reliability of the flood frequency estimates, the station skew coefficient was adjusted to a generalized skew coefficient, and the length of observed record was extended based on historic flood information (U.S. Water Resources Council, 1981). For stations where the flow is significantly altered by man, the station skew was used without adjustment. Outliers in the data array are annual flood peaks that are either higher or lower than the trend of other peaks. High outliers for each station were compared with historic flood data for nearby stations. If comparison indicated that a particular outlier was a historic peak, the length of record for that station was adjusted. For stations without historic data, high outliers were treated as a part of the systematic record. Low outliers identified were deleted.

FLOOD MAGNITUDE AND FREQUENCY FOR RURAL STREAMS

Approach and Variables

Estimates of the 2-, 5-, 10-, 25-, 50-, and 100-year floods were related to various basin and climatic characteristics by regression analyses. The form of the equation used in the regression model was:

$$Q_n = aA^bB^cC^d$$

where

Q_n = flood magnitude for n -year recurrence interval (dependent characteristic),

A, B, C = basin and climatic characteristics of the drainage basin (independent characteristics), and

a, b, c, d = constants for given recurrence intervals.

Basin and climatic characteristics tested by regression techniques are:

- *Drainage area*, (A), in square miles, is the contributing area determined by outlining the basin on the latest topographic map available and planimetering the area within the outline.
- *Main channel slope*, (S), in feet per mile, is the average slope between points 10 and 85 percent of the total channel length from the gaging station to the basin divide.
- *Main channel length*, (L), in miles, is the distance along the longest channel from the gaging station to the basin divide.
- *Mean basin elevation*, (E), in feet, measured from topographic maps by transparent grid-sampling method (20 to 60 points sampled in each basin).
- *Storage*, (ST), in percent, is that part of the basin occupied by swamps, ponds, or reservoirs.
- *Forest cover*, (F), in percent, the area of forest expressed as a percentage of the contributing drainage area; forested area shown on topographic maps.
- *Soils index*, (Si), in inches, is soil infiltration capacity estimated by the Soil Conservation Service for previous Alabama flood report (Hains, 1973).
- *Mean annual precipitation*, (P), in inches, from U.S. Weather Service, Climates of the States, v. 1-Eastern States, 1957.
- *24-hour 2-year interval rainfall*, (I_{24-2}), in inches, from U.S. Weather Bureau Technical Paper 40, 1961.
- *Latitude and longitude of the gage*, (LAT and LONG), in degrees, measured on a topographic map by engineers scale or calibrated grid.

Regression Analyses

The first regression analysis used flood magnitudes from 217 stations as dependent variables and all basin and climatic characteris-

tics as independent variables. This analysis indicated that station data from large streams could not be regionalized. Twenty stations having drainage areas greater than 1,500 mi² were deleted. These streams have channels and large flood plains unique to each stream with widely varying storage capacity and slopes; thus they can not be regionalized.

The second regression analysis included basin and climatic characteristics as independent variables and flood magnitudes as dependent variables. This analysis indicated that drainage area, main channel slope, and latitude were the only independent variables statistically significant at the 5 percent level. The standard error of estimate ranged from 35 percent for the 10-year flood to 44 percent for the 100-year flood. Drainage area and latitude were the most significant variables for estimating the 2-, 5-, and 10-year floods. Drainage area and main channel slope were the most significant variables for estimating the 25-, 50-, and 100-year floods.

Residuals (the difference in log units of flood magnitudes estimated with equations and observed annual peaks) from this regression were plotted on a map. This plot showed that significant geographic variations of the residuals occurred and, based on grouping similar residuals, the State could be divided into seven hydrologic areas for estimating flood magnitudes (fig. 1 and plate 1). The boundaries were refined using outcrop of geologic units with similar runoff properties.

Regression analyses were made using stations in the hydrologic areas to determine if further divisions were necessary. These regressions indicated that drainage area was statistically significant at the 5 percent level in all areas and that storage was statistically significant at the same level in area 2. The average standard error of estimate for these equations is 10 percent less than a single equation for the entire state.

The plot of residuals from the equations for each hydrologic area showed no geographic bias or variation, indicating that effects of main channel slope and latitude are similar in each area. The distribution of drainage areas along with the harmonic mean length of record, in years for stations in hydrologic areas 1-6 used in the regression analyses, is in table 1.

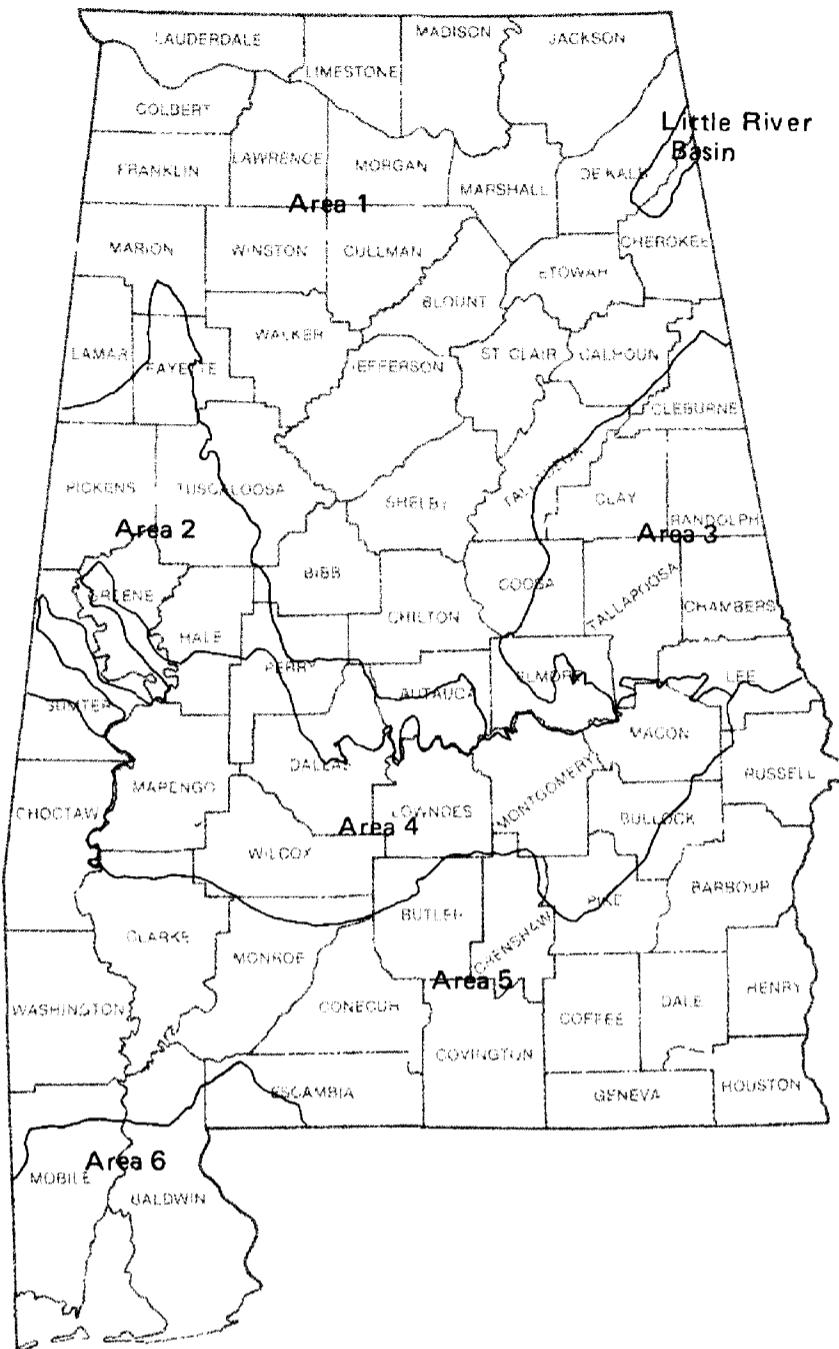


Figure 1. Hydrologic areas.

The ranges of: slope, mean annual precipitation, and 24-hour 2-year rainfall; mean station skew, mean standard deviation of logarithms of the annual peaks, and the interstation correlation coefficient for stations in hydrologic areas 1-6 used in the regression analyses are in table 2. The areal equations are in table 3.

Hydrologic Areas

The Little River basin is an area not defined by areal equations (fig. 1 and plate 1). Flood magnitudes for Little River near Jamestown, Ala. (LR-01) and Little River near Blue Pond, Ala. (LR-02) did not fit in either area 1 or 3. The Little River basin is on the top of Lookout Mountain in northeast Alabama and northwest Georgia. The river has cut a deep canyon along most of the basin and has several waterfalls. Flood runoff is very high because

of impervious rock and very little soil cover. A statistical analysis was not made for this river basin. A relation of flood magnitudes to drainage area is presented in the section on major rivers for use in estimating flood magnitudes.

Hydrologic area 1 (fig. 1 and plate 1) has very rugged topography, and contains most of the stations (87) used in the regression analyses. Rainfall both in total amount and intensity (table 2) average less than other hydrologic areas in the State.

In hydrologic area 2, the plot of residuals showed the areal equations over-estimated the observed flood magnitudes. To improve the estimates for gaging stations a storage parameter was used in the equation. This area is a narrow band extending from the Mississippi

state line southeastward to Montgomery County (fig. 1 and plate 1). Flood plain slopes along streams in the narrow band are gentle, and numerous large swamps are present which have the capacity to store large amounts of water. The equations for area 2 given in table 3 can be used to account for storage effects.

Hydrologic area 3 in eastern Alabama has less flood runoff than other areas of the State. The low flood runoff is due to a high infiltration rate that transmits water to the ground-water reservoir.

Hydrologic area 4 in central Alabama has higher flood runoff than other areas of the State. This area has impervious chalk and marl outcrops which cause more surface runoff, and the topography is not rugged.

Hydrologic area 5 in southern Alabama is about the most northern area of the State where major floods are caused by hurricanes. The topography is not rugged, but a line of hills crossing the State west to east gives the area moderate relief.

Hydrologic area 6 is located in the southernmost part of the State and receives more rainfall both in amount and 2-year 24-hour intensity than the other areas of the State. The topography is less rugged than the northern areas of Alabama.

Accuracy

Accuracy of regression equations can be expressed by two different methods; standard error of estimate of the regression (in percent) or equivalent years of record (in years). Standard error of estimate of the regression is the standard deviation of the differences between station data and the corresponding values computed from the regression equation. The standard error of estimate of the regression contains both model (space-sampling) and time-sampling error components. The accuracy of this regression depends in part on how much of the standard error of estimate is due to time-sampling error (statistical estimation error in the flow characteristics due to short record lengths at the stations used in the regression) and how much is due to error in the underlying relation between the true values of the flow characteristics and the set of basin characteristics (Hardison, 1971). When different station records are combined by means of regression

equations the time-sampling error for the equations is less than that of any one station. The variance of the time-sampling error for each regression equation corrected by the interstation correlation coefficient were computed. The ratio of the standard error of estimate of the regression to the time-sampling error (square root of the variance) was computed and are given in table 4. The standard error of estimate of the regression and the corrected time-sampling error are both used to determine the equivalent years of record (table 4). Equivalent years of record is the number of years of annual peak flow data needed to obtain the same accuracy as the equations and was computed by the following equation (Hardison, 1971):

$$Nu = R^2 \left[\frac{Iv}{SER} \right]^2$$

where:

Nu is the equivalent years of record,
 R is a factor standard error of a T-year event
 to Iv and,
 Iv is the average standard deviation of
 logarithms of annual events, and
 SER standard error of regression in log
 units.

Limitations

The following limitations should be observed when using the regression equations and graphs:

- (1) They should not be used where dams, flood-detention structures, and other man-made works have a significant effect on peak discharges. Under such conditions, stream systems studies involving reservoir and open-channel routing may be required to evaluate flood frequency, which is beyond the scope of this report.
- (2) They should not be used in urban areas unless the effects of urbanization are insignificant.
- (3) The user is cautioned that the magnitude of error may be large if equations are used to estimate flood magnitudes for streams with drainage area sizes outside the ranges listed in table 1.
- (4) Storage effects may exist in other areas besides area 2. Caution should be used

when estimating flood magnitudes along streams where storage may affect flood runoff.

Mathematical Solution for Rural Streams

The following computations demonstrate mathematical application of the regression equations (table 3) to a site on a stream in area 2. The selected site has a drainage area of 54.5 square miles, of which 3.4 square miles (6.2 percent of basin) is swamp. Using equations for area 2, the flood magnitudes computed are:

$$\begin{aligned} Q_2 &= 149(A)^{0.689} (ST + 1.0)^{-.219} \\ &= 149 (54.5)^{0.689} (7.2)^{-.219} \\ &= 1,520 \text{ ft}^3/\text{s} \end{aligned}$$

$$\begin{aligned} Q_5 &= 310(A)^{0.642} (ST + 1.0)^{-.172} \\ &= 310 (54.5)^{0.642} (7.2)^{-.172} \\ &= 2,880 \text{ ft}^3/\text{s} \end{aligned}$$

$$\begin{aligned} Q_{10} &= 459(A)^{0.616} (ST + 1.0)^{-.142} \\ &= 459 (54.5)^{0.616} (7.2)^{-.142} \\ &= 4,070 \text{ ft}^3/\text{s} \end{aligned}$$

$$\begin{aligned} Q_{25} &= 696(A)^{0.590} (ST + 1.0)^{-.109} \\ &= 696 (54.5)^{0.590} (7.2)^{-.109} \\ &= 5,940 \text{ ft}^3/\text{s} \end{aligned}$$

$$\begin{aligned} Q_{50} &= 904(A)^{0.574} (ST + 1.0)^{-0.084} \\ &= 904 (54.5)^{0.574} (7.2)^{-0.084} \\ &= 7,600 \text{ ft}^3/\text{s} \end{aligned}$$

$$\begin{aligned} Q_{100} &= 1144(A)^{0.560} (ST + 1.0)^{-0.060} \\ &= 1144 (54.5)^{0.560} (7.2)^{-0.060} \\ &= 9,540 \text{ ft}^3/\text{s} \end{aligned}$$

A plot of these example data is shown in figure 2.

Improved Estimate of Flood Magnitude

Flood magnitudes at gaging stations on streams draining less than 1,500 mi² should be weighted using the station flood magnitudes in Supplementary Data I and estimates obtained from the areal equations. The following equation taken from U.S. Water Resources Council (1981) should be used:

$$\log Q_{g(w)} = \frac{N \log Q_g + EY \log Q_n}{N + EY} \quad (1)$$

where:

$Q_{g(w)}$ = the weighted discharge for selected recurrence interval,
 Q_g = the station discharge for selected recurrence interval,
 Q_n = the regression discharge for selected recurrence interval,
 N = the number of years of station data used to compute Q_g , and
 EY = the equivalent years of record for Q_n from table 4.

The following procedure can be used to transfer flood magnitudes if the drainage area of the site is within 50 percent (either greater or less than) of that at a gaging station on the same stream:

$$Q_u = \left(\frac{A_u}{A_g} \right)^b Q_{g(w)} \quad (2)$$

and a weighted value can be computed by the equation

$$Q_{u(w)} = \left[\frac{2A}{A_g} \right] Q_N + \left[1 - \frac{2A}{A_g} \right] Q_u \quad (3)$$

where:

Q_u = the flood magnitude at the ungaged site transferred from the gaging station by the drainage area ratio,
 $Q_{g(w)}$ = the weighted flood magnitude from the gaging station if drainage area is less than 1,500 mi², station flood magnitude if drainage area greater than 1,500 mi²,

Q_N = the flood magnitude at the ungaged site computed by the areal equations of that area,

$Q_{u(w)}$ = the weighted flood magnitude at the ungaged site,

b = the drainage area exponent of the areal equation for the applicable hydrologic area and the selected recurrence interval,

A_u = drainage area at the ungaged site,

A_g = drainage area of the gaging station, and

A = difference in drainage areas of the gaging station and ungaged site.

Where the drainage area of the ungaged site differs by more than 50 percent of the drainage area of the gaging station, estimates

of flood magnitude for an ungaged site should be made from the areal equations or graphs.

Graphical Solution for Rural Streams

Regression equations using drainage area of the basin are used to estimate flood magnitudes. Solutions for the equations to estimate the 2-, 5-, 10-, 25-, 50-, and 100-year floods for rural streams are in graphical form in figures 3 to 12, respectively.

An example is given to illustrate use of the curves in figure 3 (areas 1 and 6). The dashed line and arrows in figure 3 indicate the procedure to follow.

$$A = 20 \text{ mi}^2$$

Enter figure 3 with drainage area (20 mi^2) along the bottom scale. Move upward to recurrence interval curve of 2-year. Move horizontally to the discharge scale and read the value of $1,510 \text{ ft}^3/\text{s}$. The following results were obtained for the given recurrence intervals:

Q_2	=	$1,510 \text{ ft}^3/\text{s}$
Q_5	=	$2,450 \text{ ft}^3/\text{s}$
Q_{10}	=	$3,160 \text{ ft}^3/\text{s}$
Q_{25}	=	$4,140 \text{ ft}^3/\text{s}$

Drainage area (A) = 54.5 sq. mi.
Swamp area ≈ 3.4 sq. mi.
or Storage (ST) = 6.2%

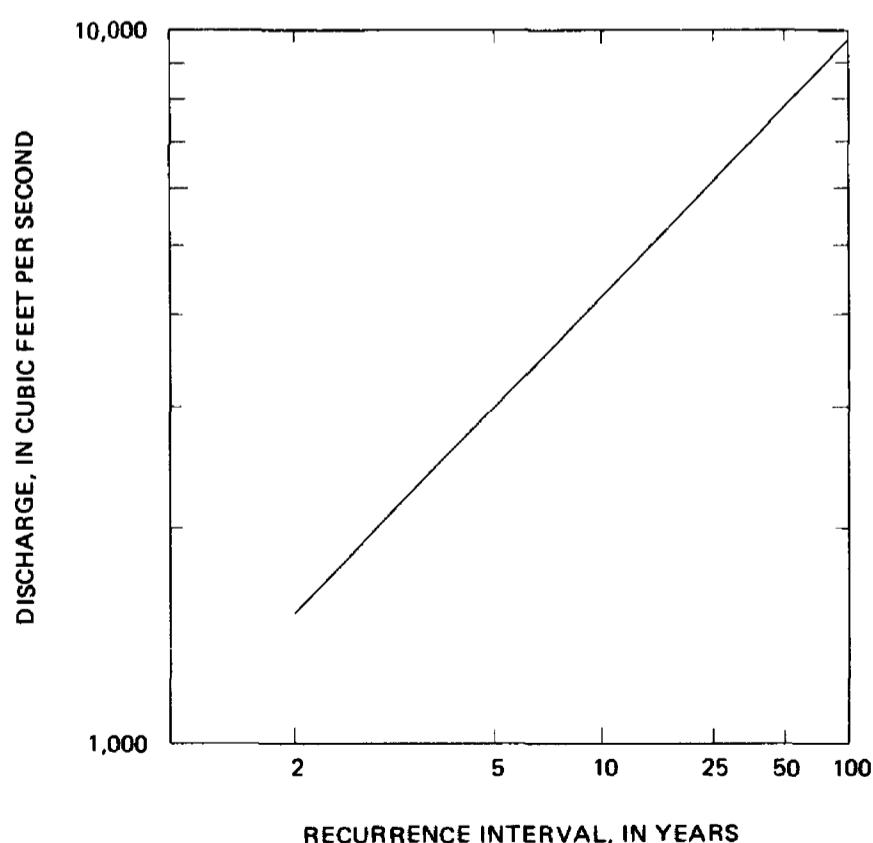


Figure 2. Flood-frequency curve for example rule stream in hydrologic area 2.

Q_{50}	=	$4,940 \text{ ft}^3/\text{s}$
Q_{100}	=	$5,770 \text{ ft}^3/\text{s}$

FLOOD MAGNITUDE AND FREQUENCY FOR URBAN STREAMS

The following equations can be used to estimate flood magnitudes for streams draining urban areas with more than 5 percent impervious cover (Olin and Bingham, 1982).

	SE*
$Q(u)_2 = 150 A^{0.70} IA^{0.36}$	26 (4)
$Q(u)_5 = 210 A^{0.70} IA^{0.39}$	24 (5)
$Q(u)_{10} = 266 A^{0.69} IA^{0.39}$	24 (6)
$Q(u)_{25} = 337 A^{0.69} IA^{0.39}$	24 (7)
$Q(u)_{50} = 396 A^{0.69} IA^{0.38}$	25 (8)
$Q(u)_{100} = 444 A^{0.69} IA^{0.39}$	25 (9)

*Standard error of estimate in percent.

where:

$Q(u)$ = the estimated urban discharge, in cubic feet per second for the indicated recurrence interval,

A = the contributing drainage area, in square miles, and

IA = impervious area, in percent.

All regression coefficients are statistically significant at the 5 percent level.

The urban equations were derived by multiple regression analyses of flood magnitudes obtained from synthetic discharge data generated with a calibrated rainfall-runoff model and basin characteristics for 23 stations in Alabama. The regression analyses indicated that drainage area size and percent of the basin occupied by impervious materials are the most significant basin characteristics affecting flood magnitudes in urban areas.

Drainage area can be measured from the best available topographic maps. The impervious cover in the basin can be measured by the grid method on topographic maps or recent aerial photographs. Some basins have non-contributing areas due to pumping surface runoff from storm drains out of the basin.

This non-contributing area should be subtracted from the total drainage area. Additional information on the urban equations is given in Olin and Bingham (1982).

Limitations of Equations

The urban equations should be used only for streams with drainage areas ranging from 0.16 to 83.5 mi² and impervious areas ranging from 5.0 to 42.9 percent. The equations do not apply to urban streams where temporary in-channel or overbank storage significantly affects the magnitude of peak flow. The user is cautioned not use the urban equations for streams located in hydrologic area 4. For urban streams in area 4 use rural areal equations.

A comparison of all areal rural equations for area 4 and the urban equations was made using the same size drainage area and with different percentages of impervious cover in the urban equations. In some of these comparisons, the areal rural equations for hydrologic area 4 estimated higher flood magni-

tudes than the urban equations. Area 4 has impervious chalk and marl which produces the higher flood runoff. No stations in area 4 were used to develop the urban equations; therefore, it is recommended that the areal rural equations be used.

Mathematical Solution of Equations for Urban Streams

The following computations demonstrate mathematical application of the regression equations for urban streams in Alabama; for example, an urban stream with a drainage area of 10.5 mi² and with 1.47 mi² (14.0 percent) impervious area. By substituting values for drainage area and percent impervious area into the equations and performing the mathematical operation, flood magnitude is estimated as follows:

$$\begin{aligned} Q(u)_2 &= 150 A^{0.70} IA^{0.36} \\ &= 150 (10.5)^{0.70} (14.0)^{0.36} \\ &= 2,010 \text{ ft}^3/\text{s} \end{aligned}$$

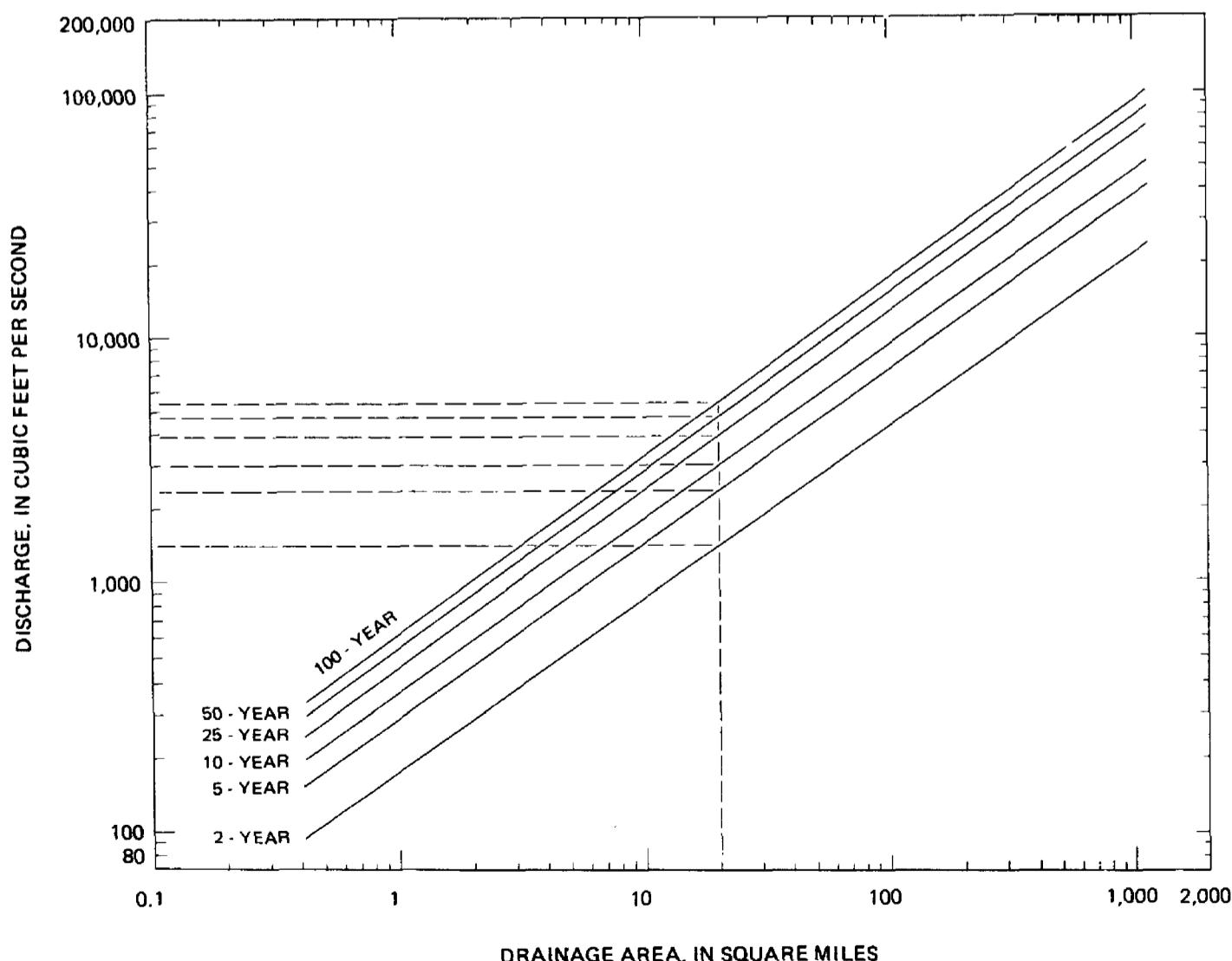


Figure 3. Relation of flood magnitudes to drainage area for selected recurrence intervals in hydrologic areas 1 and 6.

$$\begin{aligned}Q(u)_5 &= 210 A^{0.70} IA^{0.39} \\&= 210 (10.5)^{0.70} (14.0)^{0.39} \\&= 3,050 \text{ ft}^3/\text{s}\end{aligned}$$

$$\begin{aligned}Q(u)_{10} &= 266 A^{0.69} IA^{0.39} \\&= 266 (10.5)^{0.69} (14.0)^{0.39} \\&= 3,770 \text{ ft}^3/\text{s}\end{aligned}$$

$$\begin{aligned}Q(u)_{25} &= 337 A^{0.69} IA^{0.39} \\&= 337 (10.5)^{0.69} (14.0)^{0.39} \\&= 4,780 \text{ ft}^3/\text{s}\end{aligned}$$

$$\begin{aligned}Q(u)_{50} &= 396 A^{0.69} IA^{0.38} \\&= 396 (10.5)^{0.69} (14.0)^{0.38} \\&= 5,470 \text{ ft}^3/\text{s}\end{aligned}$$

$$\begin{aligned}Q(u)_{100} &= 444 A^{0.69} IA^{0.39} \\&= 444 (10.5)^{0.69} (14.0)^{0.39} \\&= 6,290 \text{ ft}^3/\text{s}\end{aligned}$$

A flood frequency curve constructed from these example computations is shown in figure 13.

FLOOD MAGNITUDE OF RIVERS DRAINING AREAS LARGER THAN 1,500 SQUARE MILES

Relations of flood magnitude to drainage areas are used for estimating flood frequencies for rivers draining more than 1,500 mi². The flood magnitude and frequencies computed from flood data at gaging stations on the large rivers were, with the exception of the Coosa River, not adjusted for regulation by reservoirs. The flood magnitude and frequencies for the Coosa River, obtained from the Corps of Engineers, Mobile District, were adjusted for regulation.

Flood magnitudes used in these relations were computed from either the unweighted or weighted flood magnitudes at gaging stations (weighted if the drainage area is less than 1,500 mi²) transferred by equations 1, 2, and 3. Flood magnitudes for selected frequencies can be estimated using figures 14 through 23 by reading the discharge scale for known drainage areas.

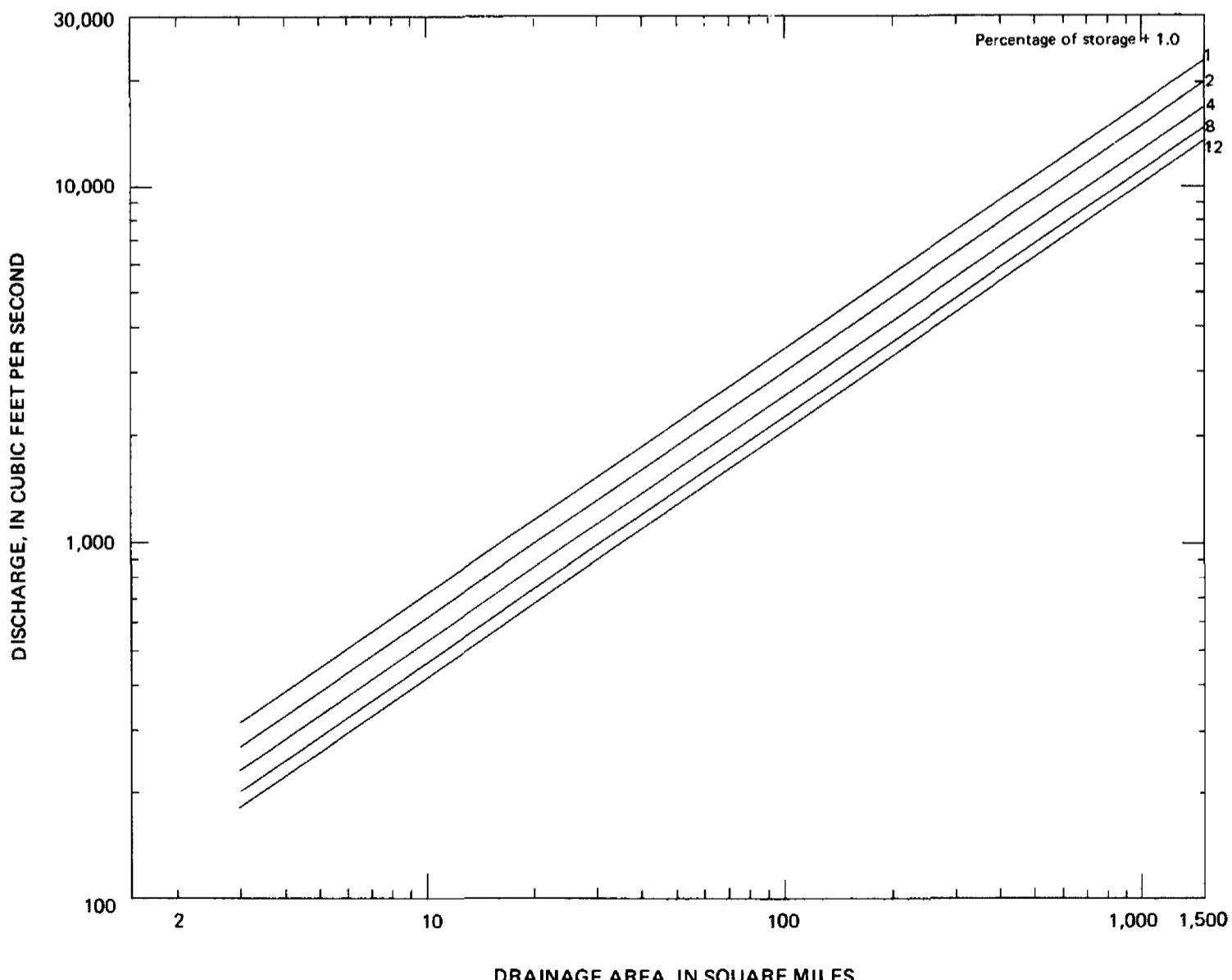


Figure 4. Relation of 2-year flood magnitudes to drainage area for selected storage factors in hydrologic area 2.

Chattahoochee River Basin

Chattahoochee River

Flood-frequency relations are not given for the Chattahoochee River due to regulation. If information is desired for flood-frequency determinations, contact the Corps of Engineers, Mobile District.

Choctawhatchee River Basin

Pea River

The relation of flood magnitudes to drainage area for selected recurrence intervals for the Pea River (fig. 14) used weighted flood magnitudes (eq. 3) transferred from the gage "near Samson, Ala." (5-17).

Choctawhatchee River

The relation of flood magnitudes to drainage area for the Choctawhatchee River is given in figure 15. The flood magnitudes at the station "near Newton, Ala." (5-12) are the same as those in Supplementary Data I.

Weighted flood magnitudes were transferred from "near Newton, Ala." (683 mi^2) by equation 3 to a drainage area of $1,020 \text{ mi}^2$ (50 percent increase). The weighted flood magnitudes at Choctawhatchee River at Carryville, Fla. (BDA-01) taken from "Techniques for Estimating Magnitude and Frequency of Floods on Natural-Flow Streams in Florida" (Bridges, 1982) were transferred to drainage areas of $3,158 \text{ mi}^2$ (Alabama-Florida state line) and $1,750 \text{ mi}^2$. The relation is to be only used from $1,500 \text{ mi}^2$ to the state line ($3,158 \text{ mi}^2$). The flood magnitudes for drainage areas less than $1,500 \text{ mi}^2$ are shown only for reference.

Escambia River Basin

Conecuh River

The relation of flood magnitudes to drainage area for the Conecuh River is given in figure 16 for selected recurrence intervals. Weighted flood magnitudes for $1,500 \text{ mi}^2$ were transferred from the station "near Andalusia, Ala." (5-28) by equation 3. Flood magnitudes at the

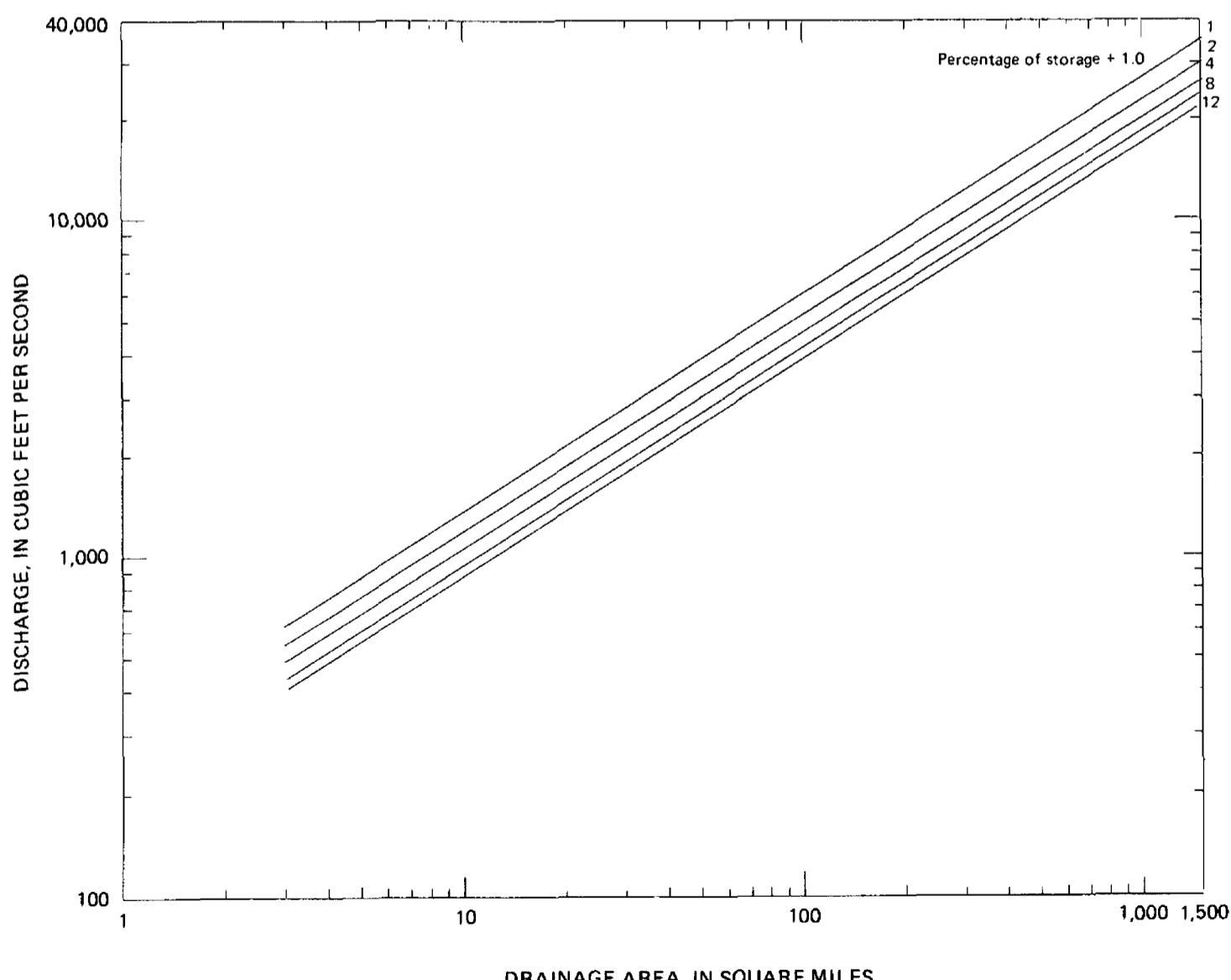


Figure 5. Relation of 5-year flood magnitudes to drainage area for selected storage factors in hydrologic area 2.

station "near Brooklyn, Ala." (BDA-02) were taken from a log-Pearson Type III analysis of the flood peak data. The weighted flood magnitudes for 3,330 mi² (Alabama-Florida state line) were estimated from the station Escambia River (Conecuh River in Alabama) near Century, Fla. (BDA-03) using data and procedures from "Technique for Estimating Magnitude and Frequency of Floods on Natural-Flow Streams in Florida" (Bridges, 1982).

Mobile River Basin

Little River

The relation of flood magnitudes to drainage area is given in figure 17 for selected recurrence intervals for the Little River. Flood magnitudes for selected recurrence intervals for gaging stations "near Jamestown, Ala." (LR-01) and "near Blue Pond, Ala." (LR-02) are given in Supplementary Data I. The flood magnitudes shown at 105 mi² were extrapolated using the flood magnitudes between the

above stations. Equations 1, 2, and 3 cannot be used because areal equations do not apply to the Little River basin.

Coosa River

The relation of flood magnitudes to drainage area for the Coosa River has been estimated by the Corps of Engineers, Mobile District, and is given in figure 18.

Tallapoosa River

The relation of flood magnitudes to drainage area is given in figure 19 for selected recurrence intervals for the Tallapoosa River. The flood magnitudes at 1,500 mi² were transferred from the station "at Wadley, Ala." (BDA-08) by equation 2. The flood magnitudes at the stations "at Wadley, Ala." (BDA-08) and "below Tallassee" (BDA-10) were taken from a log-Pearson Type III analysis using annual flood peak data. The flood magnitudes are the same at "Martin

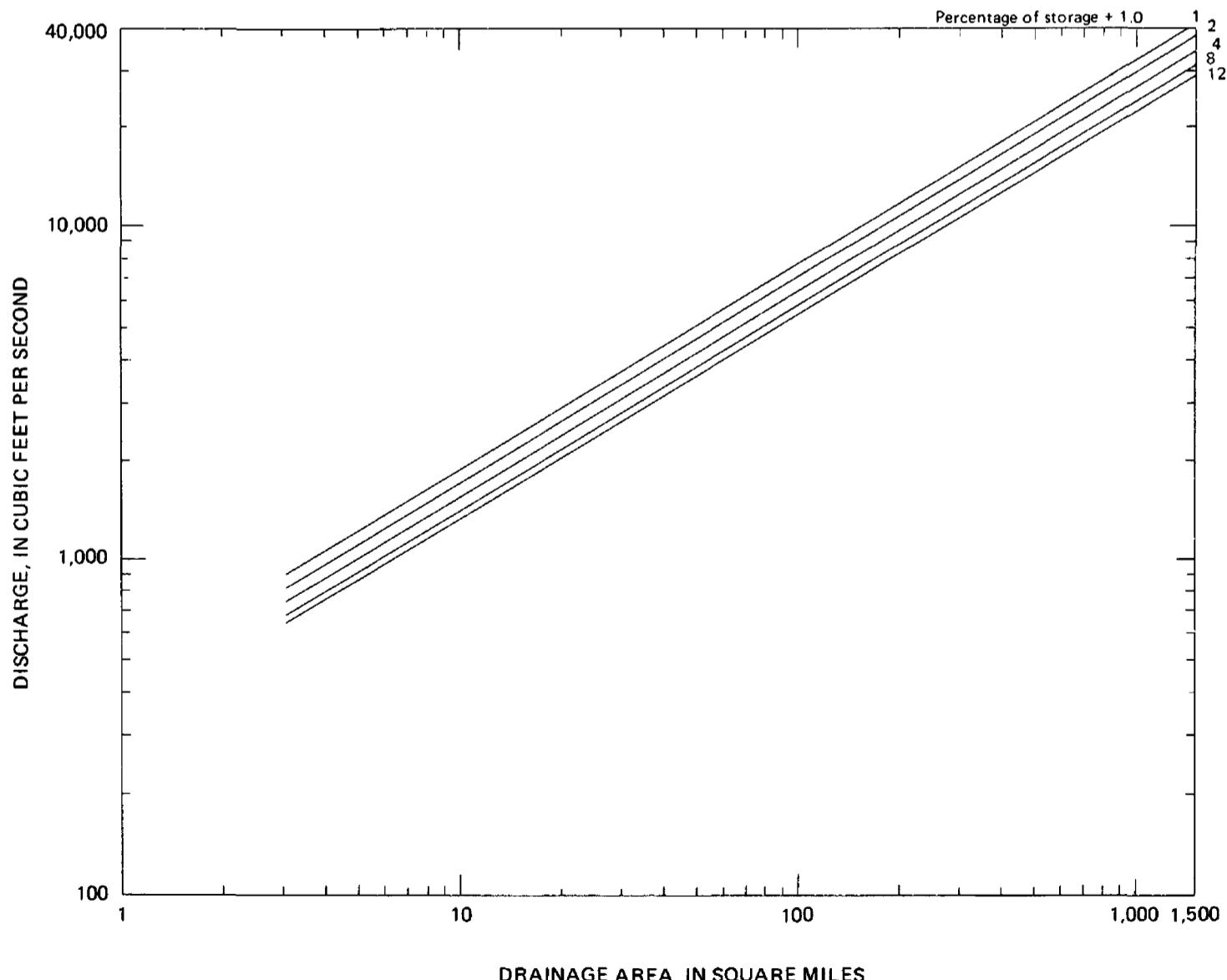


Figure 6. Relation of 10-year flood magnitudes to drainage area for selected storage factors in hydrologic area 2.

Dam" and at "below Tallassee" (BDA-10) because the peak flow data at the station is computed from releases for hydroelectric generation at "Thurlow Dam".

Alabama River

The relation of flood magnitudes to drainage area is given in figure 20 for selected recurrence intervals for the Alabama River. The flood flow of the Alabama River is regulated by dams on the Coosa and Tallapoosa Rivers. The flood magnitudes at the confluence of Coosa and Tallapoosa Rivers were extrapolated using records from "near Montgomery" (BDA-11) and "at Selma" (BDA-12). The peak discharges at the station "near Montgomery" (BDA-11) are larger than at other stations downstream because overbank storage increases. Downstream of Montgomery, the river overflows its banks onto a wide flood plain; this overflow creates storage which reduces the peak flow. The flood magnitudes at the mouth (Tombigbee River cut-off) were estimated from station magnitudes for the station "at Claiborne" (BDA-14) by equation 2.

Cahaba River

The relations of flood magnitudes to drainage area of the Cahaba River are shown in figure 21. The flood magnitudes at 1,500 mi² were estimated from Cahaba River at Sprott (BDA-15) by equation 3. The flood magnitudes at the confluence of the Cahaba and Alabama Rivers were estimated by the ratio of magnitudes between 1,500 mi² and the gaging station record "near Marion Junction" (BDA-16). The relation (fig. 21) should be used from the gaging station "BDA-15 at Sprott, Ala." to the "mouth at Alabama River." The drainage area (1,378 mi²) of the gaging station "BDA-15 at Sprott, Ala." is less than 1,500 mi² but the areal equations cannot be used here because of the over-bank storage. Flood magnitudes for the gaging station "near Marion Junction" (BDA-16) are smaller than those at the station "at Sprott" (BDA-15). This is the effect of over-bank storage in the Cahaba River.

Black Warrior River

The relations of flood magnitudes to drainage area for the three gaging stations on

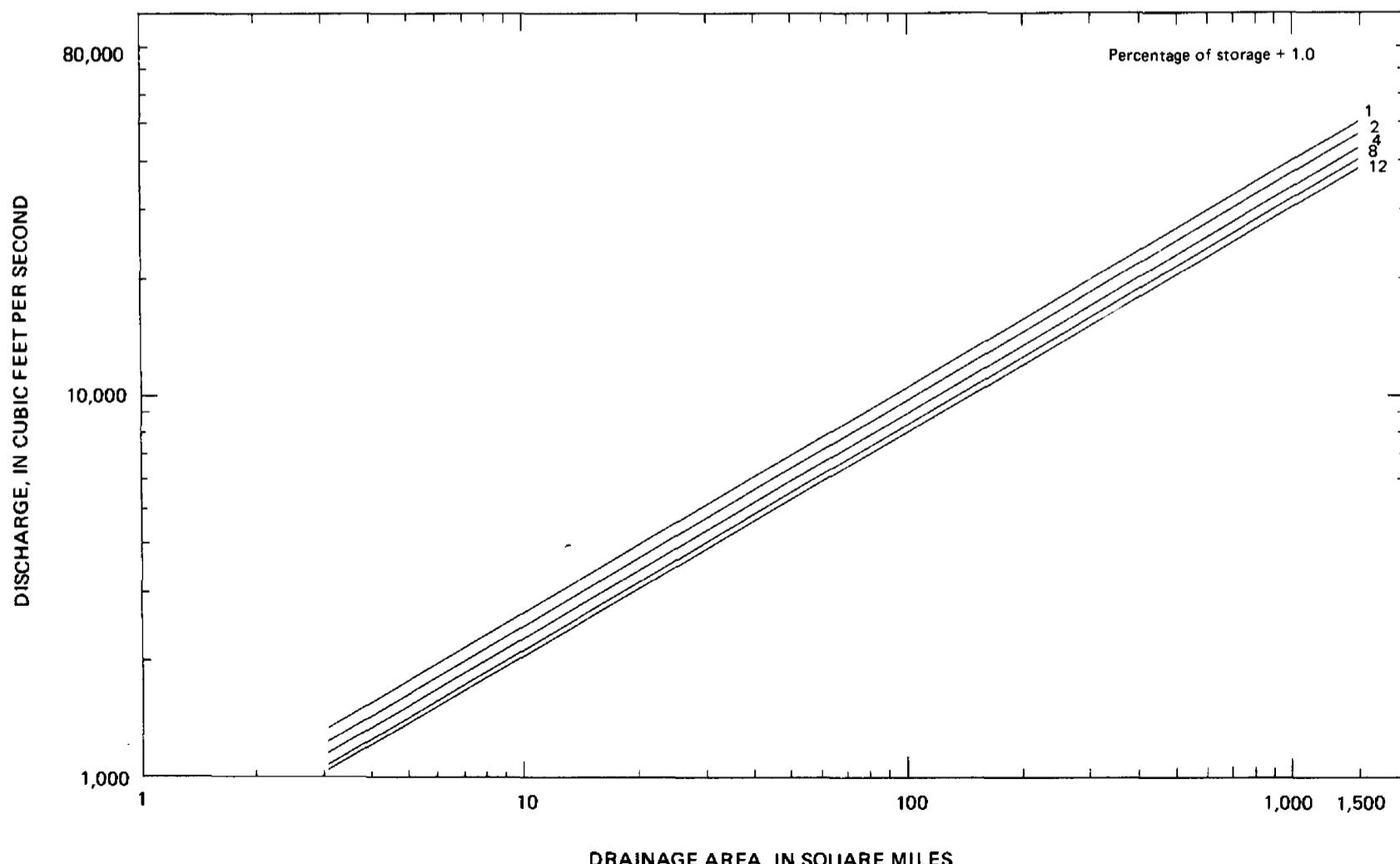


Figure 7. Relation of 25-year flood magnitudes to drainage area for selected storage factors in hydrologic area 2.

the Black Warrior River, "at Bankhead Dam" (BDA-17), "near Northport" (BDA-18), and "at Eutaw" (BDA-19), are given in figure 22. The flood magnitudes at the confluence of Locust and Mulberry Forks were estimated from the station "at Bankhead Dam" (BDA-17) by equation 2. Flood magnitudes at both "the confluence of Locust and Mulberry Forks" and "at Bankhead Dam" (BDA-17) are affected by storage of Lewis Smith Reservoir ($DA = 944 \text{ mi}^2$). The relation was not continued upstream for Mulberry Fork (drainage area of $2,366 \text{ mi}^2$ at confluence with Locust Fork) because flood magnitudes are unknown. The flood magnitudes at the confluence of the Black Warrior River and Tombigbee River were extrapolated using flood magnitudes between the stations "near Northport" (BDA-18) and "at Eutaw" (BDA-19).

Tombigbee River

Flood-frequency relations are not given for the Tombigbee River due to regulation. If information is desired for flood-frequency determinations, contact the Corps of Engineers, Mobile District.

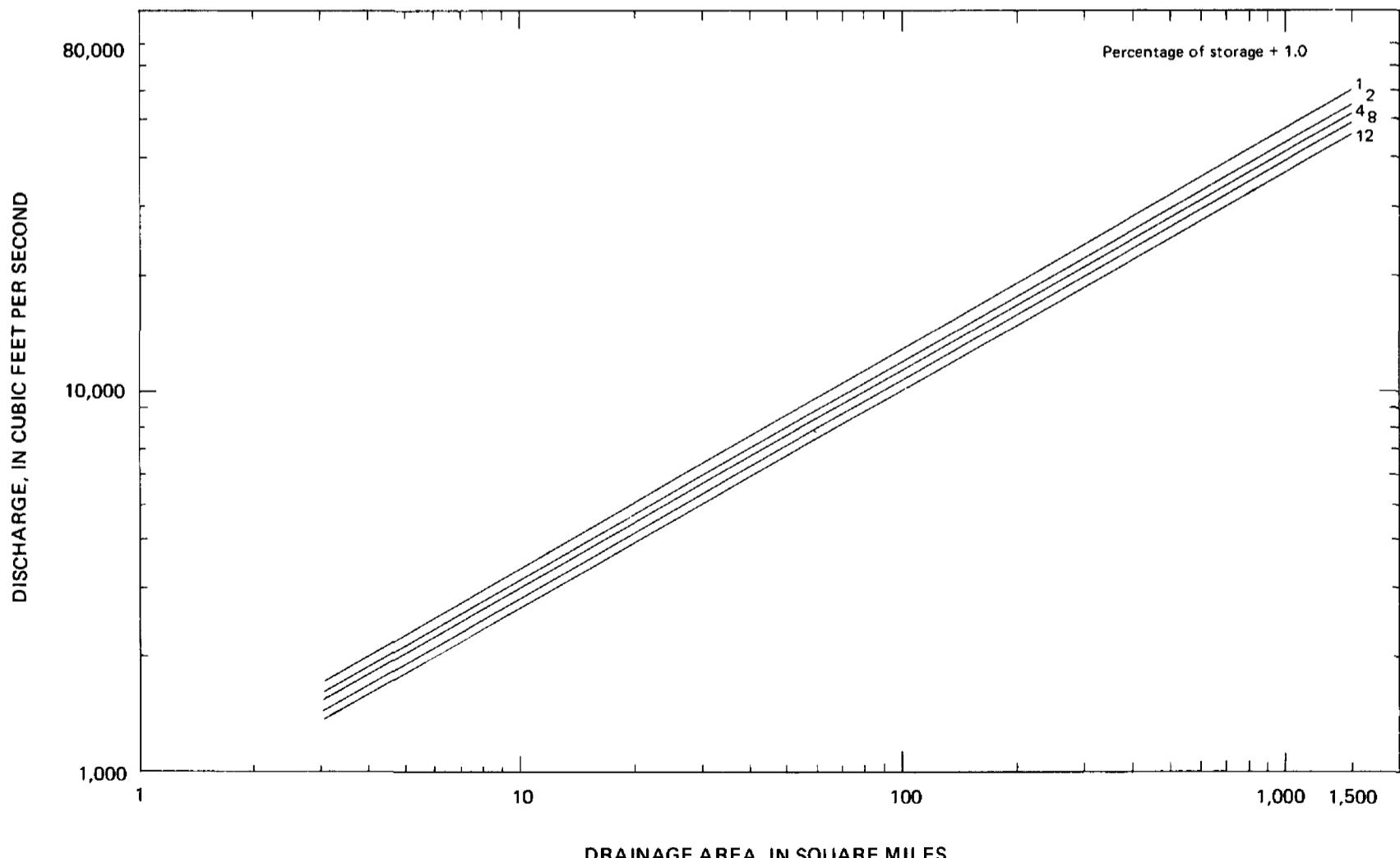


Figure 8. Relation of 50-year flood magnitudes to drainage area for selected storage factors in hydrologic area 2.

Mobile River

Flood magnitudes for selected recurrence intervals of the Mobile River at the Barry Steam Plant near Bucks, Ala., are given in table 5. The relation of flood magnitudes to drainage area for selected recurrence intervals cannot be defined.

Tennessee River Basin

Elk River

The relation of flood magnitudes to drainage area for Elk River is presented in figure 23. The flood magnitudes at "Ala.-Tenn. state line" and at drainage area $1,860 \text{ mi}^2$ were estimated using equation 2 and the flood magnitudes at "BDA-20 near Prospect, Tenn." The relation was not extended downstream to the mouth of the Elk River because of backwater caused by the Tennessee River during floods.

Tennessee River

Flood-frequency relations are not given for the Tennessee River due to regulation. If information is desired for flood-frequency

determinations, contact the Tennessee Valley Authority, Knoxville, Tennessee.

MAXIMUM FLOODS OF RECORD

The maximum observed discharge and (or) flood elevation for all rural gaging stations are listed in Supplementary Data III, for urban stations in Supplementary Data IV, and for short-term stations and miscellaneous sites in Supplementary Data V. The gaging stations listed include short-term stations (less than 10 years of record) and stations on regulated streams, and many are not shown on plate 1.

The maximum unit discharges in cubic feet per second per square mile were plotted versus drainage area in square miles for reference to estimate the maximum flood peak for any site in Alabama (fig. 24). One curve was drawn using Meyer's equation (Jarvis, 1926) to envelop the high side of all maximum unit discharges. The equation is

$$\frac{Q_{max}}{A} = C(A)^{-0.5} \quad (10)$$

where:

Q_{max} = magnitude of peak in cubic feet per

second,
 A = drainage area in square miles, and
 C = a coefficient depending upon characteristics of the basin.

The C value selected for Meyer's equation was 7,000. The other curve was drawn using Creager's equation (Creager, 1950) to envelop the high side of all maximum unit discharges. The equation is

$$\frac{Q_{max}}{A} = 46 C(A)^{[(0.894A)^{-0.08} - 1]} \quad (11)$$

The C value selected for Creager's equation was 54.

These C values reflect the maximum observed flood peaks that actually have occurred to date. The C values must increase each time a record-breaking flood (in terms of Q/A) occurs.

SUMMARY

Methods are presented to estimate flood magnitude for selected recurrence intervals for urban and rural streams with drainage areas from 1 to 22,000 square miles. Annual peak

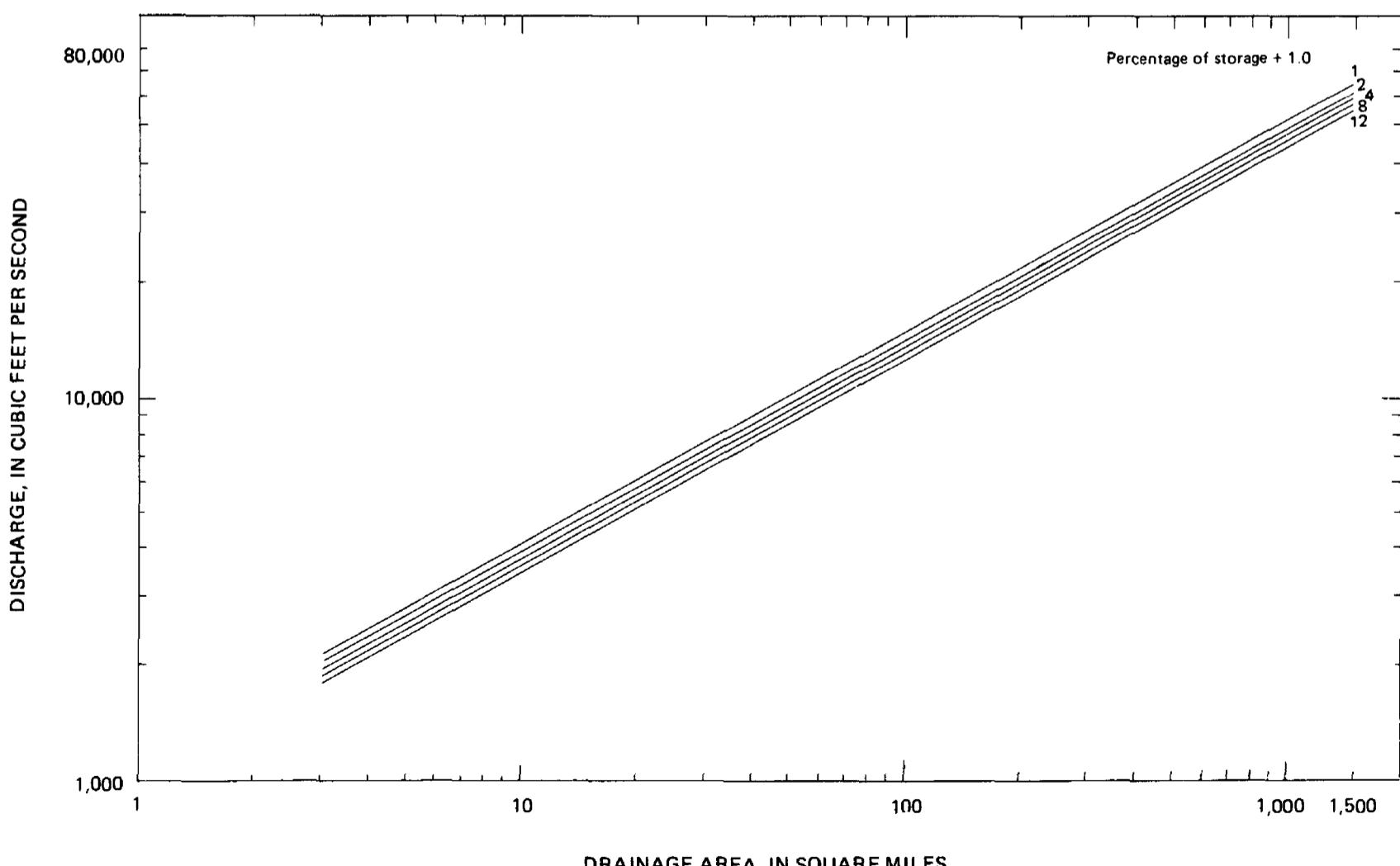


Figure 9. Relation of 100-year flood magnitudes to drainage area for selected storage factors in area 2.

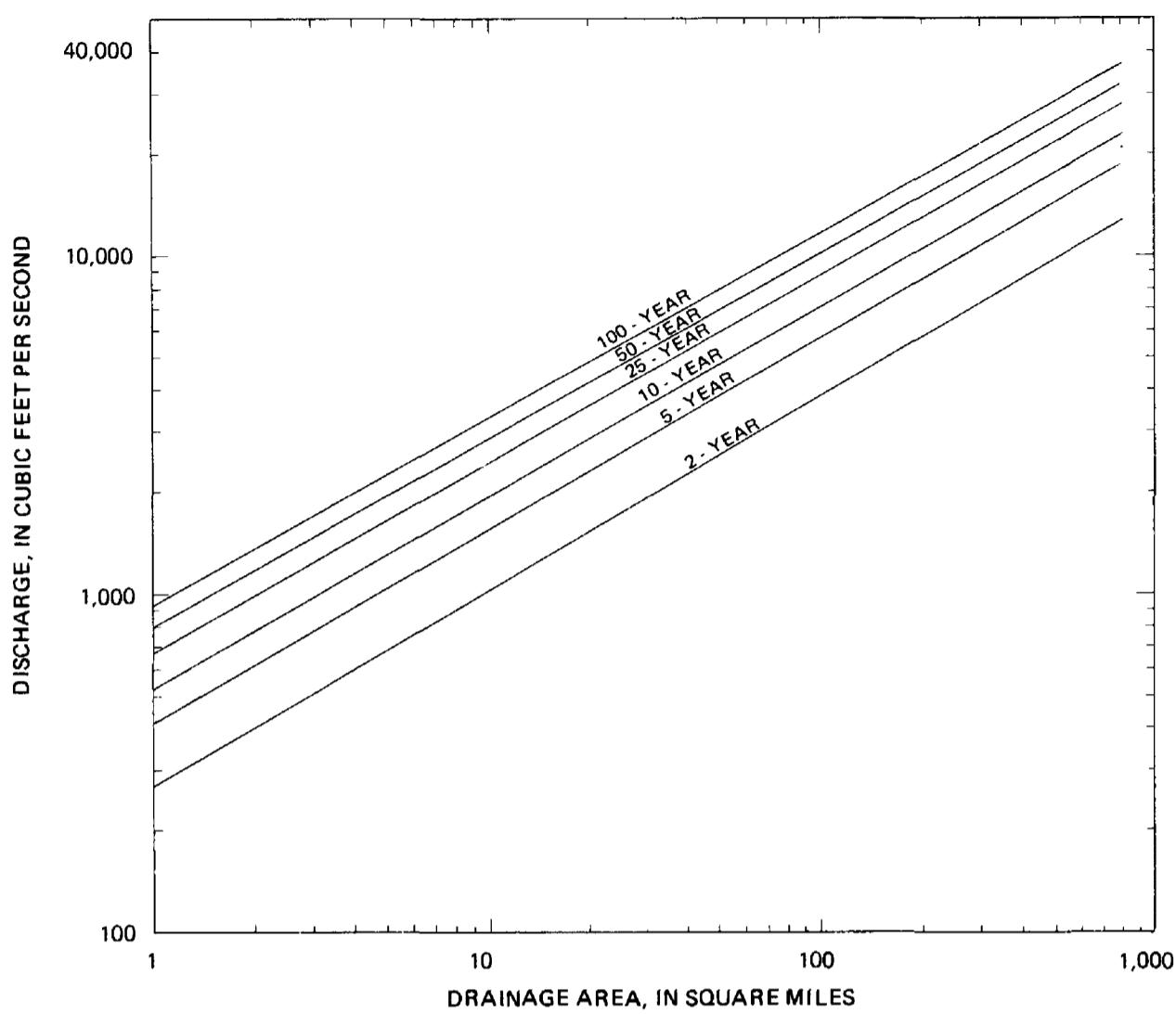


Figure 10. Relation of flood magnitudes to drainage area for selected recurrence intervals in hydrologic area 3.

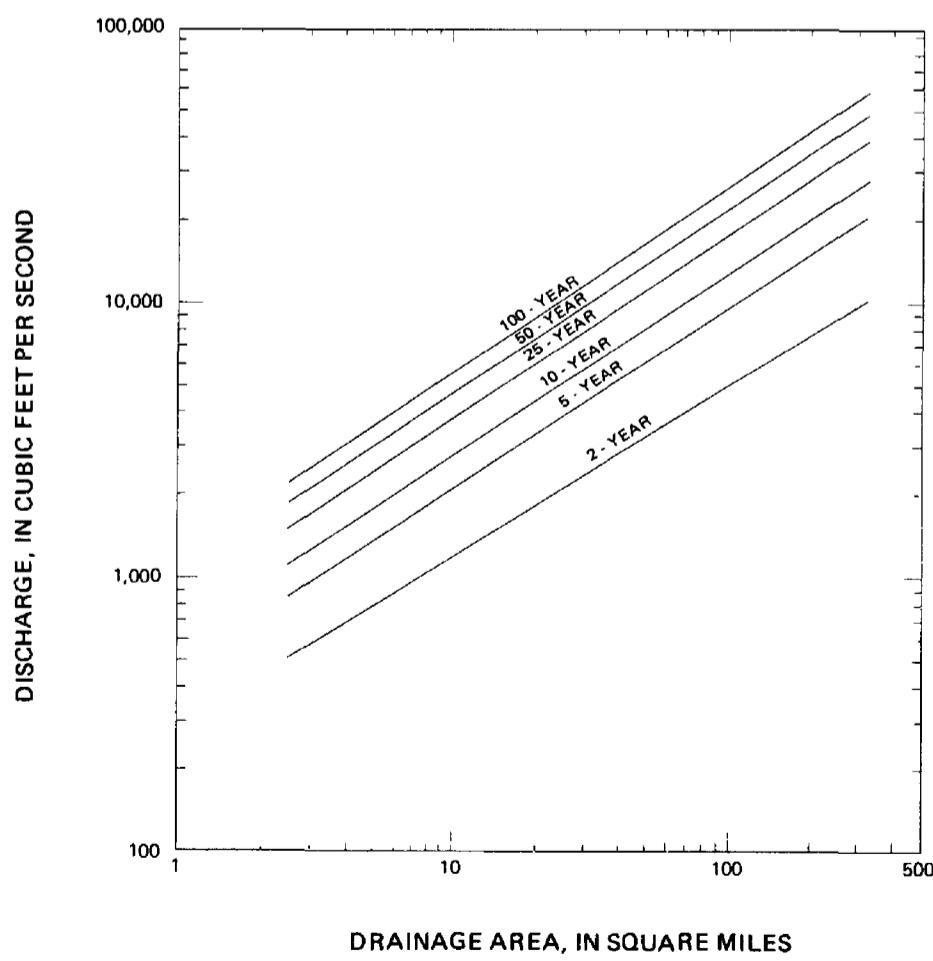


Figure 11. Relation of flood magnitudes to drainage area for selected recurrence intervals in hydrologic area 4.

discharges for stream gaging stations through September 1981 were used to compute flood magnitudes for 2-, 5-, 10-, 25-, 50-, and 100-year recurrence intervals by the log-Pearson Type III distribution. Flood magnitudes for 37 rural stations, computed from synthetic data, were available from a previous report.

Regression analyses were used to derive equations for estimating flood magnitudes for selected recurrence intervals using flood frequency data from 197 of 234 stations. Seven different hydrologic areas have been delineated. Equations using drainage area as the independent variable apply to five of the hydrologic areas. One hydrologic area (Little River basin) cannot be regionalized. Hydrologic area 2 has equations using both drainage area and storage as independent variables. Flood runoff in hydrologic area 4 was found to be two to four times greater than in other areas because of an impervious surface developed mainly on chalk and marl. Hydrologic area 3 has low flood runoff due to a high infiltration rate that transmits water to the ground-water reservoir.

Equations for estimating flood magnitudes for urban streams taken from a previous report were given. These equations use both drainage area and percentage of impervious cover as independent variables. A comparison of these urban equations to the rural areal equations was made, and the rural equations for hydrologic area 4 sometimes estimated higher flood runoff than the urban equations. In hydrologic area 4, the rural areal equations are recommended to be used in estimating flood magnitudes for both urban and rural streams.

The rural areal equations should be used on rural streams with drainage areas of 1,500 mi² or less where the flood runoff is not affected by man. Rivers with drainage areas greater than 1,500 mi² cannot be regionalized. To estimate flood magnitudes, graphs plotting flood magnitudes to drainage area for selected recurrence intervals were given for each river.

The maximum floods recorded at all gaging stations were given both in tabular and graphical form. The graph shows the plot of

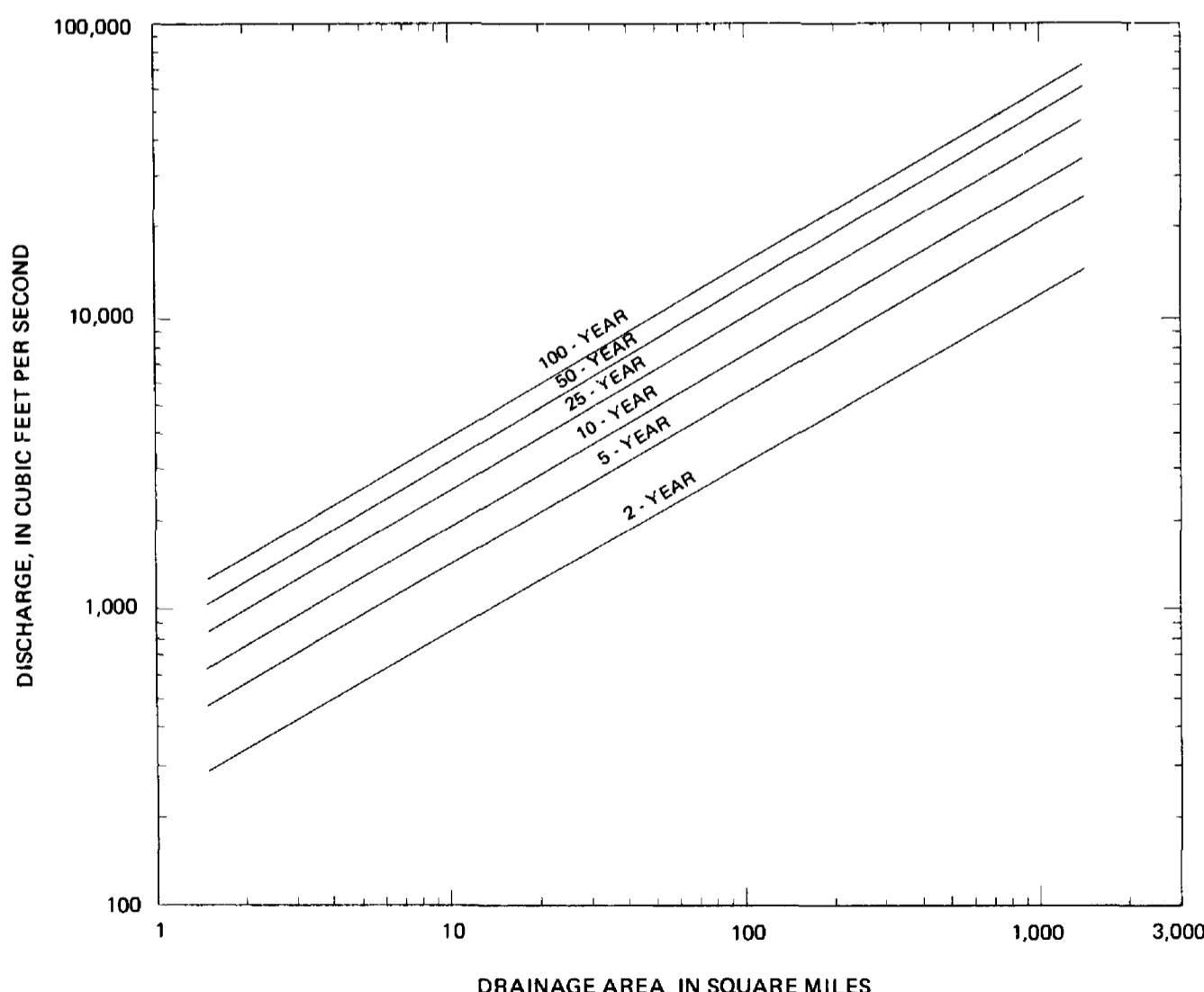


Figure 12. Relation of flood magnitudes to drainage area for selected recurrence intervals in hydrologic area 5.

maximum floods in cubic feet per second per square mile to drainage area. On this graph the Meyer's and Creager's curves showing the upper envelope of the plotted maximum magnitudes is drawn. The "C" values for the Meyer's equation is 7,000 and for Creager's, 54. The "C" values used previously in Alabama were 3,000 for Meyer's equation and 30 for Creager's equation.

SELECTED REFERENCES

- Barnes, H. H., Jr., and Golden, H. G., 1966, Magnitude and frequency of floods in the United States, Part 2-B, South Atlantic and Eastern Gulf of Mexico basins: U.S. Geological Survey Water-Supply Paper 1674, 409 p.
- Bridges, W. C., 1982, Technique for estimating magnitude and frequency of floods on natural-flow streams in Florida: U.S. Geological Survey Water-Resources Investigations 82-4012, 44 p.
- Creager, W. P., and Justin, J. D., 1950 Hydroelectric handbook: New York, John Wiley and Sons, Inc., Second Edition, 1151 p.
- Gamble, C. R., 1965, Magnitude and frequency of floods in Alabama: Alabama Highway Department HPR Report No. 5, 42 p.
- Hains, C. F., 1973, Floods in Alabama, magnitude and frequency: Alabama Highway Department, 174 p.
- Hardison, C. H., 1971, Prediction error of regression estimates of streamflow characteristics at ungaged sites: U.S. Geological Survey Professional Paper 750C, p. 228-236.

Jarvis, C. S., 1926, Flood flow characteristics: American Society of Civil Engineers Trans., v. 89, p. 985-1032.

Olin, D. A., and Bingham, R. H., 1982, Synthesized flood frequency of urban streams in Alabama: U.S. Geological Survey Water-Resources Investigations 82-683, 35 p.

— 1977, Flood frequency of small streams in Alabama: Alabama Highway Department, HPR Report No. 83, 44 p.

Peirce, L. B., 1954, Floods in Alabama, magnitude and frequency: U.S. Geological Survey Circular 342, 105 p.

Price, McGlone, 1979, Floods in Georgia, magnitude and frequency: U.S. Geological Survey Water-Resources Investigations 78-137, 269 p.

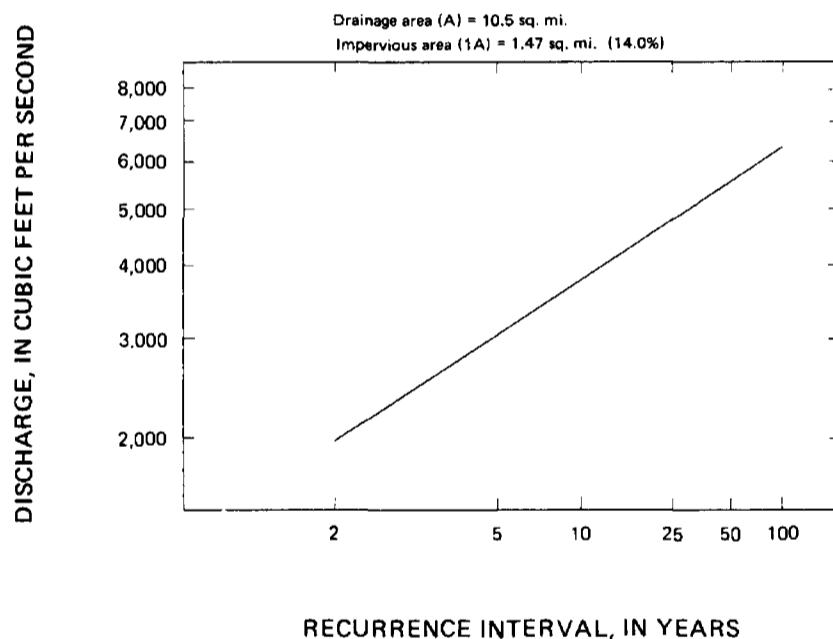


Figure 13. Flood-frequency curve for example urban stream.

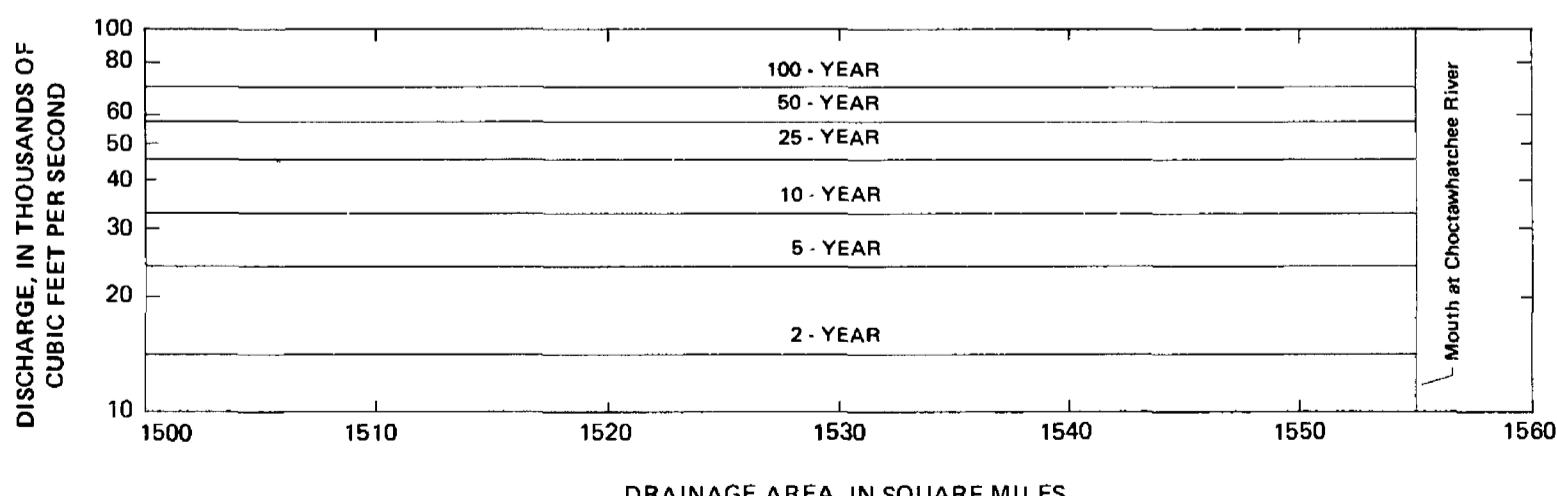


Figure 14. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Pea River.

Randolph, W. J., and Gamble, C. R., 1976, Technique for estimating magnitude and frequency of floods in Tennessee: Tennessee Department of Transportation 207, 52 p.

Speer, P. R., and Gamble, C. R., 1964, Magnitude and frequency of floods in the

United States, Part 3-B, Cumberland and Tennessee River basins: U.S. Geological Survey Water-Supply Paper 1676, 340 p.

U.S. Water Resources Council, 1981, Guidelines for determining flood flow frequencies: U.S. Water Resources Council Bulletin 17B, 28 p. 14 app.

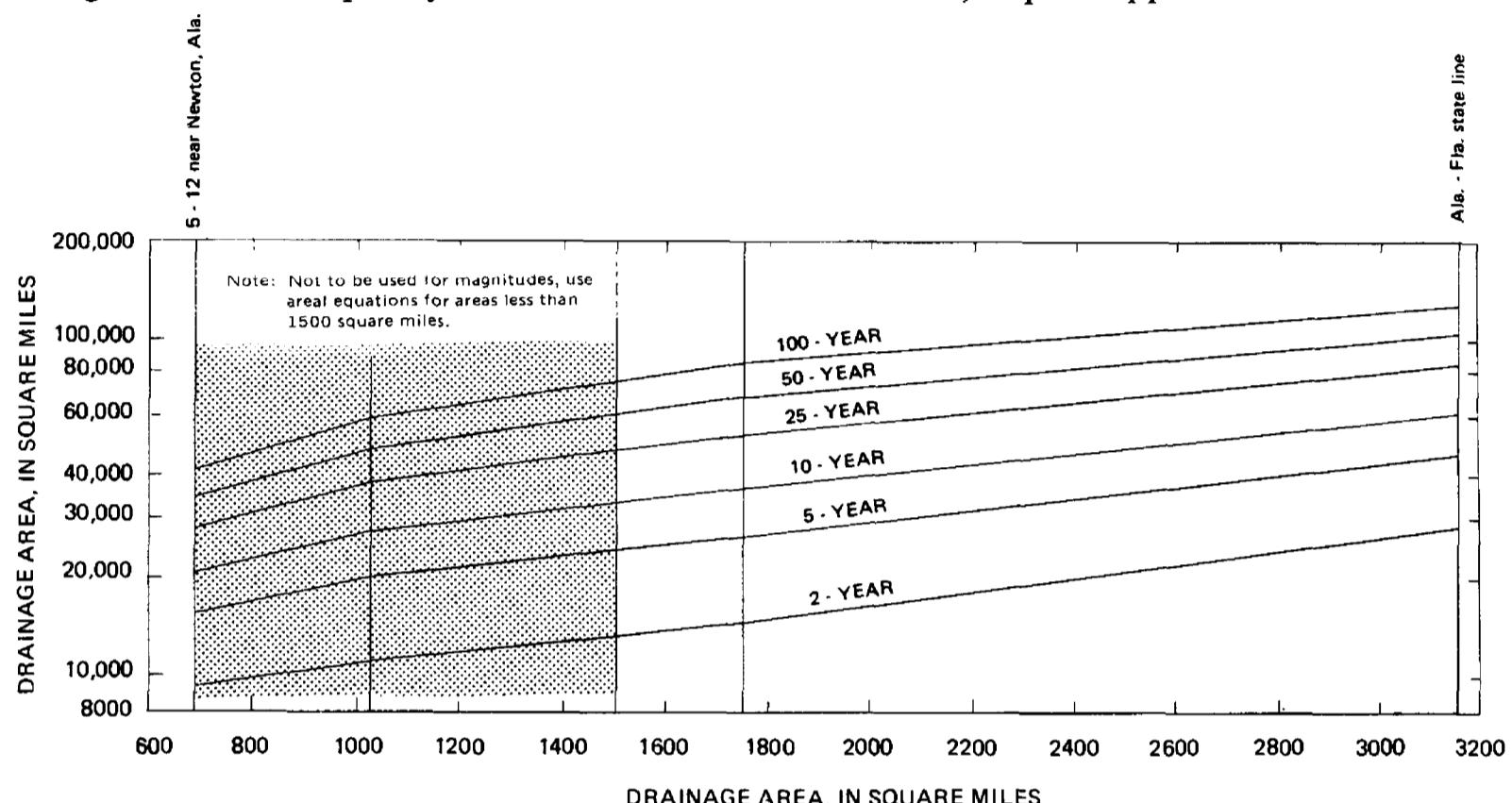


Figure 15. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Choctawhatchee River.

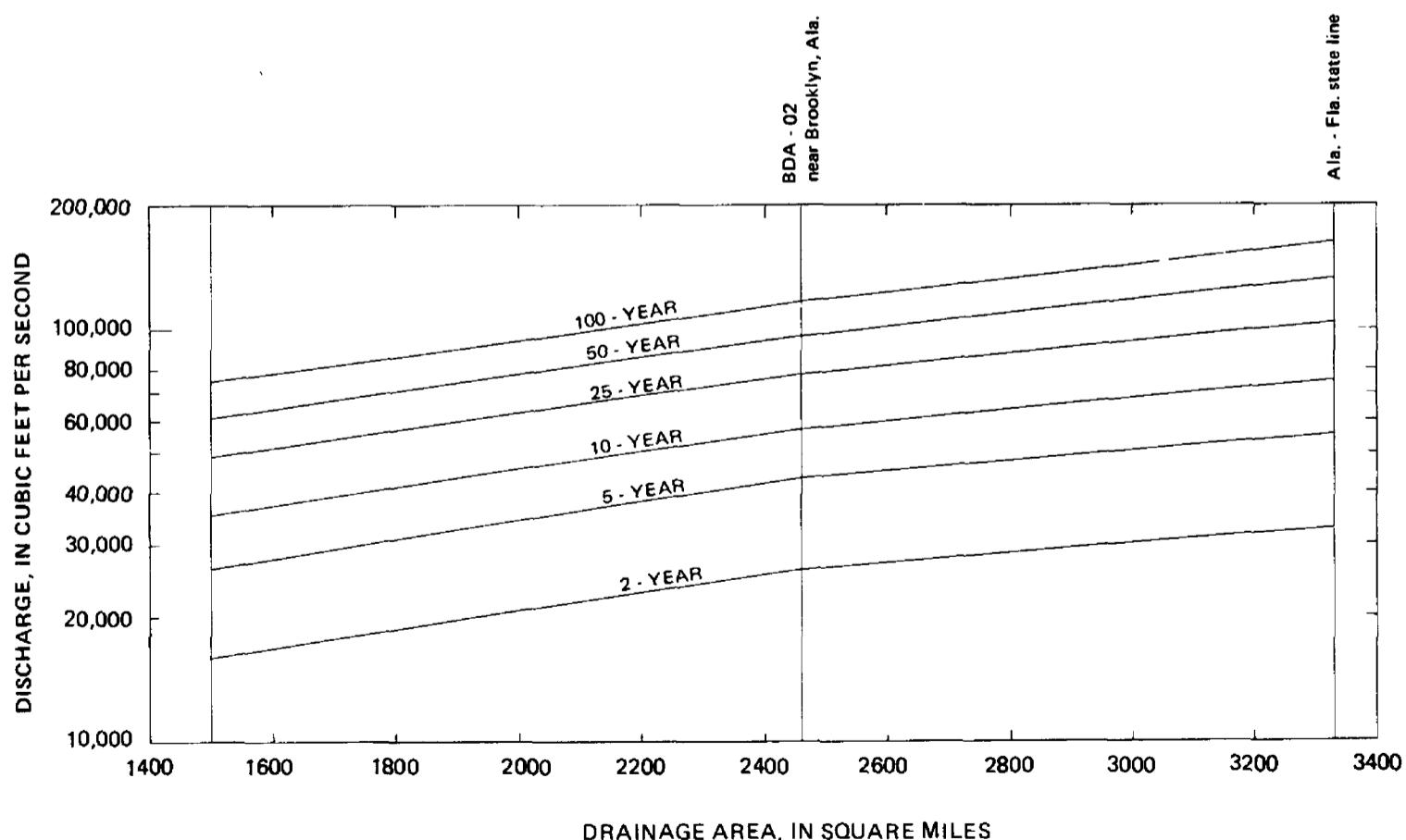


Figure 16. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Conecuh River.

U.S. Weather Bureau, 1961, Rainfall frequency atlas of the United States: U.S. Weather Bureau Technical Paper 40, 115 p.

U.S. Weather Service, 1957, Climates of the States, v. 1-Eastern States.

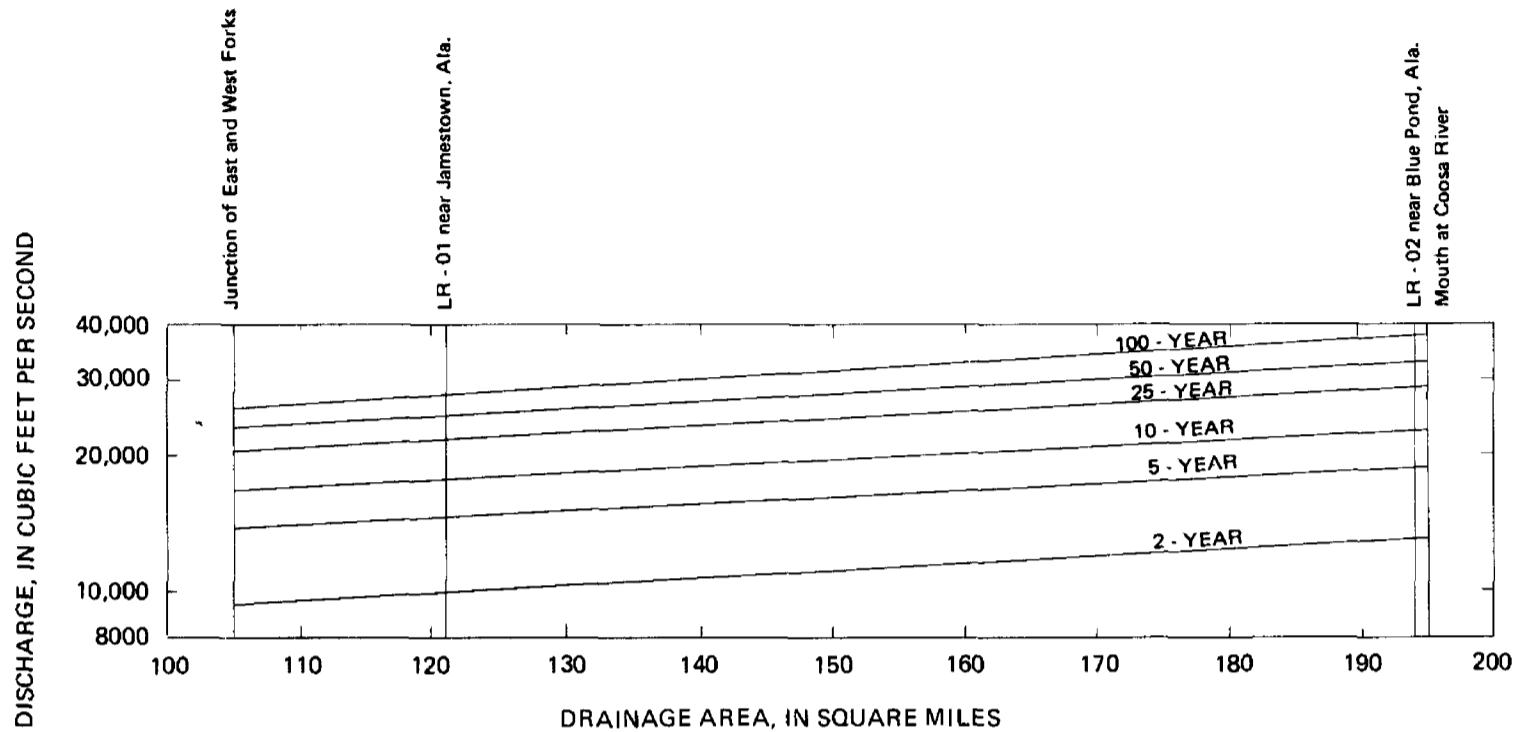
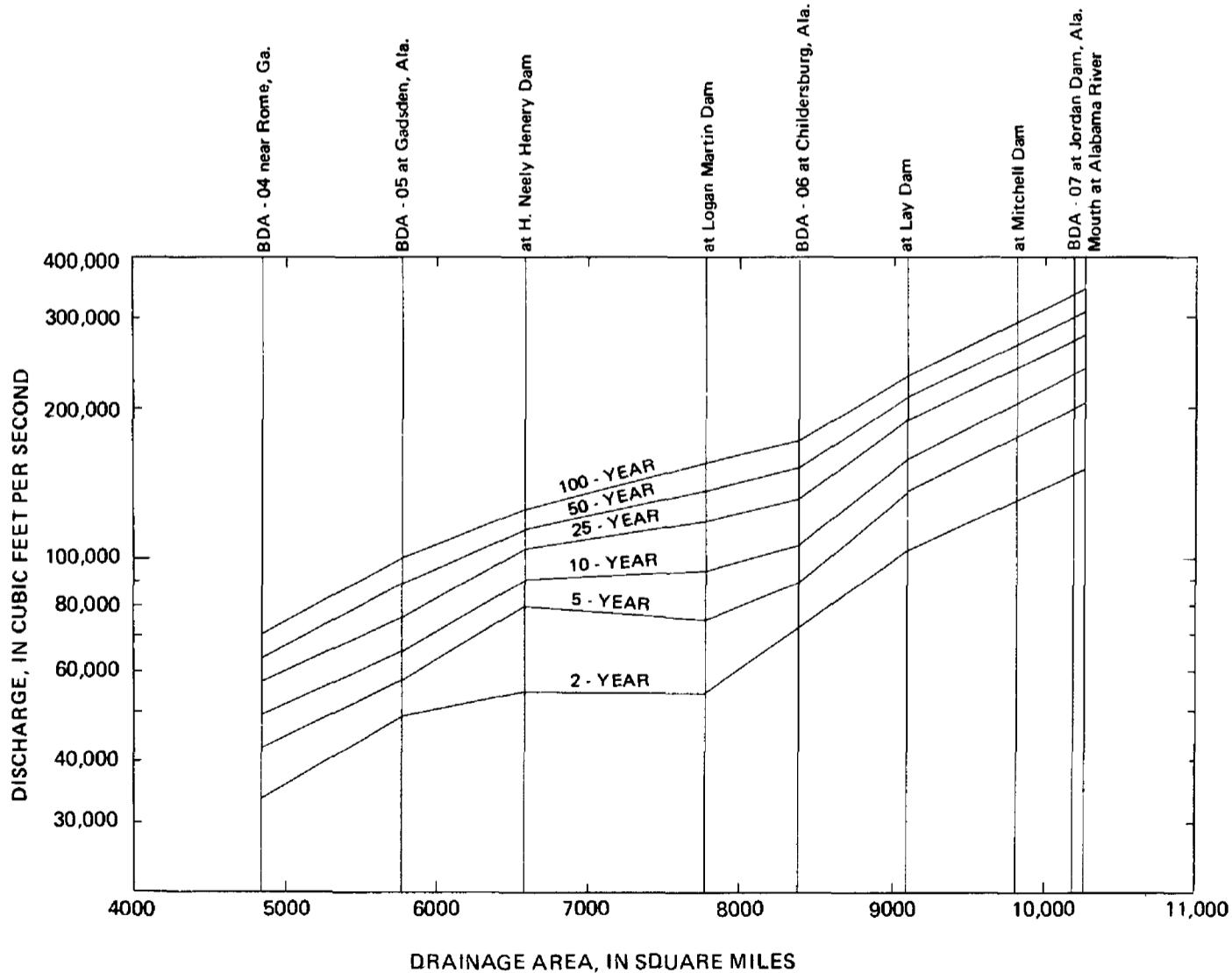


Figure 17. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Little River.



Data from U.S. Army Corps of Engineers, Mobile, Ala. District

Figure 18. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Coosa River.

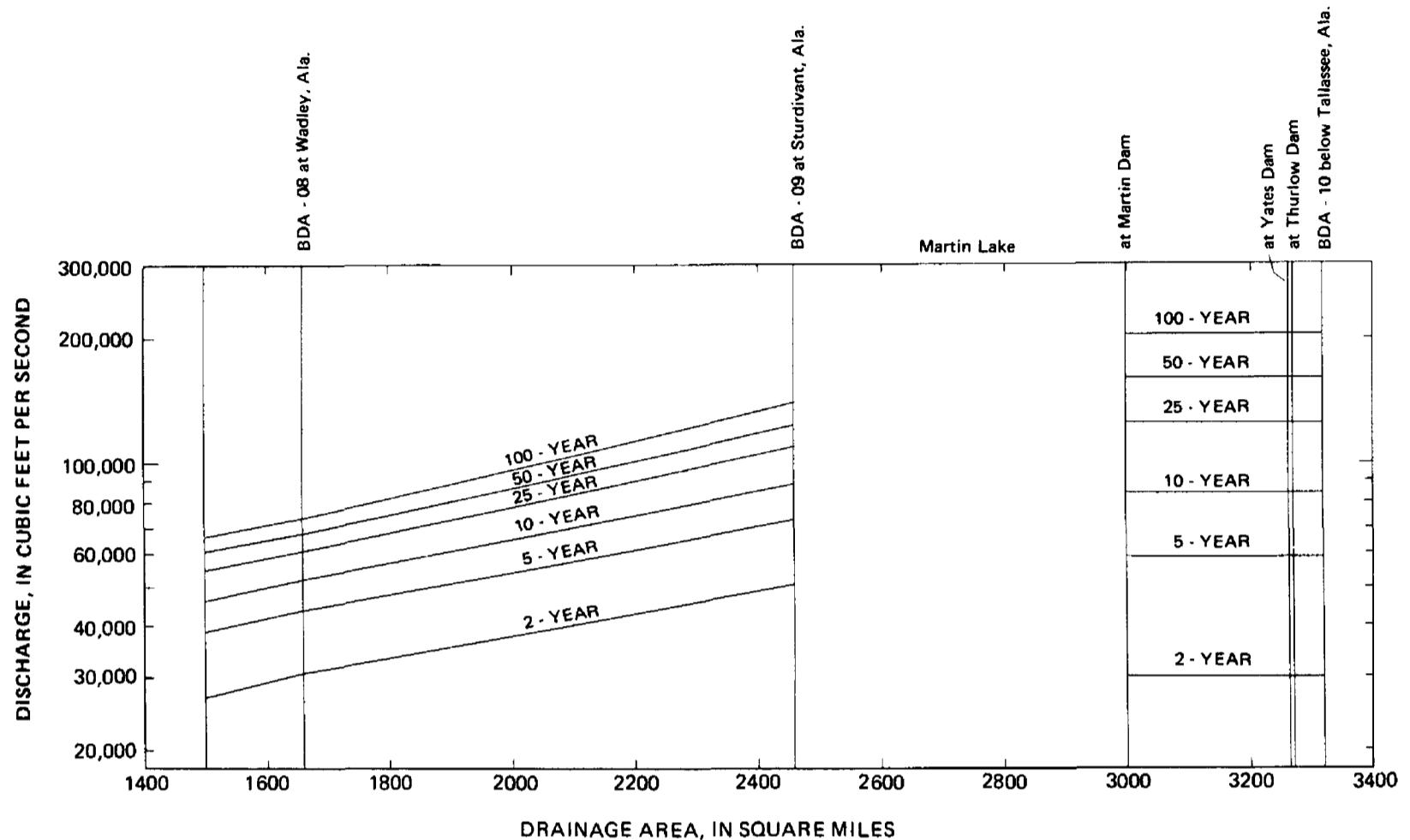


Figure 19. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Tallapoosa River.

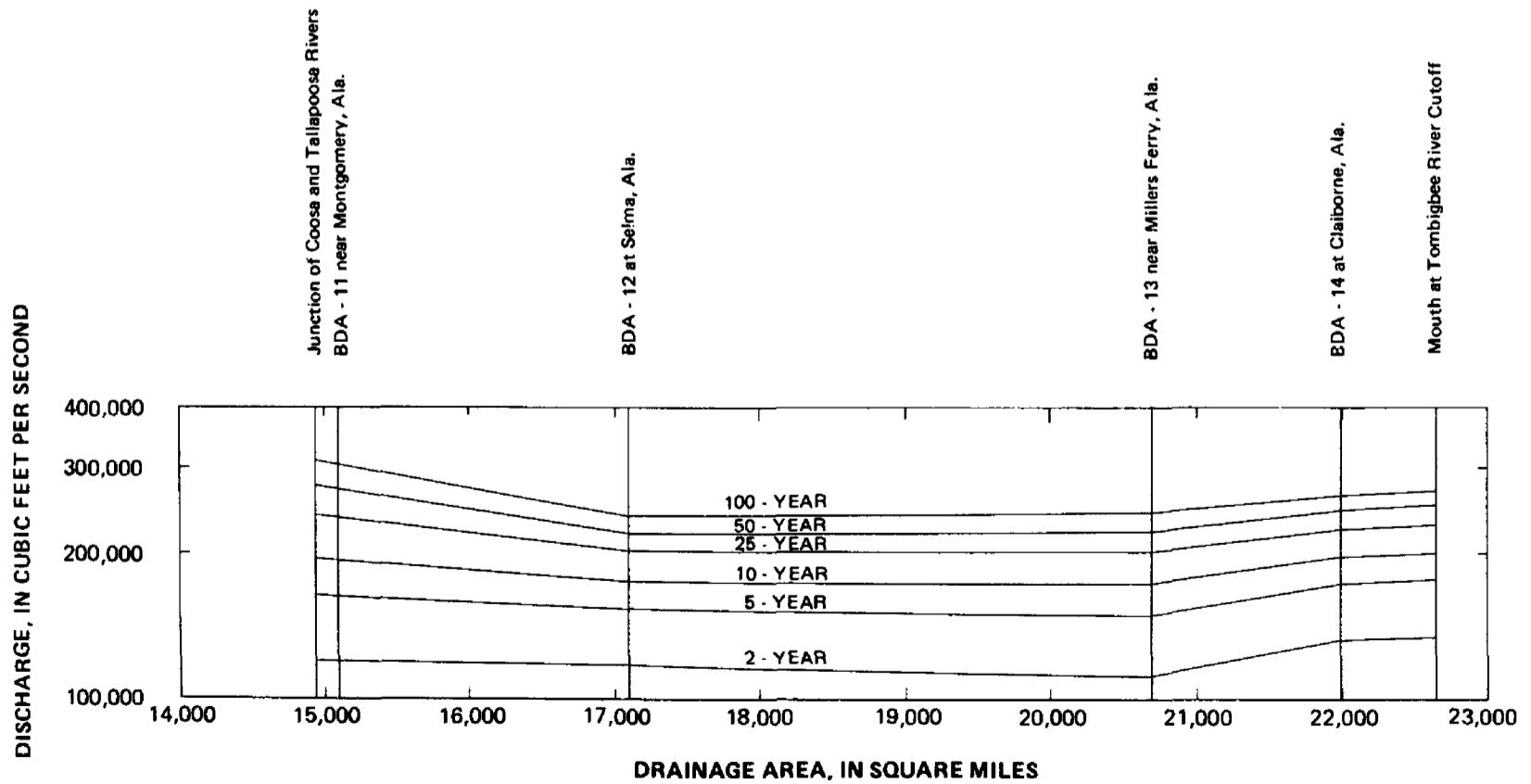


Figure 20. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Alabama River.

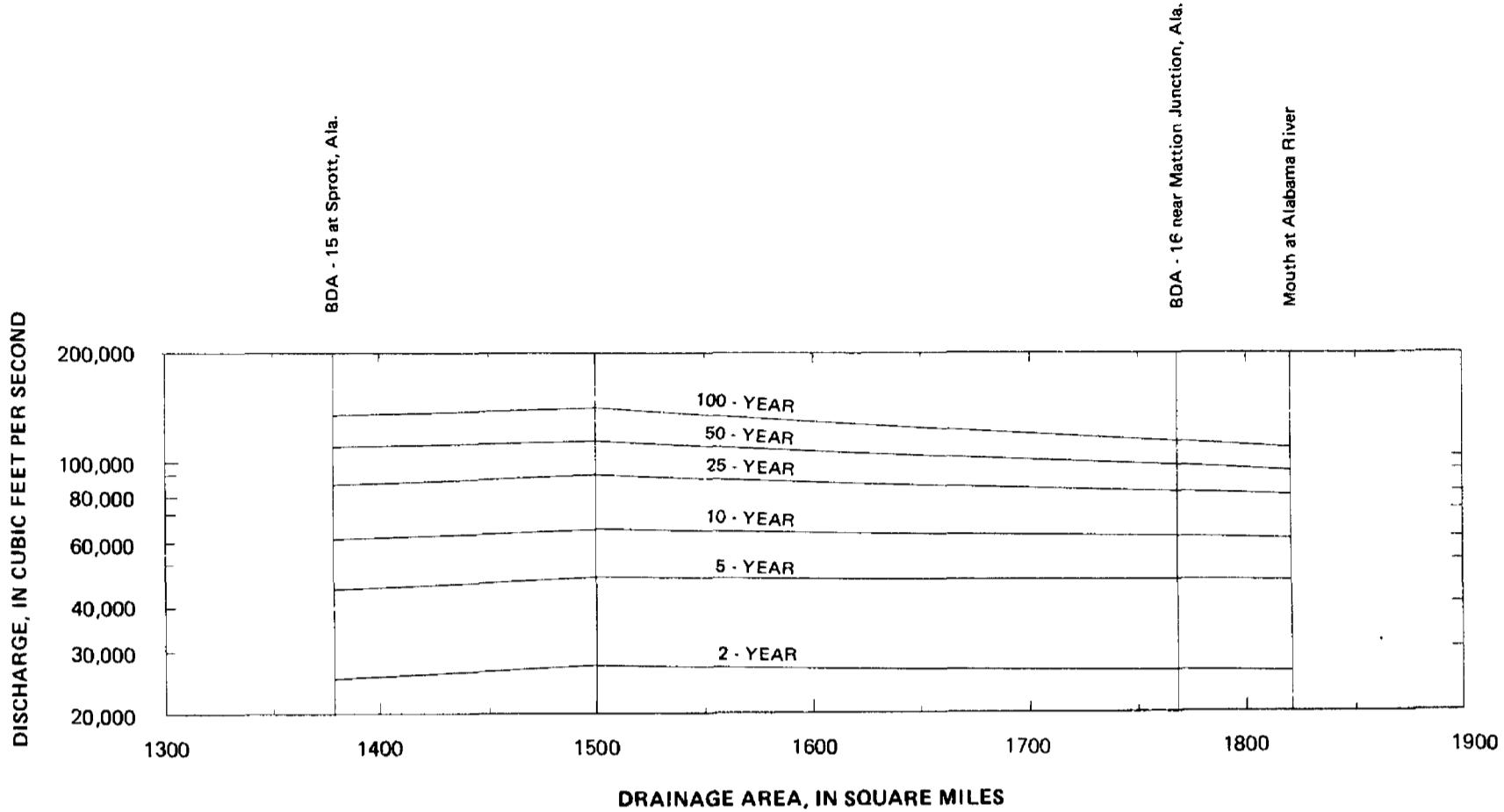


Figure 21. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Cahaba River.

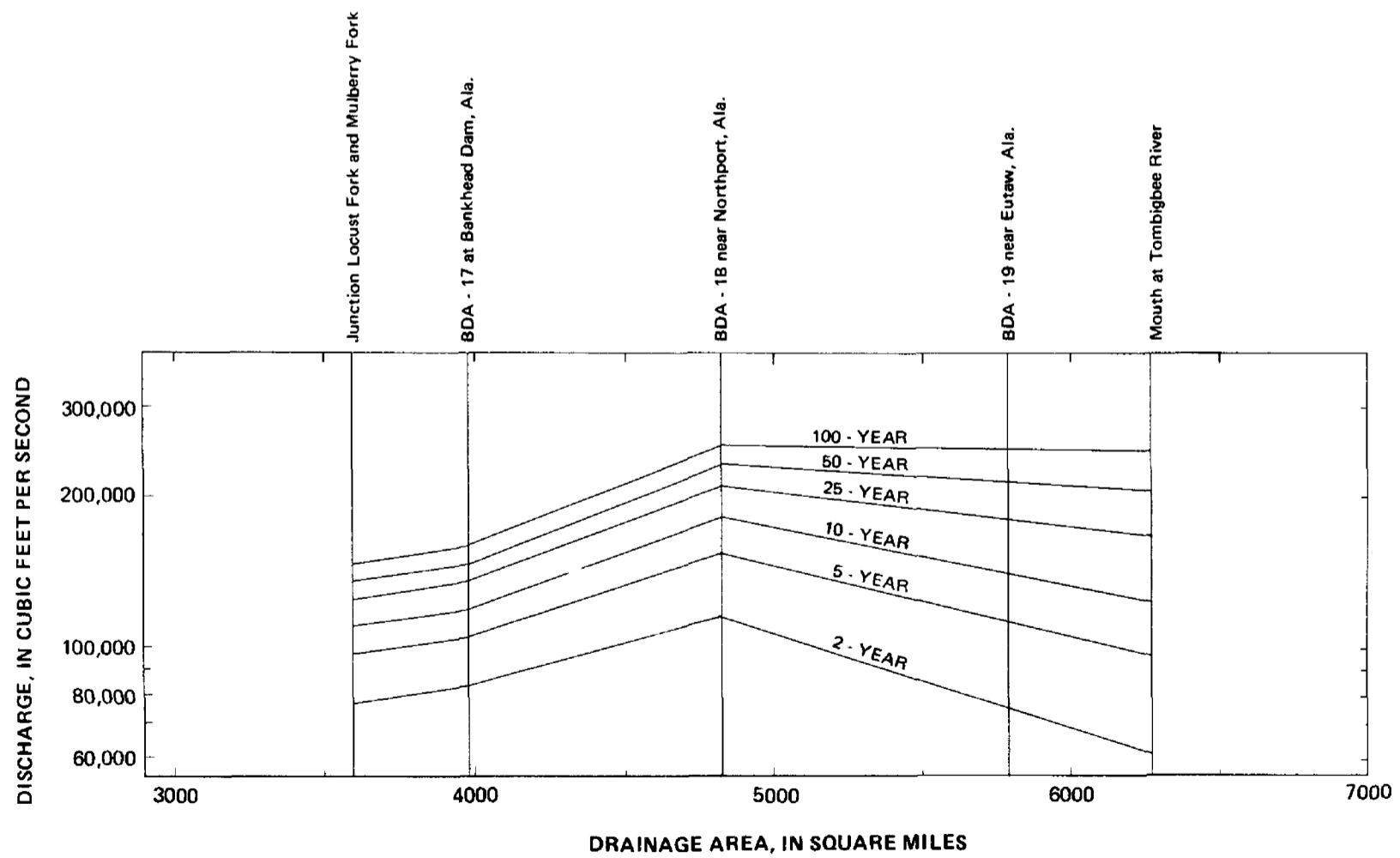


Figure 22. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Black Warrior River.

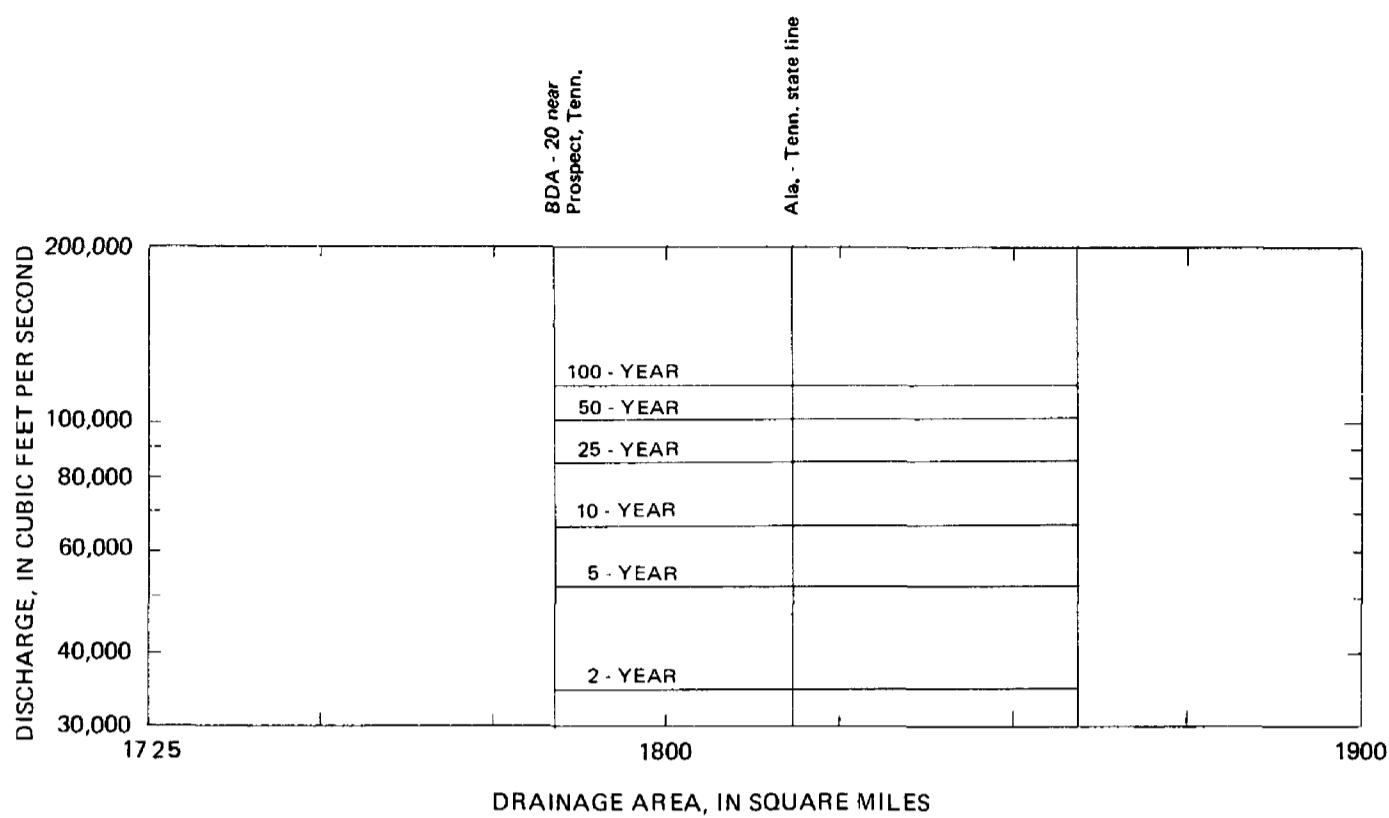


Figure 23. Relation of flood magnitudes to drainage area for selected recurrence intervals for the Elk River.

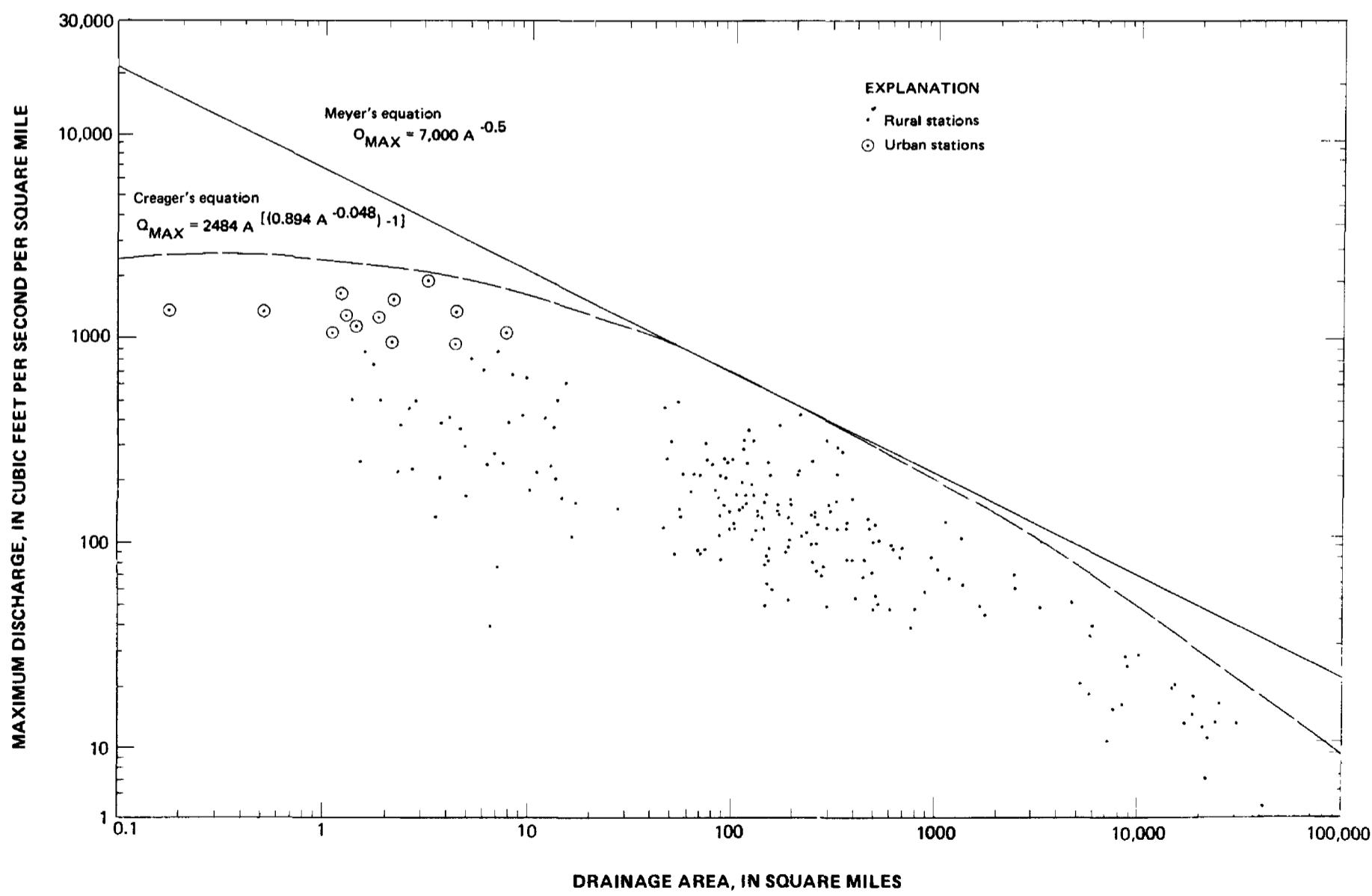


Figure 24. Maximum discharge for streams in Alabama.

Table 1. Size distribution, number of stations, and harmonic mean length of record for stations in hydrologic areas 1-6 regression analyses

Drainage area (m ²)	Hydrologic areas					
	1	2	3	4	5	6
Less than 25	18	51.84	1	50.00	4	50.00
26-50	6	45.50	0	1	20.00	1
51-100	14	35.18	1	43.00	4	31.25
101-300	30	41.97	5	43.70	5	42.47
301-500	13	73.80	2	54.28	1	61.00
501-800	3	63.29	3	63.23	2	75.44
801-1,000	2	30.90	1	85.00	0	0
1,001-1,500	1	80.00	1	89.00	0	0
Total	87	14	17	19	50	8

Table 2. Ranges of slope, mean annual precipitation, and 24-hour 2-year rainfall; and values of mean station skew, mean standard deviation of the logarithms of the annual peaks, and the interstation correlation coefficient for stations in hydrologic areas 1-6 regression analyses

	Hydrologic areas					
	1	2	3	4	5	6
Slope (ft/mi)	1.80 - 286.20	2.30 - 47.50	2.87 - 99.70	3.30 - 83.30	2.50 - 59.20	5.51 - 13.50
Mean annual precipitation (in.)	49.0 - 60.0	50.0 - 54.0	50.0 - 54.0	49.0 - 56.0	49.0 - 67.0	60.0 - 68.0
24-hour 2-year inter-val rainfall (in.)	3.40 - 4.45	4.00 - 4.50	3.20 - 4.30	4.25 - 4.80	3.85 - 5.60	4.53 - 6.20
Mean station skew (\log_{10})	+ 0.004	+ 0.153	- 0.106	+ 0.080	+ 0.128	+ 0.308
Mean standard deviation of logarithms of the annual peaks (\log_{10})	0.2451	0.2602	0.2265	0.2871	0.2798	0.3448
Interstation correlation coefficient	0.03326 *	0.03038	0.02920	0.05914	0.02683	0.03326 *

* Computed from hydrologic areas 1 and 6 combined.

Table 3.--Summary of regression equations

Regression equations for indicated hydrologic areas,
where A = drainage area in square miles, ST = storage
in percent of basin

Recurrence Interval (Years)	Hydrologic Areas				
	1 & 6*	2	3	4	5
2	$182A^{0.706}$	$149A^{0.689} (ST+1.0)^{-0.219}$	$270A^{0.569}$	$292A^{0.631}$	$226A^{0.567}$
5	$291A^{0.711}$	$310A^{0.642} (ST+1.0)^{-0.172}$	$419A^{0.566}$	$480A^{0.647}$	$376A^{0.577}$
10	$372A^{0.714}$	$459A^{0.616} (ST+1.0)^{-0.142}$	$524A^{0.564}$	$630A^{0.653}$	$495A^{0.582}$
25	$483A^{0.717}$	$696A^{0.590} (ST+1.0)^{-0.109}$	$675A^{0.559}$	$845A^{0.660}$	$668A^{0.587}$
50	$571A^{0.720}$	$904A^{0.574} (ST+1.0)^{-0.084}$	$807A^{0.554}$	$1024A^{0.665}$	$813A^{0.591}$
100	$664A^{0.722}$	$1144A^{0.560} (ST+1.0)^{-0.060}$	$937A^{0.550}$	$1215A^{0.669}$	$972A^{0.595}$

* Flood runoff from hydrologic areas 1 and 6 was found to be similar and stations in both areas were used in a single regression in the final analysis. Area 6 has more intense and larger amounts of rainfall, but gentler land slopes than area 1 (Supplementary Data I). As factors influencing floods, one counteracts the other, allowing one equation to be used to estimate floods for both areas. Statistically there was less than 10 percent difference in flood magnitude estimates between the two equations.

Table 4.--Accuracy of regression equations
Standard error of regression, in percent

Hydrologic Area	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}
1 & 6	33	29	29	31	33	36
2	30	24	21	20	22	25
3	29	26	25	27	29	31
4	46	37	35	34	34	34
5	26	26	24	25	27	29

	Equivalent years of record ¹					
	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}
1 & 6	3	6	8	9	12	15
2	4	9	16	25	34	38
3	3	5	7	9	11	14
4	2	5	7	11	16	22
5	6	11	15	19	25	31

Ratio of the standard error of estimate of the regression to the time-sampling error						
	Q_2	Q_5	Q_{10}	Q_{25}	Q_{50}	Q_{100}
1 & 6	3.73	2.85	2.59	2.18	1.91	1.73
2	3.69	2.44	1.84	1.48	1.28	1.20
3	3.38	2.66	2.23	1.98	1.78	1.60
4	4.82	3.35	2.69	2.21	1.80	1.54
5	2.75	2.07	1.76	1.56	1.36	1.23

¹ computed from standard error of regression.

Table 5. Flood magnitude and frequency for Mobile River at the Barry Steam Plant near Bucks, Ala.

Flood Magnitudes (ft ³ /s)		Log-Pearson Type III parameters		
Q2	283,000	Q25	479,000	STATION SKW
Q5	362,000	Q50	527,000	WRC MEAN
Q10	414,000	Q100	575,000	5.455-LOG10
				YRSPK
				YRSHISPK
				53-YEARS
				30-YEARS
STREAMFLOW CHARACTERISTICS FILE		FILE		
MASTER LIST OF VARIABLES		FILE	CODE	DESCRIPTION
Q2	Annual flood peak, in CFS, of T-year recurrence interval, defined by Log-Pearson Type III fitting, from computer program no. J407.	WRC SKW		WRC skew of logarithms, base 10, of annual peak discharge after outlier and historic-peak adjustments and generalized skew weighting, from computer program no. J407.
Q5	Do. T = 5.	WRC MEAN		WRC mean of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments, from computer program no. J407.
Q10	Do. T = 10.	WRC SD		WRC standard deviation of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments, from computer program no. J407.
Q25	Do. T = 25.			
Q50	Do. T = 50.			
Q100	Do. T = 100.			
		YRSPK		Number of years of systematic peak-flow record.
		YRSHISPK		Number of consecutive years used for historic-peak adjustments to flood frequency data.

SUPPLEMENTARY DATA I

Basin characteristics and locations of stream gaging stations.

BASIN AND STREAMFLOW CHARACTERISTICS FILE

MASTER LIST OF VARIABLES

Data type, identifier for data which is based
on synthetic record.

SP - Floodflow characteristics based on
synthetic or combined synthetic and
observed record.

FILE CODE	DESCRIPTION
A	Drainage area, in square miles, that contributes to surface runoff.
S	Main channel slope, in feet per mile, is the average slope of the main channel between 10 and 85 percent of the total channel length from the gaging station to the basin divide.
ST	Area of the basin that is occupied by swamps, ponds, or reservoirs, in percent.
LAT	Latitude of gaging station, in decimal degrees.
LONG	Longitude of gaging station, in decimal degrees.
P	Mean annual precipitation, in inches, from U.S. Weather Bureau series, "Climates of States"; grid sampling methods used if isohyetal map is available, otherwise anomaly map constructed (WSP 1580-D).
I24-2	Precipitation intensity; 24-hour rainfall, in inches, expected on the average of once each 2 years. (Estimated from U.S. Weather Bureau Technical Paper 40 except for western States where NOAA Atlas 2 exists).
Q2	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 2.

FILE CODE	DESCRIPTION
Q5	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 5.
Q10	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 10.
Q25	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 25.
Q50	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 50.
Q100	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 100.
WRC SKEW	WRC skew of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments and generalized skew weighting, from computer program no. J407.
WRC MEAN	WRC mean of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments, from computer program no. J407.
WRC SD	WRC standard deviation of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments, from computer program no. J407.
YRSPK	Number of years of systematic peak-flow record.
YRSHISPK	Number of consecutive years used for historic-peak adjustments to flood frequency data.

HYDROLOGIC AREA 1

1-01 02397500 CEDAR CREEK NEAR CEDARTOWN, GA.

A=	109-SQ MI	S=	4.730-FT/MI	LAT=	34.06-DEC. DEG
LONG=	85.31-DEC. DEG	P=	51.50-IN	I24-2=	3.400-IN
Q2=	5200-CFS	Q5=	8000-CFS	Q10=	10000-CFS
Q25=	12800-CFS	Q50=	14900-CFS	Q100=	17200-CFS
WRC SKEW=	0.004-LOG10	WRC MEAN=	3.716-LOG10	WRC SD=	0.223-LOG10
YRSPK=	32-YEARS	YRSHISPK=	NONE-YEARS		

1-02 02398000 CHATTOOGA RIVER AT SUMMERTVILLE, GA.

A=	193-SQ MI	S=	6.600-FT/MI	LAT=	34.47-DEC. DEG
LONG=	85.34-DEC. DEG	P=	52.50-IN	I24-2=	3.800-IN
Q2=	8560-CFS	Q5=	13700-CFS	Q10=	17500-CFS
Q25=	22600-CFS	Q50=	26700-CFS	Q100=	30900-CFS
WRC SKEW=	-0.034-LOG10	WRC MEAN=	3.931-LOG10	WRC SD=	0.243-LOG10
YRSPK=	37-YEARS	YRSHISPK=	NONE-YEARS		

1-03 02398300 CHATTOOGA RIVER ABOVE GAYLESVILLE, ALA.

A=	368-SQ MI	S=	7.000-FT/MI	LAT=	34.29-DEC. DEG
LONG=	85.51-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	9570-CFS	Q5=	14400-CFS	Q10=	17700-CFS
Q25=	22100-CFS	Q50=	25500-CFS	Q100=	29000-CFS
WRC SKEW=	-0.055-LOG10	WRC MEAN=	3.979-LOG10	WRC SD=	0.211-LOG10
YRSPK=	51-YEARS	YRSHISPK=	63-YEARS		

1-04 02398500 CHATTOOGA RIVER AT GAYLESVILLE, ALA.

A=	377-SQ MI	S=	7.000-FT/MI	LAT=	34.27-DEC. DEG
LONG=	85.57-DEC. DEG	P=	53.00-IN	I24-2=	3.800-IN
Q2=	9600-CFS	Q5=	15000-CFS	Q10=	19300-CFS
Q25=	25400-CFS	Q50=	30500-CFS	Q100=	36200-CFS
WRC SKEW=	0.290-LOG10	WRC MEAN=	3.993-LOG10	WRC SD=	0.223-LOG10
YRSPK=	23-YEARS	YRSHISPK=	44-YEARS		

1-05 02399800 LITTLE TERRAPIN CR NEAR BORDEN SPRINGS, ALA. SP

A=	15.9-SQ MI	S=	32.400-FT/MI	LAT=	33.92-DEC. DEG
LONG=	85.46-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	1260-CFS	Q5=	2150-CFS	Q10=	2840-CFS
Q25=	3810-CFS	Q50=	4620-CFS	Q100=	5440-CFS
WRC SKEW=	-0.054-LOG10	WRC MEAN=	3.908-LOG10	WRC SD=	0.279-LOG10
YRSPK=	8-YEARS	YRSHISPK=	50-YEARS		

1-06 02400000 TERRAPIN CREEK NEAR PIEDMONT, ALA.

A=	115-SQ MI	S=	24.000-FT/MI	LAT=	33.96-DEC. DEG
LONG=	85.58-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	8730-CFS	Q5=	13900-CFS	Q10=	17500-CFS
Q25=	22100-CFS	Q50=	25500-CFS	Q100=	29000-CFS
WRC SKEW=	-0.288-LOG10	WRC MEAN=	3.929-LOG10	WRC SD=	0.252-LOG10
YRSPK=	35-YEARS	YRSHISPK=	NONE-YEARS		

1-07 02400033 NANCES CREEK NEAR WHITE PLAINS, ALA. SP

A=	4.60-SQ MI	S=	166.000-FT/MI	LAT=	33.84-DEC. DEG
LONG=	85.67-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	835-CFS	Q5=	1260-CFS	Q10=	1560-CFS
Q25=	1940-CFS	Q50=	2240-CFS	Q100=	2530-CFS
WRC SKEW=	-0.086-LOG10	WRC MEAN=	2.918-LOG10	WRC SD=	0.216-LOG10
YRSPK=	5-YEARS	YRSHISPK=	50-YEARS		

1-08 02400100 TERRAPIN CREEK AT ELLISVILLE, ALA.

A=	258-SQ MI	S=	10.100-FT/MI	LAT=	34.06-DEC. DEG
LONG=	85.61-DEC. DEG	P=	52.00-IN	I24-2=	4.150-IN
Q2=	9360-CFS	Q5=	13700-CFS	Q10=	16300-CFS
Q25=	19500-CFS	Q50=	21700-CFS	Q100=	23800-CFS
WRC SKEW=	-0.437-LOG10	WRC MEAN=	3.956-LOG10	WRC SD=	0.210-LOG10
YRSPK=	19-YEARS	YRSHISPK=	30-YEARS		

1-09 02400690 JACKS CREEK NEAR FORT PAYNE, ALA. SP

A=	6.76-SQ MI	S=	33.000-FT/MI	LAT=	34.42-DEC. DEG
LONG=	85.80-DEC. DEG	P=	54.00-IN	I24-2=	3.800-IN
Q2=	730-CFS	Q5=	1200-CFS	Q10=	1560-CFS
Q25=	2070-CFS	Q50=	2490-CFS	Q100=	2920-CFS
WRC SKEW=	0.010-LOG10	WRC MEAN=	2.864-LOG10	WRC SD=	0.258-LOG10
YRSPK=	4-YEARS	YRSHISPK=	50-YEARS		

1-10 02401000 BIG WILLS CREEK NEAR CRUDUP, ALA.

A=	185-SQ MI	S=	6.400-FT/MI	LAT=	34.10-DEC. DEG
LONG=	86.03-DEC. DEG	P=	54.00-IN	I24-2=	3.900-IN
Q2=	5650-CFS	Q5=	9080-CFS	Q10=	11500-CFS
Q25=	14500-CFS	Q50=	16800-CFS	Q100=	19100-CFS
WRC SKEW=	-0.282-LOG10	WRC MEAN=	3.740-LOG10	WRC SD=	0.256-LOG10
YRSPK=	51-YEARS	YRSHISPK=	96-YEARS		

1-11 02401390 BIG CANOE CREEK AT ASHVILLE, ALA.

A=	148-SQ MI	S=	7.800-FT/MI	LAT=	33.84-DEC. DEG
LONG=	86.26-DEC. DEG	P=	53.00-IN	I24-2=	4.000-IN
Q2=	5900-CFS	Q5=	7980-CFS	Q10=	9460-CFS
Q25=	11500-CFS	Q50=	13000-CFS	Q100=	14700-CFS
WRC SKEW=	0.376-LOG10	WRC MEAN=	3.780-LOG10	WRC SD=	0.149-LOG10
YRSPK=	16-YEARS	YRSHISPK=	20-YEARS		

1-12 02401500 BIG CANOE CREEK NEAR GADSDEN, ALA.

A=	256-SQ MI	S=	7.500-FT/MI	LAT=	33.90-DEC. DEG
LONG=	86.11-DEC. DEG	P=	54.00-IN	I24-2=	3.900-IN
Q2=	7970-CFS	Q5=	12900-CFS	Q10=	17000-CFS
Q25=	23000-CFS	Q50=	28300-CFS	Q100=	34200-CFS
WRC SKEW=	0.365-LOG10	WRC MEAN=	3.916-LOG10	WRC SD=	0.239-LOG10
YRSPK=	37-YEARS	YRSHISPK=	81-YEARS		

1-13 02404000 CHOCCOLOCCO CREEK NEAR JENIFER, ALA.

A=	281-SQ MI	S=	6.800-FT/MI	LAT=	33.57-DEC. DEG
LONG=	85.93-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	7560-CFS	Q5=	14000-CFS	Q10=	18800-CFS
Q25=	25400-CFS	Q50=	30600-CFS	Q100=	36000-CFS
WRC SKEW=	-0.318-LOG10	WRC MEAN=	3.861-LOG10	WRC SD=	0.333-LOG10
YRSPK=	55-YEARS	YRSHISPK=	93-YEARS		

1-14 02404245 CHEAHA CREEK NEAR TALLADEGA, ALA.

A=	69.2-SQ MI	S=	46.000-FT/MI	LAT=	33.51-DEC. DEG
LONG=	86.02-DEC. DEG	P=	52.00-IN	I24-2=	4.100-IN
Q2=	2600-CFS	Q5=	5140-CFS	Q10=	7380-CFS
Q25=	10900-CFS	Q50=	14000-CFS	Q100=	17600-CFS
WRC SKEW=	0.050-LOG10	WRC MEAN=	3.417-LOG10	WRC SD=	0.350-LOG10
YRSPK=	29-YEARS	YRSHISPK=	43-YEARS		

1-15 02404400 CHOCCOLOCCO C AT JACKSON SHL NR LINCOLN, ALA.

A=	484-SQ MI	S=	6.700-FT/MI	LAT=	33.55-DEC. DEG
LONG=	86.10-DEC. DEG	P=	54.00-IN	I24-2=	4.050-IN
Q2=	10900-CFS	Q5=	20300-CFS	Q10=	28400-CFS
Q25=	40700-CFS	Q50=	51600-CFS	Q100=	64100-CFS
WRC SKEW=	0.124-LOG10	WRC MEAN=	4.045-LOG10	WRC SD=	0.315-LOG10
YRSPK=	50-YEARS	YRSHISPK=	96-YEARS		

1-16 02404500 CHOCCOLOCCO CREEK NEAR LINCOLN, ALA.

A=	499-SQ MI	S=	6.700-FT/MI	LAT=	33.56-DEC. DEG
LONG=	87.13-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	10900-CFS	Q5=	20300-CFS	Q10=	28400-CFS
Q25=	40700-CFS	Q50=	51600-CFS	Q100=	64100-CFS
WRC SKEW=	0.124-LOG10	WRC MEAN=	4.045-LOG10	WRC SD=	0.315-LOG10
YRSPK=	50-YEARS	YRSHISPK=	96-YEARS		

1-17 02405500 KELLY CREEK NEAR VINCENT, ALA.

A=	192-SQ MI	S=	8.900-FT/MI	LAT=	33.45-DEC. DEG
LONG=	86.39-DEC. DEG	P=	55.00-IN	I24-2=	4.100-IN
Q2=	7290-CFS	Q5=	12600-CFS	Q10=	17200-CFS
Q25=	24400-CFS	Q50=	30800-CFS	Q100=	38300-CFS
WRC SKEW=	0.369-LOG10	WRC MEAN=	3.879-LOG10	WRC SD=	0.271-LOG10
YRSPK=	19-YEARS	YRSHISPK=	28-YEARS		

1-18 02406000 TALLADEGA CREEK NEAR TALLADEGA, ALA.

A=	98.4-SQ MI	S=	22.200-FT/MI	LAT=	33.39-DEC. DEG
LONG=	86.11-DEC. DEG	P=	52.00-IN	I24-2=	4.200-IN
Q2=	3950-CFS	Q5=	7510-CFS	Q10=	10300-CFS
Q25=	14300-CFS	Q50=	17500-CFS	Q100=	20900-CFS
WRC SKEW=	-0.246-LOG10	WRC MEAN=	3.583-LOG10	WRC SD=	0.344-LOG10
YRSPK=	10-YEARS	YRSHISPK=	61-YEARS		

1-19 02406500 TALLADEGA CREEK AT ALPINE, ALA.

A=	148-SQ MI	S=	13.900-FT/MI	LAT=	33.36-DEC. DEG
LONG=	86.23-DEC. DEG	P=	52.00-IN	I24-2=	4.200-IN
Q2=	5640-CFS	Q5=	10700-CFS	Q10=	14900-CFS
Q25=	21000-CFS	Q50=	26100-CFS	Q100=	31700-CFS
WRC SKEW=	-0.129-LOG10	WRC MEAN=	3.744-LOG10	WRC SD=	0.339-LOG10
YRSPK=	55-YEARS	YRSHISPK=	79-YEARS		

1-20 02407500 YELLOWLEAF CREEK NEAR WILSONVILLE, ALA.

A=	97.2-SQ MI	S=	23.500-FT/MI	LAT=	33.31-DEC. DEG
LONG=	86.55-DEC. DEG	P=	54.00-IN	I24-2=	4.200-IN
Q2=	2840-CFS	Q5=	6180-CFS	Q10=	9720-CFS
Q25=	16300-CFS	Q50=	23400-CFS	Q100=	32700-CFS
WRC SKEW=	0.557-LOG10	WRC MEAN=	3.488-LOG10	WRC SD=	0.376-LOG10
YRSPK=	19-YEARS	YRSHISPK=	79-YEARS		

1-21 02407900 PAINT CREEK NEAR MARBLE VALLEY, ALA. SP

A= 13.5-SQ MI S= 33.000-FT/MI LAT= 33.04-DEC. DEG
LONG= 86.42-DEC. DEG P= 54.00-IN I24-2= 4.300-IN
Q2= 1140-CFS Q5= 1930-CFS Q10= 2540-CFS
Q25= 3400-CFS Q50= 4110-CFS Q100= 4830-CFS
WRC SKEW= 0.003-LOG10 WRC MEAN= 3.054-LOG10 WRC SD= 0.276-LOG10
YRSPK= 13-YEARS YRSHISPK= 50-YEARS

1-22 02408340 LITTLE HATCHET CREEK NEAR GOODWATER, ALA. SP

A= 9.44-SQ MI S= 32.900-FT/MI LAT= 33.12-DEC. DEG
LONG= 85.09-DEC. DEG P= 54.00-IN I24-2= 4.200-IN
Q2= 936-CFS Q5= 1570-CFS Q10= 2050-CFS
Q25= 2730-CFS Q50= 3290-CFS Q100= 3860-CFS
WRC SKEW= 0.018-LOG10 WRC MEAN= 2.969-LOG10 WRC SD= 0.269-LOG10
YRSPK= 6-YEARS YRSHISPK= 50-YEARS

1-23 02408500 HATCHET CREEK NEAR ROCKFORD, ALA.

A= 244-SQ MI S= 13.900-FT/MI LAT= 32.94-DEC. DEG
LONG= 86.22-DEC. DEG P= 53.00-IN I24-2= 4.300-IN
Q2= 9760-CFS Q5= 16600-CFS Q10= 22000-CFS
Q25= 30100-CFS Q50= 36900-CFS Q100= 44600-CFS
WRC SKEW= 0.162-LOG10 WRC MEAN= 3.996-LOG10 WRC SD= 0.267-LOG10
YRSPK= 51-YEARS YRSHISPK= NONE-YEARS

1-24 02409000 WEOGUFKA CREEK NEAR WEOGUFKA, ALA.

A= 73.4-SQ MI S= 12.400-FT/MI LAT= 32.98-DEC. DEG
LONG= 86.30-DEC. DEG P= 54.00-IN I24-2= 4.200-IN
Q2= 3120-CFS Q5= 6380-CFS Q10= 9530-CFS
Q25= 14900-CFS Q50= 20100-CFS Q100= 26600-CFS
WRC SKEW= 0.336-LOG10 WRC MEAN= 3.514-LOG10 WRC SD= 0.354-LOG10
YRSPK= 21-YEARS YRSHISPK= 79-YEARS

1-25 02410000 PATERSON CREEK NEAR CENTRAL, ALA. SP

A= 4.95-SQ MI S= 69.000-FT/MI LAT= 32.68-DEC. DEG
LONG= 86.13-DEC. DEG P= 52.00-IN I24-2= 4.450-IN
Q2= 712-CFS Q5= 1120-CFS Q10= 1420-CFS
Q25= 1830-CFS Q50= 2150-CFS Q100= 2480-CFS
WRC SKEW= 0.020-LOG10 WRC MEAN= 2.850-LOG10 WRC SD= 0.238-LOG10
YRSPK= 24-YEARS YRSHISPK= 50-YEARS

1-26 02422500 MULBERRY CREEK AT JONES, ALA.

A=	208-SQ MI	S=	13.700-FT/MI	LAT=	32.58-DEC. DEG
LONG=	86.90-DEC. DEG	P=	54.00-IN	I24-2=	4.400-IN
Q2=	6270-CFS	Q5=	10800-CFS	Q10=	14800-CFS
Q25=	21300-CFS	Q50=	27500-CFS	Q100=	34900-CFS
WRC SKEW=	0.600-LOG10	WRC MEAN=	3.823-LOG10	WRC SD=	0.261-LOG10
YRSPK=	53-YEARS	YRSHISPK=	113-YEARS		

1-27 02423500 CAHABA RIVER NEAR ACTON, ALA.

A=	230-SQ MI	S=	8.400-FT/MI	LAT=	33.37-DEC. DEG
LONG=	86.82-DEC. DEG	P=	54.00-IN	I24-2=	4.000-IN
Q2=	10100-CFS	Q5=	17100-CFS	Q10=	22500-CFS
Q25=	30100-CFS	Q50=	36300-CFS	Q100=	43100-CFS
WRC SKEW=	0.004-LOG10	WRC MEAN=	4.005-LOG10	WRC SD=	0.270-LOG10
YRSPK=	31-YEARS	YRSHISPK=	NONE-YEARS		

1-28 02423800 LITTLE CAHABA RIVER NEAR BRIERFIELD, ALA.

A=	148-SQ MI	S=	10.000-FT/MI	LAT=	33.06-DEC. DEG
LONG=	86.95-DEC. DEG	P=	54.00-IN	I24-2=	4.300-IN
Q2=	4190-CFS	Q5=	6560-CFS	Q10=	8350-CFS
Q25=	10900-CFS	Q50=	12900-CFS	Q100=	15200-CFS
WRC SKEW=	0.154-LOG10	WRC MEAN=	3.628-LOG10	WRC SD=	0.227-LOG10
YRSPK=	13-YEARS	YRSHISPK=	36-YEARS		

1-29 02424000 CAHABA RIVER AT CENTREVILLE, ALA.

A=	1029-SQ MI	S=	6.500-FT/MI	LAT=	32.94-DEC. DEG
LONG=	87.14-DEC. DEG	P=	54.00-IN	I24-2=	4.200-IN
Q2=	29300-CFS	Q5=	48200-CFS	Q10=	62600-CFS
Q25=	82900-CFS	Q50=	99400-CFS	Q100=	117000-CFS
WRC SKEW=	0.030-LOG10	WRC MEAN=	4.468-LOG10	WRC SD=	0.255-LOG10
YRSPK=	72-YEARS	YRSHISPK=	80-YEARS		

1-30 02424010 SANDY CREEK NEAR CENTERVILLE, ALA. SP

A=	0.630-SQ MI	S=	72.900-FT/MI	LAT=	32.88-DEC. DEG
LONG=	87.00-DEC. DEG	P=	54.00-IN	I24-2=	4.400-IN
Q2=	190-CFS	Q5=	277-CFS	Q10=	339-CFS
Q25=	420-CFS	Q50=	483-CFS	Q100=	551-CFS
WRC SKEW=	0.118-LOG10	WRC MEAN=	2.282-LOG10	WRC SD=	0.191-LOG10
YRSPK=	6-YEARS	YRSHISPK=	50-YEARS		

1-31 02433000 BULL MOUNTAIN CREEK NEAR SMITHVILLE, MISS.

A=	336-SQ MI	S=	1.900-FT/MI	LAT=	34.09-DEC. DEG
LONG=	88.39-DEC. DEG	P=	54.00-IN	I24-2=	4.000-IN
Q2=	10700-CFS	Q5=	19600-CFS	Q10=	26100-CFS
Q25=	36100-CFS	Q50=	44400-CFS	Q100=	53500-CFS
WRC SKEW=	-0.056-LOG10	WRC MEAN=	4.025-LOG10	WRC SD=	0.308-LOG10
YRSPK=	36-YEARS	YRSHISPK=	62-YEARS		

1-32 02437800 BARN CREEK NEAR HACKLEBURG, ALA. SP

A=	12.9-SQ MI	S=	35.200-FT/MI	LAT=	34.18-DEC. DEG
LONG=	87.79-DEC. DEG	P=	55.00-IN	I24-2=	3.800-IN
Q2=	1130-CFS	Q5=	1900-CFS	Q10=	2490-CFS
Q25=	3310-CFS	Q50=	4000-CFS	Q100=	4700-CFS
WRC SKEW=	0.009-LOG10	WRC MEAN=	3.052-LOG10	WRC SD=	0.268-LOG10
YRSPK=	15-YEARS	YRSHISPK=	50-YEARS		

1-33 02437900 WOODS CREEK NEAR HAMILTON, ALA. SP

A=	14.1-SQ MI	S=	31.500-FT/MI	LAT=	34.13-DEC. DEG
LONG=	87.90-DEC. DEG	P=	52.00-IN	I24-2=	3.900-IN
Q2=	1160-CFS	Q5=	1970-CFS	Q10=	2600-CFS
Q25=	3480-CFS	Q50=	4220-CFS	Q100=	4970-CFS
WRC SKEW=	0.005-LOG10	WRC MEAN=	3.063-LOG10	WRC SD=	0.275-LOG10
YRSPK=	12-YEARS	YRSHISPK=	50-YEARS		

1-34 02438000 BUTTAHATCHEE RIVER BELOW HAMILTON, ALA.

A=	284-SQ MI	S=	6.200-FT/MI	LAT=	34.11-DEC. DEG
LONG=	87.98-DEC. DEG	P=	54.00-IN	I24-2=	3.900-IN
Q2=	15900-CFS	Q5=	21900-CFS	Q10=	25900-CFS
Q25=	31000-CFS	Q50=	34900-CFS	Q100=	38800-CFS
WRC SKEW=	0.030-LOG10	WRC MEAN=	4.203-LOG10	WRC SD=	0.164-LOG10
YRSPK=	30-YEARS	YRSHISPK=	65-YEARS		

1-35 02439000 BUTTAHATCHEE RIVER NEAR SULLIGENT, ALA.

A=	472-SQ MI	S=	4.800-FT/MI	LAT=	33.92-DEC. DEG
LONG=	88.13-DEC. DEG	P=	54.00-IN	I24-2=	4.000-IN
Q2=	13600-CFS	Q5=	22200-CFS	Q10=	28000-CFS
Q25=	35500-CFS	Q50=	41000-CFS	Q100=	46400-CFS
WRC SKEW=	-0.371-LOG10	WRC MEAN=	4.117-LOG10	WRC SD=	0.268-LOG10
YRSPK=	53-YEARS	YRSHISPK=	65-YEARS		

1-36 02439500 BUTTAHATCHEE R NR CALEDONIA, MISS.

A=	823-SQ MI	S=	3.900-FT/MI	LAT=	33.70-DEC. DEG
LONG=	88.35-DEC. DEG	P=	54.00-IN	I24-2=	4.000-IN
Q2=	15500-CFS	Q5=	26800-CFS	Q10=	35600-CFS
Q25=	48300-CFS	Q50=	58900-CFS	Q100=	70300-CFS
WRC SKEW=	0.000-LOG10	WRC MEAN=	4.189-LOG10	WRC SD=	0.283-LOG10
YRSPK=	18-YEARS	YRSHISPK=	NONE-YEARS		

1-37 02440500 CHUQUATONCHEE CREEK NEAR WEST POINT, MISS.

A=	514-SQ MI	S=	1.800-FT/MI	LAT=	33.61-DEC. DEG
LONG=	88.71-DEC. DEG	P=	52.00-IN	I24-2=	4.100-IN
Q2=	17300-CFS	Q5=	27200-CFS	Q10=	34500-CFS
Q25=	44200-CFS	Q50=	52000-CFS	Q100=	60000-CFS
WRC SKEW=	-0.044-LOG10	WRC MEAN=	4.235-LOG10	WRC SD=	0.237-LOG10
YRSPK=	34-YEARS	YRSHISPK=	51-YEARS		

1-38 02445245 NEW RIVER NEAR WINFIELD, ALA.

A=	55.6-SQ MI	S=	7.600-FT/MI	LAT=	33.93-DEC. DEG
LONG=	87.68-DEC. DEG	P=	54.00-IN	I24-2=	4.000-IN
Q2=	3740-CFS	Q5=	5900-CFS	Q10=	7410-CFS
Q25=	9350-CFS	Q50=	10800-CFS	Q100=	12300-CFS
WRC SKEW=	-0.221-LOG10	WRC MEAN=	3.564-LOG10	WRC SD=	0.243-LOG10
YRSPK=	23-YEARS	YRSHISPK=	NONE-YEARS		

1-39 02450000 MULBERRY FORK NEAR GARDEN CITY, ALA.

A=	365-SQ MI	S=	6.200-FT/MI	LAT=	33.99-DEC. DEG
LONG=	86.75-DEC. DEG	P=	54.00-IN	I24-2=	3.900-IN
Q2=	24100-CFS	Q5=	33100-CFS	Q10=	38300-CFS
Q25=	44100-CFS	Q50=	47900-CFS	Q100=	51300-CFS
WRC SKEW=	-0.568-LOG10	WRC MEAN=	4.365-LOG10	WRC SD=	0.181-LOG10
YRSPK=	53-YEARS	YRSHISPK=	81-YEARS		

1-40 02450200 DORSEY CREEK NEAR ARKADELPHIA, ALA. SP

A=	13.0-SQ MI	S=	26.800-FT/MI	LAT=	33.95-DEC. DEG
LONG=	87.00-DEC. DEG	P=	54.00-IN	I24-2=	3.900-IN
Q2=	1090-CFS	Q5=	1850-CFS	Q10=	2440-CFS
Q25=	3270-CFS	Q50=	3970-CFS	Q100=	4670-CFS
WRC SKEW=	0.013-LOG10	WRC MEAN=	3.036-LOG10	WRC SD=	0.275-LOG10
YRSPK=	16-YEARS	YRSHISPK=	50-YEARS		

1-41 02450250 SIPSEY FORK NEAR GRAYSON, ALA.

A=	90.1-SQ MI	S=	13.300-FT/MI	LAT=	34.28-DEC. DEG
LONG=	87.40-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	7660-CFS	Q5=	11900-CFS	Q10=	14900-CFS
Q25=	18800-CFS	Q50=	21800-CFS	Q100=	24900-CFS
WRC SKEW=	-0.106-LOG10	WRC MEAN=	3.880-LOG10	WRC SD=	0.230-LOG10
YRSPK=	15-YEARS	YRSHISPK=	20-YEARS		

1-42 02451550 JAYBIRD CREEK NEAR WEST POINT, ALA. SP

A=	1.42-SQ MI	S=	52.400-FT/MI	LAT=	34.25-DEC. DEG
LONG=	87.00-DEC. DEG	P=	54.00-IN	I24-2=	3.800-IN
Q2=	297-CFS	Q5=	453-CFS	Q10=	567-CFS
Q25=	719-CFS	Q50=	841-CFS	Q100=	970-CFS
WRC SKEW=	0.091-LOG10	WRC MEAN=	2.473-LOG10	WRC SD=	0.214-LOG10
YRSPK=	8-YEARS	YRSHISPK=	50-YEARS		

1-43 02451750 VEST CREEK NEAR BALDWIN, ALA. SP

A=	1.64-SQ MI	S=	109.700-FT/MI	LAT=	34.20-DEC. DEG
LONG=	86.93-DEC. DEG	P=	54.00-IN	I24-2=	3.800-IN
Q2=	389-CFS	Q5=	577-CFS	Q10=	709-CFS
Q25=	880-CFS	Q50=	1010-CFS	Q100=	1150-CFS
WRC SKEW=	-0.008-LOG10	WRC MEAN=	2.590-LOG10	WRC SD=	0.203-LOG10
YRSPK=	9-YEARS	YRSHISPK=	50-YEARS		

1-44 02453900 CHEATHAM CREEK NEAR CARBON HILL, ALA. SP

A=	4.77-SQ MI	S=	49.000-FT/MI	LAT=	33.89-DEC. DEG
LONG=	87.45-DEC. DEG	P=	54.00-IN	I24-2=	4.000-IN
Q2=	640-CFS	Q5=	1020-CFS	Q10=	1310-CFS
Q25=	1700-CFS	Q50=	2010-CFS	Q100=	2340-CFS
WRC SKEW=	-0.013-LOG10	WRC MEAN=	2.806-LOG10	WRC SD=	0.243-LOG10
YRSPK=	7-YEARS	YRSHISPK=	50-YEARS		

1-45 02454000 LOST CREEK NEAR OAKMAN, ALA.

A=	134-SQ MI	S=	5.400-FT/MI	LAT=	33.76-DEC. DEG
LONG=	87.36-DEC. DEG	P=	58.00-IN	I24-2=	4.000-IN
Q2=	5020-CFS	Q5=	8050-CFS	Q10=	10700-CFS
Q25=	14800-CFS	Q50=	18500-CFS	Q100=	22900-CFS
WRC SKEW=	0.659-LOG10	WRC MEAN=	3.725-LOG10	WRC SD=	0.227-LOG10
YRSPK=	22-YEARS	YRSHISPK=	81-YEARS		

1-46 02454200 WOLF CREEK NEAR OAKMAN, ALA.

A=	85.0-SQ MI	S=	8.500-FT/MI	LAT=	33.67-DEC. DEG
LONG=	87.39-DEC. DEG	P=	58.00-IN	I24-2=	4.000-IN
Q2=	4710-CFS	Q5=	7830-CFS	Q10=	10300-CFS
Q25=	14000-CFS	Q50=	17100-CFS	Q100=	20600-CFS
WRC SKEW=	0.213-LOG10	WRC MEAN=	3.682-LOG10	WRC SD=	0.255-LOG10
YRSPK=	13-YEARS	YRSHISPK=	22-YEARS		

1-47 02454500 LOCUST FORK BELOW SNEAD, ALA.

A=	147-SQ MI	S=	4.600-FT/MI	LAT=	34.13-DEC. DEG
LONG=	86.39-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	5710-CFS	Q5=	8280-CFS	Q10=	10100-CFS
Q25=	12600-CFS	Q50=	14600-CFS	Q100=	16700-CFS
WRC SKEW=	0.175-LOG10	WRC MEAN=	3.762-LOG10	WRC SD=	0.187-LOG10
YRSPK=	17-YEARS	YRSHISPK=	20-YEARS		

1-48 02455000 LOCUST FORK NEAR CLEVELAND, ALA.

A=	303-SQ MI	S=	6.400-FT/MI	LAT=	34.03-DEC. DEG
LONG=	86.57-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	13300-CFS	Q5=	19800-CFS	Q10=	24600-CFS
Q25=	31400-CFS	Q50=	37000-CFS	Q100=	43000-CFS
WRC SKEW=	0.279-LOG10	WRC MEAN=	4.132-LOG10	WRC SD=	0.198-LOG10
YRSPK=	53-YEARS	YRSHISPK=	81-YEARS		

1-49 02455500 LOCUST FORK AT TRAFFORD, ALA.

A=	624-SQ MI	S=	7.700-FT/MI	LAT=	33.83-DEC. DEG
LONG=	86.76-DEC. DEG	P=	53.00-IN	I24-2=	4.100-IN
Q2=	24600-CFS	Q5=	36300-CFS	Q10=	44900-CFS
Q25=	56700-CFS	Q50=	66100-CFS	Q100=	76200-CFS
WRC SKEW=	0.222-LOG10	WRC MEAN=	4.399-LOG10	WRC SD=	0.194-LOG10
YRSPK=	53-YEARS	YRSHISPK=	NONE-YEARS		

1-50 02456000 TURKEY CREEK AT MORRIS, ALA.

A=	80.9-SQ MI	S=	24.700-FT/MI	LAT=	33.74-DEC. DEG
LONG=	86.81-DEC. DEG	P=	53.00-IN	I24-2=	4.100-IN
Q2=	5950-CFS	Q5=	9070-CFS	Q10=	11300-CFS
Q25=	14200-CFS	Q50=	16500-CFS	Q100=	18800-CFS
WRC SKEW=	-0.039-LOG10	WRC MEAN=	3.773-LOG10	WRC SD=	0.218-LOG10
YRSPK=	51-YEARS	YRSHISPK=	NONE-YEARS		

1-51 02456500 LOCUST FORK AT SAYRE, ALA.

A=	885-SQ MI	S=	5.700-FT/MI	LAT=	33.71-DEC. DEG
LONG=	86.98-DEC. DEG	P=	53.00-IN	I24-2=	4.200-IN
Q2=	24600-CFS	Q5=	34800-CFS	Q10=	42200-CFS
Q25=	52200-CFS	Q50=	60200-CFS	Q100=	68500-CFS
WRC SKEW=	0.255-LOG10	WRC MEAN=	4.398-LOG10	WRC SD=	0.174-LOG10
YRSPK=	52-YEARS	YRSHISPK=	109-YEARS		

1-52 02457700 FIVEMILE CREEK AT LINN CROSSING, ALA.

A=	96.2-SQ MI	S=	12.100-FT/MI	LAT=	33.67-DEC. DEG
LONG=	86.96-DEC. DEG	P=	53.00-IN	I24-2=	4.100-IN
Q2=	4850-CFS	Q5=	7090-CFS	Q10=	8510-CFS
Q25=	10200-CFS	Q50=	11400-CFS	Q100=	12600-CFS
WRC SKEW=	-0.363-LOG10	WRC MEAN=	3.673-LOG10	WRC SD=	0.208-LOG10
YRSPK=	11-YEARS	YRSHISPK=	32-YEARS		

1-53 02462000 VALLEY CREEK NEAR OAK GROVE, ALA.

A=	145-SQ MI	S=	6.900-FT/MI	LAT=	33.45-DEC. DEG
LONG=	87.12-DEC. DEG	P=	54.00-IN	I24-2=	4.200-IN
Q2=	9120-CFS	Q5=	13500-CFS	Q10=	17100-CFS
Q25=	22400-CFS	Q50=	27000-CFS	Q100=	32200-CFS
WRC SKEW=	0.638-LOG10	WRC MEAN=	3.980-LOG10	WRC SD=	0.190-LOG10
YRSPK=	29-YEARS	YRSHISPK=	81-YEARS		

1-54 02462600 BLUE CREEK NEAR OAKMAN, ALA. SP

A=	5.32-SQ MI	S=	65.000-FT/MI	LAT=	33.52-DEC. DEG
LONG=	87.48-DEC. DEG	P=	54.00-IN	I24-2=	4.200-IN
Q2=	768-CFS00-CFS	Q5=	1220-CFS	Q10=	1550-CFS
Q25=	2000-CFS	Q50=	2370-CFS	Q100=	2730-CFS
WRC SKEW=	-0.029-LOG10	WRC MEAN=	2.884-LOG10	WRC SD=	0.239-LOG10
YRSPK=	14-YEARS	YRSHISPK=	50-YEARS		

1-55 02462800 DAVIS CREEK BELOW ABERNANT, ALA.

A=	45.2-SQ MI	S=	14.800-FT/MI	LAT=	33.31-DEC. DEG
LONG=	87.22-DEC. DEG	P=	53.00-IN	I24-2=	4.200-IN
Q2=	2160-CFS	Q5=	2960-CFS	Q10=	3500-CFS
Q25=	4220-CFS	Q50=	4760-CFS	Q100=	5320-CFS
WRC SKEW=	0.142-LOG10	WRC MEAN=	3.338-LOG10	WRC SD=	0.159-LOG10
YRSPK=	17-YEARS	YRSHISPK=	63-YEARS		

1-56 02463500 HURRICANE CREEK NEAR HOLT, ALA.

A=	108-SQ MI	S=	10.600-FT/MI	LAT=	33.21-DEC. DEG
LONG=	87.45-DEC. DEG	P=	54.00-IN	I24-2=	4.200-IN
Q2=	5360-CFS	Q5=	8470-CFS	Q10=	10900-CFS
Q25=	14400-CFS	Q50=	17400-CFS	Q100=	20600-CFS
WRC SKEW=	0.262-LOG10	WRC MEAN=	3.739-LOG10	WRC SD=	0.228-LOG10
YRSPK=	17-YEARS	YRSHISPK=	NONE-YEARS		

1-57 02464000 NORTH RIVER NEAR SAMANTHA, ALA.

A=	219-SQ MI	S=	5.200-FT/MI	LAT=	33.48-DEC. DEG
LONG=	87.60-DEC. DEG	P=	53.00-IN	I24-2=	4.200-IN
Q2=	8330-CFS	Q5=	11800-CFS	Q10=	14500-CFS
Q25=	18200-CFS	Q50=	21400-CFS	Q100=	24800-CFS
WRC SKEW=	0.517-LOG10	WRC MEAN=	3.935-LOG10	WRC SD=	0.170-LOG10
YRSPK=	53-YEARS	YRSHISPK=	65-YEARS		

1-58 03567500 SOUTH CHICKAMAUGA CREEK NEAR CHICKAMAUGA, TENN.

A=	428-SQ MI	S=	5.580-FT/MI	LAT=	35.01-DEC. DEG
LONG=	85.21-DEC. DEG	P=	52.20-IN	I24-2=	4.000-IN
Q2=	12800-CFS	Q5=	18400-CFS	Q10=	22200-CFS
Q25=	26900-CFS	Q50=	30400-CFS	Q100=	33900-CFS
WRC SKEW=	-0.136-LOG10	WRC MEAN=	4.103-LOG10	WRC SD=	0.192-LOG10
YRSPK=	47-YEARS	YRSHISPK=	NONE-YEARS		

1-59 03568500 CHATTANOOGA CREEK NEAR FLINTSTONE, GA.

A=	50.6-SQ MI	S=	14.900-FT/MI	LAT=	34.97-DEC. DEG
LONG=	85.33-DEC. DEG	P=	52.50-IN	I24-2=	3.500-IN
Q2=	2920-CFS	Q5=	4350-CFS	Q10=	5350-CFS
Q25=	6670-CFS	Q50=	7700-CFS	Q100=	8760-CFS
WRC SKEW=	0.000-LOG10	WRC MEAN=	3.466-LOG10	WRC SD=	0.205-LOG10
YRSPK=	24-YEARS	YRSHISPK=	NONE-YEARS		

1-60 03571800 BATTLE CREEK NEAR MONTEAGLE, TENN.

A=	50.4-SQ MI	S=	136.000-FT/MI	LAT=	35.13-DEC. DEG
LONG=	85.77-DEC. DEG	P=	60.00-IN	I24-2=	3.500-IN
Q2=	3890-CFS	Q5=	5380-CFS	Q10=	6350-CFS
Q25=	7510-CFS	Q50=	8390-CFS	Q100=	9290-CFS
WRC SKEW=	0.000-LOG10	WRC MEAN=	3.558-LOG10	WRC SD=	0.169-LOG10
YRSPK=	21-YEARS	YRSHISPK=	76-YEARS		

1-61 03572900 TOWN CREEK NEAR GERALDINE, ALA.

A=	141-SQ MI	S= 10.900-FT/MI	LAT= 34.38-DEC. DEG
LONG=	85.99-DEC. DEG	P= 53.00-IN	I24-2= 3.800-IN
Q2=	8440-CFS	Q5= 11900-CFS	Q10= 14200-CFS
Q25=	16800-CFS	Q50= 18700-CFS	Q100= 20600-CFS
WRC SKEW=	-0.285-LOG10	WRC MEAN= 3.917-LOG10	WRC SD= 0.187-LOG10
YRSPK=	23-YEARS	YRSHISPK= 51-YEARS	

1-62 03573000 SHORT CREEK NEAR ALBERTVILLE, ALA.

A=	91.6-SQ MI	S= 8.100-FT/MI	LAT= 34.30-DEC. DEG
LONG=	86.18-DEC. DEG	P= 54.00-IN	I24-2= 3.800-IN
Q2=	6380-CFS	Q5= 10100-CFS	Q10= 13200-CFS
Q25=	17600-CFS	Q50= 21400-CFS	Q100= 25700-CFS
WRC SKEW=	0.350-LOG10	WRC MEAN= 3.818-LOG10	WRC SD= 0.229-LOG10
YRSPK=	21-YEARS	YRSHISPK= 27-YEARS	

1-63 03574405 LITTLE DRY CREEK NEAR GARTH, ALA. SP

A=	3.91-SQ MI	S=286.200-FT/MI	LAT= 34.74-DEC. DEG
LONG=	86.32-DEC. DEG	P= 54.00-IN	I24-2= 3.700-IN
Q2=	856-CFS	Q5= 1250-CFS	Q10= 1520-CFS
Q25=	1860-CFS	Q50= 2120-CFS	Q100= 2360-CFS
WRC SKEW=	-0.109-LOG10	WRC MEAN= 2.927-LOG10	WRC SD= 0.203-LOG10
YRSPK=	4-YEARS	YRSHISPK= 50-YEARS	

1-64 03574500 PAINT ROCK RIVER NEAR WOODVILLE, ALA.

A=	320-SQ MI	S= 14.800-FT/MI	LAT= 34.62-DEC. DEG
LONG=	86.31-DEC. DEG	P= 54.00-IN	I24-2= 3.700-IN
Q2=	17700-CFS	Q5= 26900-CFS	Q10= 33100-CFS
Q25=	40900-CFS	Q50= 46600-CFS	Q100= 52200-CFS
WRC SKEW=	-0.279-LOG10	WRC MEAN= 4.237-LOG10	WRC SD= 0.227-LOG10
YRSPK=	46-YEARS	YRSHISPK= 114-YEARS	

1-65 03574796 WALKER BRANCH NEAR PLEVNA, ALA.

A=	0.440-SQ MI	S= 65.400-FT/MI	LAT= 34.96-DEC. DEG
LONG=	86.45-DEC. DEG	P= 54.00-IN	I24-2= 3.700-IN
Q2=	57-CFS	Q5= 113-CFS	Q10= 158-CFS
Q25=	222-CFS	Q50= 275-CFS	Q100= 331-CFS
WRC SKEW=	-0.264-LOG10	WRC MEAN= 1.742-LOG10	WRC SD= 0.365-LOG10
YRSPK=	74-YEARS	YRSHISPK= NONE-YEARS	

1-66 03575000 FLINT RIVER NEAR CHASE, ALA.

A=	342-SQ MI	S=	8.000-FT/MI	LAT=	34.62-DEC. DEG
LONG=	86.31-DEC. DEG	P=	51.00-IN	I24-2=	3.800-IN
Q2=	15400-CFS	Q5=	28600-CFS	Q10=	38700-CFS
Q25=	52700-CFS	Q50=	63700-CFS	Q100=	75200-CFS
WRC SKEW=	-0.303-LOG10	WRC MEAN=	4.170-LOG10	WRC SD=	0.336-LOG10
YRSPK=	53-YEARS	YRSHISPK=	114-YEARS		

1-67 03575340 CLOVER COVE CREEK NR OWENS CROSS ROADS, ALA.

A=	3.52-SQ MI	S=	70.300-FT/MI	LAT=	34.60-DEC. DEG
LONG=	86.41-DEC. DEG	P=	54.00-IN	I24-2=	3.700-IN
Q2=	270-CFS	Q5=	429-CFS	Q10=	539-CFS
Q25=	680-CFS	Q50=	786-CFS	Q100=	893-CFS
WRC SKEW=	-0.271-LOG10	WRC MEAN=	2.419-LOG10	WRC SD=	0.250-LOG10
YRSPK=	73-YEARS	YRSHISPK=	NONE-YEARS		

1-68 03575830 INDIAN CREEK NEAR MADISON, ALA.

A=	49.0-SQ MI	S=	15.200-FT/MI	LAT=	34.70-DEC. DEG
LONG=	86.70-DEC. DEG	P=	49.00-IN	I24-2=	3.650-IN
Q2=	2850-CFS	Q5=	5050-CFS	Q10=	6740-CFS
Q25=	9140-CFS	Q50=	11100-CFS	Q100=	13200-CFS
WRC SKEW=	-0.120-LOG10	WRC MEAN=	3.450-LOG10	WRC SD=	0.299-LOG10
YRSPK=	22-YEARS	YRSHISPK=	114-YEARS		

1-69 03576148 COTACO CREEK AT FLORETTE, ALA.

A=	136-SQ MI	S=	3.100-FT/MI	LAT=	34.41-DEC. DEG
LONG=	86.69-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	5750-CFS	Q5=	9410-CFS	Q10=	12200-CFS
Q25=	16200-CFS	Q50=	19500-CFS	Q100=	23100-CFS
WRC SKEW=	0.088-LOG10	WRC MEAN=	3.763-LOG10	WRC SD=	0.251-LOG10
YRSPK=	15-YEARS	YRSHISPK=	17-YEARS		

1-70 03576250 LIMESTONE CREEK NEAR ATHENS, ALA.

A=	119-SQ MI	S=	10.600-FT/MI	LAT=	34.75-DEC. DEG
LONG=	86.82-DEC. DEG	P=	50.00-IN	I24-2=	3.800-IN
Q2=	7000-CFS	Q5=	11900-CFS	Q10=	15700-CFS
Q25=	21300-CFS	Q50=	25800-CFS	Q100=	30800-CFS
WRC SKEW=	0.039-LOG10	WRC MEAN=	3.847-LOG10	WRC SD=	0.272-LOG10
YRSPK=	42-YEARS	YRSHISPK=	114-YEARS		

1-71 03576400 PINEY CREEK NEAR ATHENS, ALA.

A=	55.8-SQ MI	S=	11.400-FT/MI	LAT=	34.80-DEC. DEG
LONG=	86.88-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	3430-CFS	Q5=	5160-CFS	Q10=	6530-CFS
Q25=	8520-CFS	Q50=	10200-CFS	Q100=	12100-CFS
WRC SKEW=	0.478-LOG10	WRC MEAN=	3.551-LOG10	WRC SD=	0.199-LOG10
YRSPK=	13-YEARS	YRSHISPK=	19-YEARS		

1-72 03576500 FLINT CREEK NEAR FALKVILLE, ALA.

A=	86.3-SQ MI	S=	19.200-FT/MI	LAT=	34.37-DEC. DEG
LONG=	86.93-DEC. DEG	P=	53.00-IN	I24-2=	3.800-IN
Q2=	6120-CFS	Q5=	8630-CFS	Q10=	10100-CFS
Q25=	11800-CFS	Q50=	13000-CFS	Q100=	14100-CFS
WRC SKEW=	-0.491-LOG10	WRC MEAN=	3.771-LOG1-	WRC SD=	0.192-LOG10
YRSPK=	21-YEARS	YRSHISPK=	113-YEARS		

1-73 03577000 WEST FLINT CREEK NEAR OAKVILLE, ALA.

A=	87.6-SQ MI	S=	2.800-FT/MI	LAT=	34.48-DEC. DEG
LONG=	87.14-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	3100-CFS	Q5=	4900-CFS	Q10=	6220-CFS
Q25=	8020-CFS	Q50=	9450-CFS	Q100=	10900-CFS
WRC SKEW=	-0.016-LOG10	WRC MEAN=	3.490-LOG10	WRC SD=	0.237-LOG10
YRSPK=	28-YEARS	YRSHISPK=	NONE-YEARS		

1-74 03577110 WEST FLINT CREEK NEAR HARTSELLE, ALA.

A=	158-SQ MI	S=	2.200-FT/MI	LAT=	34.49-DEC. DEG
LONG=	87.03-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	4270-CFS	Q5=	6710-CFS	Q10=	8390-CFS
Q25=	10600-CFS	Q50=	12200-CFS	Q100=	13800-CFS
WRC SKEW=	-0.243-LOG10	WRC MEAN=	3.621-LOG10	WRC SD=	0.242-LOG10
YRSPK=	18-YEARS	YRSHISPK=	91-YEARS		

1-75 03578500 BRADLEY CREEK NR PRAIRIE PLAINS, TENN.

A=	41.3-SQ MI	S=	14.200-FT/MI	LAT=	35.36-DEC. DEG
LONG=	85.98-DEC. DEG	P=	53.00-IN	I24-2=	3.500-IN
Q2=	2460-CFS	Q5=	3850-CFS	Q10=	0-4870-CFS
Q25=	6250-CFS	Q50=	7350-CFS	Q100=	8500-CFS
WRC SKEW=	0.000-LOG10	WRC MEAN=	3.396-LOG10	WRC SD=	0.229-LOG10
YRSPK=	23-YEARS	YRSHISPK=	NONE-YEARS		

1-76 03584000 RICHLAND CREEK NEAR PULASKI, TENN.

A=	366-SQ MI	S=	5.320-FT/MI	LAT=	35.21-DEC. DEG
LONG=	87.10-DEC. DEG	P=	53.00-IN	I24-2=	3.600-IN
Q2=	16500-CFS	Q5=	29100-CFS	Q10=	42000-CFS
Q25=	60000-CFS	Q50=	75000-CFS	Q100=	93000-CFS
WRC SKEW=	0.221-LOG10	WRC MEAN=	4.225-LOG10	WRC SD=	0.295-LOG10
YRSPK=	41-YEARS	YRSHISPK=	135-YEARS		

1-77 03585300 SUGAR CREEK NEAR GOOD SPRINGS, ALA.

A=	152-SQ MI	S=	11.500-FT/MI	LAT=	34.94-DEC. DEG
LONG=	87.16-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	9820-CFS	Q5=	16200-CFS	Q10=	21200-CFS
Q25=	28300-CFS	Q50=	34200-CFS	Q100=	40700-CFS
WRC SKEW=	0.100-LOG10	WRC MEAN=	3.996-LOG10	WRC SD=	0.255-LOG10
YRSPK=	12-YEARS	YRSHISPK=	14-YEARS		

1-78 03585380 WEST FORK ANDERSON CR NR LEXINGTON, ALA. SP

A=	5.92-SQ MI	S=	23.000-FT/MI	LAT=	34.97-DEC. DEG
LONG=	87.28-DEC. DEG	P=	52.00-IN	I24-2=	3.700-IN
Q2=	616-CFS	Q5=	1020-CFS	Q10=	1340-CFS
Q25=	1790-CFS	Q50=	2170-CFS	Q100=	2570-CFS
WRC SKEW=	0.114-LOG10	WRC MEAN=	2.793-LOG10	WRC SD=	0.259-LOG10
YRSPK=	7-YEARS	YRSHISPK=	50-YEARS		

1-79 03586500 BIG NANCE CREEK AT COURTLAND, ALA.

A=	166-SQ MI	S=	5.400-FT/MI	LAT=	34.67-DEC. DEG
LONG=	87.32-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	6140-CFS	Q5=	8980-CFS	Q10=	10900-CFS
Q25=	13200-CFS	Q50=	14900-CFS	Q100=	16600-CFS
WRC SKEW=	-0.223-LOG10	WRC MEAN=	3.781-LOG10	WRC SD=	0.203-LOG10
YRSPK=	41-YEARS	YRSHISPK=	114-YEARS		

1-80 03588500 SHOAL CREEK AT IRON CITY, TENN.

A=	348-SQ MI	S=	8.160-FT/MI	LAT=	35.02-DEC. DEG
LONG=	87.58-DEC. DEG	P=	54.00-IN	I24-2=	3.800-IN
Q2=	16800-CFS	Q5=	32000-CFS	Q10=	44000-CFS
Q25=	64000-CFS	Q50=	82000-CFS	Q100=	100000-CFS
WRC SKEW=	0.099-LOG10	WRC MEAN=	4.240-LOG10	WRC SD=	0.295-LOG10
YRSPK=	48-YEARS	YRSHISPK=	70-YEARS		

1-81 03590000 CYPRESS CREEK NEAR FLORENCE, ALA.

A=	209-SQ MI	S=	19.800-FT/MI	LAT=	34.81-DEC. DEG
LONG=	87.70-DEC. DEG	P=	52.00-IN	I24-2=	3.800-IN
Q2=	9990-CFS	Q5=	16700-CFS	Q10=	21200-CFS
Q25=	26900-CFS	Q50=	31100-CFS	Q100=	35200-CFS
WRC SKEW=	-0.441-LOG10	WRC MEAN=	3.979-LOG10	WRC SD=	0.284-LOG10
YRSPK=	19-YEARS	YRSHISPK=	112-YEARS		

1-82 03591570 BEAR CREEK AT POSEY HILL, ALA.

A=	26.8-SQ MI	S=	10.100-FT/MI	LAT=	34.33-DEC. DEG
LONG=	87.58-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	1400-CFS	Q5=	2090-CFS	Q10=	2530-CFS
Q25=	3060-CFS	Q50=	3430-CFS	Q100=	3780-CFS
WRC SKEW=	-0.419-LOG10	WRC MEAN=	3.131-LOG10	WRC SD=	0.222-LOG10
YRSPK=	12-YEARS	YRSHISPK=	112-YEARS		

1-83 03591800 BEAR CREEK NEAR HACKLEBURG, ALA.

A=	143-SQ MI	S=	6.100-FT/MI	LAT=	34.28-DEC. DEG
LONG=	87.77-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	7570-CFS	Q5=	11800-CFS	Q10=	14700-CFS
Q25=	18400-CFS	Q50=	21100-CFS	Q100=	23800-CFS
WRC SKEW=	-0.277-LOG10	WRC MEAN=	3.868-LOG10	WRC SD=	0.240-LOG10
YRSPK=	24-YEARS	YRSHISPK=	114-YEARS		

1-84 03592000 BEAR CREEK NEAR RED BAY, ALA.

A=	263-SQ MI	S=	4.000-FT/MI	LAT=	34.44-DEC. DEG
LONG=	88.12-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	4990-CFS	Q5=	8460-CFS	Q10=	11500-CFS
Q25=	16200-CFS	Q50=	20500-CFS	Q100=	25500-CFS
WRC SKEW=	0.482-LOG10	WRC MEAN=	3.719-LOG10	WRC SD=	0.257-LOG10
YRSPK=	30-YEARS	YRSHISPK=	114-YEARS		

1-85 03592200 CEDAR CREEK NEAR PLEASANT SITE, ALA.

A=	189-SQ MI	S=	4.800-FT/MI	LAT=	34.55-DEC. DEG
LONG=	88.02-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	8010-CFS	Q5=	11700-CFS	Q10=	14200-CFS
Q25=	17500-CFS	Q50=	20000-CFS	Q100=	22500-CFS
WRC SKEW=	-0.053-LOG10	WRC MEAN=	3.902-LOG10	WRC SD=	0.197-LOG10
YRSPK=	22-YEARS	YRSHISPK=	112-YEARS		

1-86 03592300 LITTLE BEAR CREEK NEAR HALLTOWN, ALA.

A=	78.2-SQ MI	S=	10.600-FT/MI	LAT=	34.49-DEC. DEG
LONG=	88.04-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	3620-CFS	Q5=	5500-CFS	Q10=	6830-CFS
Q25=	8580-CFS	Q50=	9940-CFS	Q100=	11300-CFS
WRC SKEW=	-0.047-LOG10	WRC MEAN=	3.557-LOG10	WRC SD=	0.217-LOG10
YRSPK=	27-YEARS	YRSHISPK=	112-YEARS		

1-87 03592500 BEAR CREEK AT BISHOP, ALA.

A=	667-SQ MI	S=	3.800-FT/MI	LAT=	34.66-DEC. DEG
LONG=	88.12-DEC. DEG	P=	53.00-IN	I24-2=	3.800-IN
Q2=	16000-CFS	Q5=	23700-CFS	Q10=	29100-CFS
Q25=	36100-CFS	Q50=	41500-CFS	Q100=	47100-CFS
WRC SKEW=	-0.028-LOG10	WRC MEAN=	4.203-LOG10	WRC SD=	0.204-LOG10
YRSPK=	52-YEARS	YRSHISPK=	112-YEARS		

HYDROLOGIC AREA 2

2-01 02420500 AUTAUGA CREEK AT PRATTVILLE, ALA.

A=	109-SQ MI	S=	16.900-FT/MI	ST=	10.50-%
LAT=	32.46-DEC. DEG	LONG=	86.47-DEC. DEG		
P=	53.00-IN	I24-2=	4.500-IN	Q2=	1880-CFS
Q5=	3520-CFS	Q10=	5130-CFS	Q25=	8000-CFS
Q50=	10900-CFS	Q100=	14700-CFS	WRC SKEW=	0.758-LOG10
WRC MEAN=	3.312-LOG10	WRC SD=	0.298-LOG10		
YRSPK=	43-YEARS	YRSHISPK=	94-YEARS		

2-02 02442000 LUXAPALLILA CREEK NEAR FAYETTE, ALA.

A=	127-SQ MI	S=	10.300-FT/MI	ST=	0.40-%
LAT=	33.72-DEC. DEG	LONG=	87.87-DEC. DEG		
P=	53.00-IN	I24-2=	4.000-IN	Q2=	5970-CFS
Q5=	8310-CFS	Q10=	9820-CFS	Q25=	11700-CFS
Q50=	13000-CFS	Q100=	14400-CFS	WRC SKEW=-0.158-LOG10	
WRC MEAN=	3.771-LOG10	WRC SD=	0.175-LOG10		
YRSPK=	42-YEARS	YRSHISPK=	79-YEARS		

2-03 02443000 LUXAPALLILA CREEK AT STEENS, MISS.

A=	309-SQ MI	S=	5.800-FT/MI	ST=	0.00-%
LAT=	33.56-DEC. DEG	LONG=	88.32-DEC. DEG		
P=	52.00-IN	I24-2=	4.200-IN	Q2=	6990-CFS
Q5=	9820-CFS	Q10=	11800-CFS	Q25=	14400-CFS
Q50=	16400-CFS	Q100=	18500-CFS	WRC SKEW=	0.134-LOG10
WRC MEAN=	3.848-LOG10	WRC SD=	0.172-LOG10		
YRSPK=	36-YEARS	YRSHISPK=	51-YEARS		

2-04 02444000 COAL FIRE CREEK NEAR PICKENSVILLE, ALA.

A=	131-SQ MI	S=	5.500-FT/MI	ST=	2.00-%
LAT=	33.30-DEC. DEG	LONG=	88.26-DEC. DEG		
P=	50.00-IN	I24-2=	4.100-IN	Q2=	2510-CFS
Q5=	4940-CFS	Q10=	7150-CFS	Q25=	10700-CFS
Q50=	14000-CFS	Q100=	17800-CFS	WRC SKEW=	0.185-LOG10
WRC MEAN=	3.410-LOG10	WRC SD=	0.342-LOG10		
YRSPK=	26-YEARS	YRSHISPK=	NONE-YEARS		

2-05 02445000 LUBBUB CREEK NEAR CARROLLTON, ALA.

A=	116-SQ MI	S=	5.400-FT/MI	ST=	1.00-%
LAT=	33.45-DEC. DEG	LONG=	88.08-DEC. DEG		
P=	51.00-IN	I24-2=	4.200-IN	Q2=	2460-CFS
Q5=	4600-CFS	Q10=	6630-CFS	Q25=	10100-CFS
Q50=	13400-CFS	Q100=	17500-CFS	WRC SKEW=	0.549-LOG10
WRC MEAN=	3.419-LOG10	WRC SD=	0.303-LOG10		
YRSPK=	14-YEARS	YRSHISPK=	25-YEARS		

2-06 02445500 SIPSEY RIVER AT FAYETTE, ALA.

A=	276-SQ MI	S=	3.600-FT/MI	ST=	5.00-%
LAT=	33.67-DEC. DEG	LONG=	87.82-DEC. DEG		DEG
P=	54.00-IN	I24-2=	4.000-IN	Q2=	7990-CFS
Q5=	1 3100-CFS	Q10=	16600-CFS	Q25=	21100-CFS
Q50=	24300-CFS	Q100=	27600-CFS	WRC SKEW=	-0.381-LOG10
WRC MEAN=	3.885-LOG10	WRC SD=	0.272-LOG10		
YRSPK=	32-YEARS	YRSHISPK=	79-YEARS		

2-07 02446000 SIPSEY RIVER AT MOORES BRIDGE, ALA.

A=	403-SQ MI	S=	3.700-FT/MI	ST=	4.00-%
LAT=	33.45-DEC. DEG	LONG=	87.76-DEC. DEG		
P=	54.00-IN	I24-2=	4.250-IN	Q2=	9750-CFS
Q5=	1 4700-CFS	Q10=	18300-CFS	Q25=	23100-CFS
Q50=	26900-CFS	Q100=	30900-CFS	WRC SKEW=	0.063-LOG10
WRC MEAN=	3.991-LOG10	WRC SD=	0.210-LOG10		
YRSPK=	23-YEARS	YRSHISPK=	58-YEARS		

2-08 02446500 SIPSEY RIVER NEAR ELROD, ALA.

A=	518-SQ MI	S=	2.700-FT/MI	ST=	4.00-%
LAT=	33.26-DEC. DEG	LONG=	87.76-DEC. DEG		
P=	52.00-IN	I24-2=	4.200-IN	Q2=	8920-CFS
Q5=	1 4000-CFS	Q10=	17600-CFS	Q25=	22300-CFS
Q50=	25900-CFS	Q100=	29600-CFS	WRC SKEW=	-0.157-LOG10
WRC MEAN=	3.944-LOG10	WRC SD=	0.238-LOG10		
YRSPK=	46-YEARS	YRSHISPK=	81-YEARS		

2-09 02447000 SIPSEY RIVER NEAR PLEASANT RIDGE, ALA.

A=	753-SQ MI	S=	2.300-FT/MI	ST=	8.50-%
LAT=	33.04-DEC. DEG	LONG=	88.36-DEC. DEG		
P=	52.00-IN	I24-2=	4.400-IN	Q2=	7800-CFS
Q5=	1 2600-CFS	Q10=	16300-CFS	Q25=	21400-CFS
Q50=	25600-CFS	Q100=	30100-CFS	WRC SKEW=	0.068-LOG10
WRC MEAN=	3.895-LOG10	WRC SD=	0.246-LOG10		
YRSPK=	31-YEARS	YRSHISPK=	63-YEARS		

2-10 02448000 NOXUBEE RIVER AT MACON, MISS.

A=	812-SQ MI	S=	2.500-FT/MI	ST=	2.00-%
LAT=	33.10-DEC. DEG	LONG=	88.56-DEC. DEG		
P=	52.00-IN	I24-2=	4.300-IN	Q2=	12400-CFS
Q5=	2 1600-CFS	Q10=	28800-CFS	Q25=	39000-CFS
Q50=	47500-CFS	Q100=	56700-CFS	WRC SKEW=	-0.030-LOG10
WRC MEAN=	4.092-LOG10	WRC SD=	0.287-LOG10		
YRSPK=	42-YEARS	YRSHISPK=	85-YEARS		

2-11 02448500 NOXUBEE RIVER NEAR GEIGER, ALA.

A=	1140-SQ MI	S=	2.700-FT/MI	ST=	3.00-%
LAT=	32.92-DEC. DEG	LONG=	88.30-DEC. DEG		
P=	50.00-IN	I24-2=	4.400-IN	Q2=	12100-CFS
Q5=	2 1000-CFS	Q10=	28800-CFS	Q25=	41200-CFS
Q50=	52600-CFS	Q100=	66000-CFS	WRC SKEW=	0.446-LOG10
WRC MEAN=	4.102-LOG10	WRC SD=	0.271-LOG10		
YRSPK=	53-YEARS	YRSHISPK=	89-YEARS		

2-12 02465205 JAY CREEK NEAR COKER, ALA.

A=	3.56-SQ MI	S=	47.500-FT/MI	ST=	8.40-%
LAT=	33.22-DEC. DEG	LONG=	87.70-DEC. DEG		
P=	54.00-IN	I24-2=	4.200-IN	Q2=	527-CFS
Q5=	832-CFS	Q10=	1060-CFS	Q25=	1370-CFS
Q50=	1620-CFS	Q100=	1880-CFS	WRC SKEW=	0.019-LOG10
WRC MEAN=	2.723-LOG10	WRC SD=	0.236-LOG10		
YRSPK=	10-YEARS	YRSHISPK=	50-YEARS		

2-13 02465500 FIVEMILE CREEK NEAR GREENSBORO, ALA.

A=	72.2-SQ MI	S=	9.500-FT/MI	ST=	3.25-%
LAT=	32.83-DEC. DEG	LONG=	87.61-DEC. DEG		
P=	53.00-IN	I24-2=	4.400-IN	Q2=	1780-CFS
Q5=	3080-CFS	Q10=	4110-CFS	Q25=	5570-CFS
Q50=	6780-CFS	Q100=	8090-CFS	WRC SKEW=-0.006-LOG10	
WRC MEAN=	3.251-LOG10	WRC SD=	0.283-LOG10		
YRSPK=	19-YEARS	YRSHISPK=	43-YEARS		

2-14 02467500 SUCARNOOCHEE RIVER AT LIVINGSTON, ALA.

A=	606-SQ MI	S=	6.500-FT/MI	ST=	2.00-%
LAT=	32.57-DEC. DEG	LONG=	88.19-DEC. DEG		
P=	52.00-IN	I24-2=	4.500-IN	Q2=	7490-CFS
Q5=	1 3600-CFS	Q10=	19200-CFS	Q25=	28400-CFS
Q50=	37000-CFS	Q100=	47300-CFS	WRC SKEW=	0.447-LOG10
WRC MEAN=	3.896-LOG10	WRC SD=	0.294-LOG10		
YRSPK=	52-YEARS	YRSHISPK=	NONE-YEARS		

HYDROLOGIC AREA 3

3-01 02337500 SNAKE CREEK NEAR WHITESBURG, GA.

A=	37.0-SQ MI	S=	20.100-FT/MI	LAT=	33.53-DEC. DEG
LONG=	84.93-DEC. DEG	P=	50.00-IN	I24-2=	3.450-IN
Q2=	3400-CFS	Q5=	4520-CFS	Q10=	5310-CFS
Q25=	6380-CFS	Q50=	7220-CFS	Q100=	8100-CFS
WRC SKEW=	0.410-LOG10	WRC MEAN=	3.541-LOG10	WRC SD=	0.140-LOG10
YRSPK=	20-YEARS	YRSHISPK=	NONE-YEARS		

3-02 02339000 YELLOWJACKET CREEK NEAR LA GRANGE, GA.

A=	182-SQ MI	S=	7.540-FT/MI	LAT=	33.09-DEC. DEG
LONG=	85.06-DEC. DEG	P=	50.00-IN	I24-2=	3.400-IN
Q2=	3600-CFS	Q5=	6420-CFS	Q10=	8690-CFS
Q25=	1 2000-CFS	Q50=	14700-CFS	Q100=	17700-CFS
WRC SKEW=	-0.024-LOG10	WRC MEAN=	3.555-LOG10	WRC SD=	0.301-LOG10
YRSPK=	21-YEARS	YRSHISPK=	61-YEARS		

3-03 02340500 MOUNTAIN OAK CREEK NEAR HAMILTON, GA.

A=	61.7-SQ MI	S=	10.600-FT/MI	LAT=	32.74-DEC. DEG
LONG=	85.07-DEC. DEG	P=	50.00-IN	I24-2=	3.700-IN
Q2=	1720-CFS	Q5=	3270-CFS	Q10=	4650-CFS
Q25=	6860-CFS	Q50=	8880-CFS	Q100=	11300-CFS
WRC SKEW==	0.224-LOG10	WRC MEAN=	3.246-LOG10	WRC SD=	0.323-LOG10
YRSPK=	30-YEARS	YRSHISPK=	61-YEARS		

3-04 02340750 OSANIPPA CREEK NEAR FAIRFAX, ALA.

A=	101-SQ MI	S=	10.000-FT/MI	LAT=	32.79-DEC. DEG
LONG=	85.19-DEC. DEG	P=	52.00-IN	I24-2=	4.050-IN
Q2=	3860-CFS	Q5=	5690-CFS	Q10=	6930-CFS
Q25=	8540-CFS	Q50=	9750-CFS	Q100=	11000-CFS
WRC SKEW=	-0.092-LOG10	WRC MEAN=	3.584-LOG10	WRC SD=	0.202-LOG10
YRSPK=	22-YEARS	YRSHISPK=	31-YEARS		

3-05 02405800 TALLADEGA CREEK ABOVE TALLADEGA, ALA.

A=	67.3-SQ MI	S=	16.200-FT/MI	LAT=	33.38-DEC. DEG
LONG=	86.02-DEC. DEG	P=	52.00-IN	I24-2=	4.200-IN
Q2=	2980-CFS	Q5=	4680-CFS	Q10=	5830-CFS
Q25=	7300-CFS	Q50=	8390-CFS	Q100=	9470-CFS
WRC SKEW=	-0.281-LOG10	WRC MEAN=	3.463-LOG10	WRC SD=	0.242-LOG10
YRSPK=	13-YEARS	YRSHISPK=	19-YEARS		

3-06 02411900 TALLAPOOSA RIVER AT TALLAPOOSA, GA.

A=	236-SQ MI	S=	5.050-FT/MI	LAT=	33.77-DEC. DEG
LONG=	85.30-DEC. DEG	P=	51.00-IN	I24-2=	3.300-IN
Q2=	5930-CFS	Q5=	8850-CFS	Q10=	10900-CFS
Q25=	1 3700-CFS	Q50=	15900-CFS	Q100=	18200-CFS
WRC SKEW=	0.067-LOG10	WRC MEAN=	3.776-LOG10	WRC SD=	0.204-LOG10
YRSPK=	28-YEARS	YRSHISPK=	94-YEARS		

3-07 02412000 TALLAPOOSA RIVER NEAR HEFLIN, ALA.

A=	444-SQ MI	S=	5.800-FT/MI	LAT=	33.62-DEC. DEG
LONG=	85.52-DEC. DEG	P=	52.00-IN	I24-2=	3.900-IN
Q2=	8200-CFS	Q5=	12400-CFS	Q10=	15700-CFS
Q25=	2 0700-CFS	Q50=	24900-CFS	Q100=	29600-CFS
WRC SKEW=	0.490-LOG10	WRC MEAN=	3.930-LOG10	WRC SD=	0.202-LOG10
YRSPK=	29-YEARS	YRSHISPK=	61-YEARS		

3-08 02412320 ELDER CREEK NEAR DEMPSEY, ALA. SP

A=	1.79-SQ MI	S=	99.700-FT/MI	LAT=	33.46-DEC. DEG
LONG=	85.78-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	402-CFS	Q5=	601-CFS	Q10=	742-CFS
Q25=	925-CFS	Q50=	1070-CFS	Q100=	1220-CFS
WRC SKEW=	0.027-LOG10	WRC MEAN=	2.605-LOG10	WRC SD=	0.205-LOG10
YRSPK=	3-YEARS	YRSHISPK=	50-YEARS		

3-09 02412500 TALLAPOOSA RIVER NEAR OFELIA, ALA.

A=	787-SQ MI	S=	4.100-FT/MI	LAT=	33.33-DEC. DEG
LONG=	85.58-DEC. DEG	P=	52.00-IN	I24-2=	3.900-IN
Q2=	15000-CFS	Q5=	23000-CFS	Q10=	28200-CFS
Q25=	3 4700-CFS	Q50=	39300-CFS	Q100=	43900-CFS
WRC SKEW=	-.0357-LOG10	WRC MEAN=	4.162-LOG10	WRC SD=	0.233-LOG10
YRSPK=	51-YEARS	YRSHISPK=	94-YEARS		

3-10 02413000 LITTLE TALLAPOOSA RIVER AT CARROLLTON, GA.

A=	95.1-SQ MI	S=	5.380-FT/MI	LAT=	33.60-DEC. DEG
LONG=	85.08-DEC. DEG	P=	50.00-IN	I24-2=	3.250-IN
Q2=	2650-CFS	Q5=	3840-CFS	Q10=	4640-CFS
Q25=	5640-CFS	Q50=	6390-CFS	Q100=	7130-CFS
WRC SKEW=	-0.152-LOG10	WRC MEAN=	3.418-LOG10	WRC SD=	0.196-LOG10
YRSPK=	29-YEARS	YRSHISPK=	39-YEARS		

3-11 02413200 LITTLE TALLAPOOSA RIVER NEAR BOWDEN, GA.

A=	220-SQ MI	S=	2.870-FT/MI	LAT=	33.51-DEC. DEG
LONG=	85.23-DEC. DEG	P=	50.50-IN	I24-2=	3.200-IN
Q2=	3960-CFS	Q5=	5750-CFS	Q10=	6850-CFS
Q25=	8130-CFS	Q50=	9020-CFS	Q100=	9850-CFS
WRC SKEW=	-0.469-LOG10	WRC MEAN=	3.581-LOG10	WRC SD=	0.208-LOG10
YRSPK=	29-YEARS	YRSHISPK=	44-YEARS		

3-12 02413400 WEDOWEE CREEK ABOVE WEDOWEE, ALA. SP

A=	6.50-SQ MI	S=	41.800-FT/MI	LAT=	33.32-DEC. DEG
LONG=	85.35-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	753-CFS	Q5=	1230-CFS	Q10=	1580-CFS
Q25=	2080-CFS	Q50=	2480-CFS	Q100=	2890-CFS
WRC SKEW=	-0.039-LOG10	WRC MEAN=	2.875-LOG10	WRC SD=	0.255-LOG10
YRSPK=	13-YEARS	YRSHISPK=	50-YEARS		

3-13 02413475 WEDOWEE CREEK NEAR WEDOWEE, ALA.

A=	51.1-SQ MI	S=	20.900-FT/MI	LAT=	33.32-DEC. DEG
LONG=	85.48-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	2800-CFS	Q5=	3900-CFS	Q10=	4550-CFS
Q25=	5280-CFS	Q50=	5770-CFS	Q100=	6220-CFS
WRC SKEW=	-0.534-LOG10	WRC MEAN=	3.430-LOG10	WRC SD=	0.189-LOG10
YRSPK=	25-YEARS	YRSHISPK=	30-YEARS		

3-14 02413500 LITTLE TALLAPOOSA RIVER NEAR WEDOWEE, ALA.

A=	592-SQ MI	S=	5.100-FT/MI	LAT=	33.35-DEC. DEG
LONG=	85.55-DEC. DEG	P=	50.00-IN	I24-2=	3.900-IN
Q2=	13800-CFS	Q5=	19700-CFS	Q10=	23400-CFS
Q25=	27700-CFS	Q50=	30800-CFS	Q100=	33700-CFS
WRC SKEW=	-0.358-LOG10	WRC MEAN=	4.130-LOG10	WRC SD=	0.193-LOG10
YRSPK=	53-YEARS	YRSHISPK=	63-YEARS		

3-15 02414800 HARBUCK CREEK NEAR HACKNEYVILLE, ALA. SP

A=	6.70-SQ MI	S=	67.200-FT/MI	LAT=	33.12-DEC. DEG
LONG=	85.95-DEC. DEG	P=	54.00-IN	I24-2=	4.100-IN
Q2=	858-CFS	Q5=	1370-CFS	Q10=	1750-CFS
Q25=	2260-CFS	Q50=	2670-CFS	Q100=	3080-CFS
WRC SKEW=	-0.071-LOG10	WRC MEAN=	2.931-LOG10	WRC SD=	0.245-LOG10
YRSPK=	20-YEARS	YRSHISPK=	50-YEARS		

3-16 02415000 HILLABEE CREEK NEAR HACKNEYVILLE, ALA.

A=	196-SQ MI	S=	22.200-FT/MI	LAT=	33.07-DEC. DEG
LONG=	85.88-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	7390-CFS	Q5=	11500-CFS	Q10=	14600-CFS
Q25=	1 9000-CFS	Q50=	22600-CFS	Q100=	26500-CFS
WRC SKEW=	0.176-LOG10	WRC MEAN=	3.875-LOG10	WRC SD=	0.223-LOG10
YRSPK=	21-YEARS	YRSHISPK=	28-YEARS		

3-17 02417400 STEARNS CREEK NEAR SEMAN, ALA.

SP

A=	1.28-SQ MI	S=	74.800-FT/MI	LAT=	32.72-DEC. DEG
LONG=	86.09-DEC. DEG	P=	52.00-IN	I24-2=	4.300-IN
Q2=	303-CFS	Q5=	452-CFS	Q10=	559-CFS
Q25=	700-CFS	Q50=	811-CFS	Q100=	928-CFS
WRC SKEW=	0.063-LOG10	WRC MEAN=	2.484-LOG10	WRC SD=	0.204-LOG10
YRSPK=	8-YEARS	YRSHISPK=	50-YEARS		

HYDROLOGIC AREA 4

4-01 02371000 CONECAH RIVER NEAR TROY, ALA.

A=	253-SQ-MI	S=	5.400-FT/MI	LAT=	31.84-DEC. DEG
LONG=	85.99-DEC. DEG	P=	53.00-IN	I24-2=	4.250-IN
Q2=	6100-CFS	Q5=	11400-CFS	Q10=	15400-CFS
Q25=	2 0800-CFS	Q50=	25100-CFS	Q100=	29500-CFS
WRC SKEW=	-0.344-LOG10	WRC MEAN=	3.766-LOG10	WRC SD=	0.340-LOG10
YRSPK=	40-YEARS	YRSHISPK=	116-YEARS		

4-02 02371200 INDIAN CREEK NEAR TROY, ALA.

A=	8.88-SQ MI	S=	27.700-FT/MI	LAT=	31.810-DEC. DEG
LONG=	86.12-DEC. DEG	P=	54.00-IN	I24-2=	4.700-IN
Q2=	836-CFS	Q5=	1400-CFS	Q10=	1840-CFS
Q25=	2460-CFS	Q50=	2970-CFS	Q100=	3510-CFS
WRC SKEW=	0.054-LOG10	WRC MEAN=	2.925-LOG10	WRC SD=	0.260-LOG10
YRSPK=	17-YEARS	YRSHISPK=	50-YEARS		

4-03 04419000 UPHAPEE CREEK NEAR TUSKEGEE, ALA.

A=	330-SQ MI	S=	13.100-FT/MI	LAT=	32.48-DEC. DEG
LONG=	85.69-DEC. DEG	P=	51.00-IN	I24-2=	4.300-IN
Q2=	10500-CFS	Q5=	18600-CFS	Q10=	25000-CFS
Q25=	34200-CFS	Q50=	41800-CFS	Q100=	50200-CFS
WRC SKEW=	-0.034-LOG10	WRC MEAN=	4.019-LOG10	WRC SD=	0.296-LOG10
YRSPK=	52-YEARS	YRSHISPK=	80-YEARS		

4-04 02419625 CALEBEE CREEK NEAR TUSKEGEE, ALA.

A=	126-SQ MI	S=	5.900-FT/MI	LAT=	32.38-DEC. DEG
LONG=	85.83-DEC. DEG	P=	51.00-IN	I24-2=	4.500-IN
Q2=	3650-CFS	Q5=	9600-CFS	Q10=	15500-CFS
Q25=	25100-CFS	Q50=	34000-CFS	Q100=	44300-CFS
WRC SKEW=	-0.268-LOG10	WRC MEAN=	3.539-LOG10	WRC SD=	0.520-LOG10
YRSPK=	19-YEARS	YRSHISPK=	26-YEARS		

4-05 02421000 CATOMA CREEK NEAR MONTGOMERY, ALA.

A=	298-SQ MI	S=	8.400-FT/MI	LAT=	32.31-DEC. DEG
LONG=	86.30-DEC. DEG	P=	52.00-IN	I24-2=	4.500-IN
Q2=	11300-CFS	Q5=	20000-CFS	Q10=	27600-CFS
Q25=	39400-CFS	Q50=	50000-CFS	Q100=	62200-CFS
WRC SKEW=	0.291-LOG10	WRC MEAN=	4.065-LOG10	WRC SD=	0.287-LOG10
YRSPK=	30-YEARS	YRSHISPK=	NONE-YEARS		

4-06 02421300 IVY CREEK AT MULBERRY, ALA. SP

A=	10.5-SQ MI	S=	27.400-FT/MI	LAT=	32.46-DEC. DEG
LONG=	86.78-DEC. DEG	P=	52.00-IN	I24-2=	4.500-IN
Q2=	930-CFS	Q5=	1570-CFS	Q10=	2070-CFS
Q25=	2770-CFS	Q50=	3360-CFS	Q100=	3960-CFS
WRC SKEW=	0.029-LOG10	WRC MEAN=	2.970-LOG10	WRC SD=	0.265-LOG10
YRSPK=	11-YEARS	YRSHISPK=	50-YEARS		

4-07 02422000 BIG SWAMP CREEK NEAR LOWNDESBORO, ALA.

A=	247-SQ MI	S=	8.400-FT/MI	LAT=	32.27-DEC. DEG
LONG=	86.69-DEC. DEG	P=	52.00-IN	I24-2=	4.600-IN
Q2=	10100-CFS	Q5=	17400-CFS	Q10=	22700-CFS
Q25=	3 0000-CFS	Q50=	35700-CFS	Q100=	41600-CFS
WRC SKEW=	-0.210-LOG10	WRC MEAN=	3.995-LOG10	WRC SD=	0.287-LOG10
YRSPK=	44-YEARS	YRSHISPK=	53-YEARS		

4-08 02425500 CEDAR CREEK AT MINTER, ALA.

A=	217-SQ MI	S=	7.400-FT/MI	LAT=	32.08-DEC. DEG
LONG=	86.98-DEC. DEG	P=	56.00-IN	I24-2=	4.700-IN
Q2=	7590-CFS	Q5=	12500-CFS	Q10=	17200-CFS
Q25=	2 5100-CFS	Q50=	33000-CFS	Q100=	42900-CFS
WRC SKEW==	1.015-LOG10	WRC MEAN=	3.919-LOG10	WRC SD=	0.235-LOG10
YRSPK=	29-YEARS	YRSHISPK=	79-YEARS		

4-09 02425655 MUSH CREEK NEAR SELMA, ALA.

A=	45.4-SQ MI	S=	12.200-FT/MI	LAT=	32.24-DEC. DEG
LONG=	86.99-DEC. DEG	P=	52.00-IN	I24-2=	4.700-IN
Q2=	5230-CFS	Q5=	9390-CFS	Q10=	12800-CFS
Q25=	1 7800-CFS	Q50=	22000-CFS	Q100=	26600-CFS
WRC SKEW=	0.018-LOG10	WRC MEAN=	3.719-LOG10	WRC SD=	0.302-LOG10
YRSPK=	21-YEARS	YRSHISPK=	55-YEARS		

4-10 02426000 BOGUECHITTO CREEK NEAR BROWNS, ALA.

A=	104-SQ MI	S=	9.300-FT/MI	LAT=	32.44-DEC. DEG
LONG=	87.33-DEC. DEG	P=	52.00-IN	I24-2=	4.600-IN
Q2=	4820-CFS	Q5=	8060-CFS	Q10=	10600-CFS
Q25=	1 4300-CFS	Q50=	17500-CFS	Q100=	20900-CFS
WRC SKEW=	0.135-LOG10	WRC MEAN=	3.689-LOG10	WRC SD=	0.260-LOG10
YRSPK=	52-YEARS	YRSHISPK=	NONE-YEARS		

4-11 02427013 CAINE CREEK NEAR SAFFORD, ALA. SP

A=	2.67-SQ MI	S=	83.300-FT/MI	LAT=	32.30-DEC. DEG
LONG=	87.34-DEC. DEG	P=	54.00-IN	I24-2=	4.600-IN
Q2=	499-CFS	Q5=	763-CFS	Q10=	952-CFS
Q25=	1200-CFS	Q50=	1400-CFS	Q100=	1600-CFS
WRC SKEW=	0.008-LOG10	WRC MEAN=	2.698-LOG10	WRC SD=	0.215-LOG10
YRSPK=	4-YEARS	YRSHISPK=	50-YEARS		

4-12 02427300 PRAIRIE CREEK NEAR OAK HILL, ALA. SP

A=	9.73-SQ MI	S=	30.000-FT/MI	LAT=	31.93-DEC. DEG
LONG=	87.10-DEC. DEG	P=	54.00-IN	I24-2=	4.300-IN
Q2=	901-CFS	Q5=	1510-CFS	Q10=	1980-CFS
Q25=	2650-CFS	Q50=	3200-CFS	Q100=	3770-CFS
WRC SKEW=	0.052-LOG10	WRC MEAN=	2.957-LOG10	WRC SD=	0.260-LOG10
YRSPK=	15-YEARS	YRSHISPK=	50-YEARS		

4-13 02427700 TURKEY CREEK AT KIMBROUGH, ALA.

A=	114-SQ MI	S=	14.200-FT/MI	LAT=	32.02-DEC. DEG
LONG=	87.56-DEC. DEG	P=	54.00-IN	I24-2=	4.800-IN
Q2=	5160-CFS	Q5=	9170-CFS	Q10=	12700-CFS
Q25=	1 8200-CFS	Q50=	23200-CFS	Q100=	29200-CFS
WRC SKEW=	0.370-LOG10	WRC MEAN=	3.730-LOG10	WRC SD=	0.283-LOG10
YRSPK=	24-YEARS	YRSHISPK=	52-YEARS		

4-14 02427875 PURSLEY CREEK NEAR CAMDEN, ALA.

A=	60.2-SQ MI	S=	12.600-FT/MI	LAT=	31.95-DEC. DEG
LONG=	87.33-DEC. DEG	P=	55.00-IN	I24-2=	4.800-IN
Q2=	3420-CFS	Q5=	5240-CFS	Q10=	6720-CFS
Q25=	8930-CFS	Q50=	10900-CFS	Q100=	13000-CFS
WRC SKEW=	0.541-LOG10	WRC MEAN=	3.553-LOG10	WRC SD=	0.207-LOG10
YRSPK=	19-YEARS	YRSHISPK=	21-YEARS		

4-15 02428300 TALLATCHEE CREEK NEAR VREDENBURGH, ALA. SP

A=	14.6-SQ MI	S=	24.500-FT/MI	LAT=	31.80-DEC. DEG
LONG=	87.30-DEC. DEG	P=	56.00-IN	I24-2=	4.800-IN
Q2=	1120-CFS	Q5=	1920-CFS	Q10=	2550-CFS
Q25=	3450-CFS	Q50=	4210-CFS	Q100=	4990-CFS
WRC SKEW=	0.046-LOG10	WRC MEAN=	3.051-LOG10	WRC SD=	0.272-LOG10
YRSPK=	15-YEARS	YRSHISPK=	50-YEARS		

4-16 02449400 JONES CREEK NEAR EPES, ALA. SP

A=	11.7-SQ MI	S=	19.600-FT/MI	LAT=	32.69-DEC. DEG
LONG=	88.17-DEC. DEG	P=	49.00-IN	I24-2=	4.400-IN
Q2=	920-CFS	Q5=	1580-CFS	Q10=	2110-CFS
Q25=	2860-CFS	Q50=	3500-CFS	Q100=	4160-CFS
WRC SKEW=	0.051-LOG10	WRC MEAN=	2.966-LOG10	WRC SD=	0.274-LOG10
YRSPK=	15-YEARS	YRSHISPK=	50-YEARS		

4-17 02466500 PRAIRIE CREEK NEAR GALLION, ALA.

A=	169-SQ MI	S=	10.300-FT/MI	LAT=	32.54-DEC. DEG
LONG=	87.68-DEC. DEG	P=	52.00-IN	I24-2=	4.500-IN
Q2=	15700-CFS	Q5=	25300-CFS	Q10=	32100-CFS
Q25=	4 1000-CFS	Q50=	47700-CFS	Q100=	54600-CFS
WRC SKEW=	-0.234-LOG10	WRC MEAN=	4.186-LOG10	WRC SD=	0.256-LOG10
YRSPK=	24-YEARS	YRSHISPK=	61-YEARS		

4-18 02468500 CHICKASAW BOGUE NEAR LINDEN, ALA.

A=	258-SQ MI	S=	3.300-FT/MI	LAT=	32.33-DEC. DEG
LONG=	87.79-DEC. DEG	P=	52.00-IN	I24-2=	4.650-IN
Q2=	13600-CFS	Q5=	20500-CFS	Q10=	25800-CFS
Q25=	3 3500-CFS	Q50=	39900-CFS	Q100=	47000-CFS
WRC SKEW=	0.414-LOG10	WRC MEAN=	4.148-LOG10	WRC SD=	0.199-LOG10
YRSPK=	19-YEARS	YRSHISPK=	79-YEARS		

4-19 02469550 HORSE CREEK NEAR SWEETWATER, ALA.

A=	60.4-SQ MI	S=	26.300-FT/MI	LAT=	32.05-DEC. DEG
LONG=	87.87-DEC. DEG	P=	55.00-IN	I24-2=	4.700-IN
Q2=	5210-CFS	Q5=	11000-CFS	Q10=	16100-CFS
Q25=	2 3900-CFS	Q50=	30600-CFS	Q100=	38200-CFS
WRC SKEW=	-0.149-LOG10	WRC MEAN=	3.707-LOG10	WRC SD=	0.395-LOG10
YRSPK=	22-YEARS	YRSHISPK=	114-YEARS		

HYDROLOGIC AREA 5

5-01 02342150 UCHEE CREEK NEAR SEALE, ALA.

A=	134-SQ MI	S=	7.100-FT/MI	LAT=	32.35-DEC. DEG
LONG=	85.10-DEC. DEG	P=	50.00-IN	I24-2=	4.200-IN
Q2=	4480-CFS	Q5=	7810-CFS	Q10=	10200-CFS
Q25=	1 3300-CFS	Q50=	15700-CFS	Q100=	18000-CFS
WRC SKEW=	-0.382-LOG10	WRC MEAN=	3.631-LOG10	WRC SD=	0.306-LOG10
YRSPK=	34-YEARS	YRSHISPK=	51-YEARS		

5-02 02342200 PHELPS CREEK NEAR OPALIKA, ALA.

A=	7.47-SQ MI	S=	42.300-FT/MI	LAT=	32.57-DEC. DEG
LONG=	85.27-DEC. DEG	P=	52.00-IN	I24-2=	4.200-IN
Q2=	826-CFS	Q5=	1350-CFS	Q10=	1750-CFS
Q25=	2300-CFS	Q50=	2750-CFS	Q100=	3210-CFS
WRC SKEW=	-0.025-LOG10	WRC MEAN=	2.916-LOG10	WRC SD=	0.256-LOG10
YRSPK=	16-YEARS	YRSHISPK=	50-YEARS		

5-03 02342500 UCHEE CREEK NEAR FORT MITCHELL, ALA.

A=	325-SQ MI	S=	9.100-FT/MI	LAT=	32.32-DEC. DEG
LONG=	85.01-DEC. DEG	P=	49.00-IN	I24-2=	4.200-IN
Q2=	8960-CFS	Q5=	15000-CFS	Q10=	19600-CFS
Q25=	2 5900-CFS	Q50=	30900-CFS	Q100=	36300-CFS
WRC SKEW=	-0.081-LOG10	WRC MEAN=	3.949-LOG10	WRC SD=	0.269-LOG10
YRSPK=	35-YEARS	YRSHISPK=	52-YEARS		

5-04 02342933 SOUTH FORK COWIKEE CR NEAR BATESVILLE, ALA.

A=	114-SQ MI	S=	13.900-FT/MI	LAT=	32.02-DEC. DEG
LONG=	85.30-DEC. DEG	P=	52.00-IN	I24-2=	4.400-IN
Q2=	5870-CFS	Q5=	8140-CFS	Q10=	9850-CFS
Q25=	1 2200-CFS	Q50=	14200-CFS	Q100=	16300-CFS
WRC SKEW=	0.526-LOG10	WRC MEAN=	3.782-LOG10	WRC SD=	0.159-LOG10
YRSPK=	18-YEARS	YRSHISPK=	52-YEARS		

5-05 02343200 PATAULA CREEK NEAR LUMPKIN, GA.

A=	70-SQ MI	S=	22.200-FT/MI	LAT=	31.93-DEC. DEG
LONG=	84.80-DEC. DEG	P=	49.50-IN	I24-2=	3.850-IN
Q2=	1360-CFS	Q5=	2820-CFS	Q10=	4180-CFS
Q25=	6450-CFS	Q50=	8570-CFS	Q100=	11100-CFS
WRC SKEW=	0.174-LOG10	WRC MEAN=	3.144-LOG10	WRC SD=	0.368-LOG10
YRSPK=	30-YEARS	YRSHISPK=	51-YEARS		

5-06 02343275 ABBIE CREEK NEAR ABBEVILLE, ALA.

A=	46.7-SQ MI	S=	12.000-FT/MI	LAT=	31.56-DEC. DEG
LONG=	85.20-DEC. DEG	P=	53.00-IN	I24-2=	4.600-IN
Q2=	1830-CFS	Q5=	3610-CFS	Q10=	5140-CFS
Q25=	7490-CFS	Q50=	9560-CFS	Q100=	11900-CFS
WRC SKEW=	-0.010-LOG10	WRC MEAN=	3.261-LOG10	WRC SD=	0.351-LOG10
YRSPK=	30-YEARS	YRSHISPK=	51-YEARS		

5-07 02343300 ABBIE CREEK NEAR HALEBURG, ALA.

A=	144-SQ MI	S=	8.200-FT/MI	LAT=	31.47-DEC. DEG
LONG=	85.16-DEC. DEG	P=	53.00-IN	I24-2=	4.700-IN
Q2=	3290-CFS	Q5=	5040-CFS	Q10=	6230-CFS
Q25=	7760-CFS	Q50=	8900-CFS	Q100=	10000-CFS
WRC SKEW=	-0.216-LOG10	WRC MEAN=	3.509-LOG10	WRC SD=	0.227-LOG10
YRSPK=	24-YEARS	YRSHISPK=	28-YEARS		

5-08 02343700 STEVENSON CREEK NEAR HEADLAND, ALA. SP

A=	12.4-SQ MI	S=	33.300-FT/MI	LAT=	31.35-DEC. DEG
LONG=	85.22-DEC. DEG	P=	52.00-IN	I24-2=	4.700-IN
Q2=	1080-CFS	Q5=	1830-CFS	Q10=	2400-CFS
Q25=	3200-CFS	Q50=	3870-CFS	Q100=	4550-CFS
WRC SKEW=	-0.037-LOG10	WRC MEAN=	3.032-LOG10	WRC SD=	0.272-LOG10
YRSPK=	15-YEARS	YRSHISPK=	50-YEARS		

5-09 02360000 W FORK CHOCTAWHATCHEE R AT BLUE SPRINGS, ALA.

A=	84.7-SQ MI	S=	9.800-FT/MI	LAT=	31.66-DEC. DEG
LONG=	85.50-DEC. DEG	P=	53.00-IN	I24-2=	4.600-IN
Q2=	1880-CFS	Q5=	3360-CFS	Q10=	4570-CFS
Q25=	6360-CFS	Q50=	7900-CFS	Q100=	9620-CFS
WRC SKEW=	0.084-LOG10	WRC MEAN=	3.279-LOG10	WRC SD=	0.295-LOG10
YRSPK=	42-YEARS	YRSHISPK=	43-YEARS		

5-10 02360275 JUDY CREEK NEAR OZARK, ALA.

A=	102-SQ MI	S=	11.700-FT/MI	LAT=	31.46-DEC. DEG
LONG=	85.57-DEC. DEG	P=	53.50-IN	I24-2=	4.700-IN
Q2=	3030-CFS	Q5=	5710-CFS	Q10=	8070-CFS
Q25=	1 1800-CFS	Q50=	15200-CFS	Q100=	19200-CFS
WRC SKEW=	0.215-LOG10	WRC MEAN=	3.492-LOG10	WRC SD=	0.319-LOG10
YRSPK=	27-YEARS	YRSHISPK=	41-YEARS		

5-11 02360500 E FK CHOCTAWHATCHEE R NEAR MIDLAND CITY, ALA.

A=	297-SQ MI	S=	4.700-FT/MI	LAT=	31.37-DEC. DEG
LONG=	85.48-DEC. DEG	P=	53.00-IN	I24-2=	4.600-IN
Q2=	4370-CFS	Q5=	7500-CFS	Q10=	10000-CFS
Q25=	1 3800-CFS	Q50=	17100-CFS	Q100=	20700-CFS
WRC SKEW=	0.173-LOG10	WRC MEAN=	3.649-LOG10	WRC SD=	0.272-LOG10
YRSPK=	16-YEARS	YRSHISPK=	NONE-YEARS		

5-12 02361000 CHOCTAWHATCHEE RIVER NEAR NEWTON, ALA.

A=	683-SQ MI	S=	4.600-FT/MI	LAT=	31.34-DEC. DEG
LONG=	85.61-DEC. DEG	P=	53.00-IN	I24-2=	4.800-IN
Q2=	9730-CFS	Q5=	15900-CFS	Q10=	20800-CFS
Q25=	2 8000-CFS	Q50=	34100-CFS	Q100=	40900-CFS
WRC SKEW=	0.253-LOG10	WRC MEAN=	3.999-LOG10	WRC SD=	0.244-LOG10
YRSPK=	53-YEARS	YRSHISPK=	81-YEARS		

5-13 02362745 HURRICANE CREEK NEAR CLAYTON, ALA. SP

A=	4.40-SQ MI	S=	34.200-FT/MI	LAT=	31.91-DEC. DEG
LONG=	85.58-DEC. DEG	P=	56.00-IN	I24-2=	4.600-IN
Q2=	558-CFS	Q5=	904-CFS	Q10=	1170-CFS
Q25=	1530-CFS	Q50=	1830-CFS	Q100=	2140-CFS
WRC SKEW=	0.031-LOG10	WRC MEAN=	2.748-LOG10	WRC SD=	0.248-LOG10
YRSPK=	5-YEARS	YRSHISPK=	50-YEARS		

5-14 02363000 PEA RIVER NEAR ARITON, ALA.

A=	492-SQ MI	S=	6.000-FT/MI	LAT=	31.59-DEC. DEG
LONG=	85.78-DEC. DEG	P=	53.00-IN	I24-2=	4.350-IN
Q2=	7650-CFS	Q5=	14000-CFS	Q10=	19300-CFS
Q25=	2 7100-CFS	Q50=	33700-CFS	Q100=	41000-CFS
WRC SKEW=	0.011-LOG10	WRC MEAN=	3.884-LOG10	WRC SD=	0.312-LOG10
YRSPK=	52-YEARS	YRSHISPK=	81-YEARS		

5-15 02363055 MOORES BRANCH NEAR VICTORIA, ALA. SP

A=	2.17-SQ MI	S=	59.200-FT/MI	LAT=	31.46-DEC. DEG
LONG=	85.90-DEC. DEG	P=	56.00-IN	I24-2=	4.850-IN
Q2=	403-CFS	Q5=	620-CFS	Q10=	778-CFS
Q25=	991-CFS	Q50=	1160-CFS	Q100=	1340-CFS
WRC SKEW=	0.052-LOG10	WRC MEAN=	2.607-LOG10	WRC SD=	0.220-LOG10
YRSPK=	4-YEARS	YRSHISPK=	50-YEARS		

5-16 02364000 PEA RIVER AT ELBA, ALA.

A=	966-SQ MI	S=	5.000-FT/MI	LAT=	31.40-DEC. DEG
LONG=	86.07-DEC. DEG	P=	55.00-IN	I24-2=	4.800-IN
Q2=	13100-CFS	Q5=	22400-CFS	Q10=	29800-CFS
Q25=	4 0600-CFS	Q50=	49600-CFS	Q100=	59600-CFS
WRC SKEW=	0.093-LOG10	WRC MEAN=	4.122-LOG10	WRC SD=	0.273-LOG10
YRSPK=	36-YEARS	YRSHISPK=	81-YEARS		

5-17 02364500 PEA RIVER NEAR SAMSON, ALA.

A=	1187-SQ MI	S=	4.100-FT/MI	LAT=	31.11-DEC. DEG
LONG=	86.10-DEC. DEG	P=	54.00-IN	I24-2=	4.550-IN
Q2=	11800-CFS	Q5=	19100-CFS	Q10=	25200-CFS
Q25=	3 4400-CFS	Q50=	42500-CFS	Q100=	51600-CFS
WRC SKEW=	0.396-LOG10	WRC MEAN=	4.086-LOG10	WRC SD=	0.240-LOG10
YRSPK=	59-YEARS	YRSHISPK=	80-YEARS		

5-18 02365310 GRANTS BRANCH TRIB NEAR FADETTE, ALA. SP

A=	1.50-SQ MI	S=	34.800-FT/MI	LAT=	31.04-DEC. DEG
LONG=	85.59-DEC. DEG	P=	56.00-IN	I24-2=	4.800-IN
Q2=	280-CFS	Q5=	435-CFS	Q10=	550-CFS
Q25=	709-CFS	Q50=	837-CFS	Q100=	975-CFS
WRC SKEW=	0.112-LOG10	WRC MEAN=	2.451-LOG10	WRC SD=	0.223-LOG10
YRSPK=	4-YEARS	YRSHISPK=	50-YEARS		

5-19 02367000 ALAQUA CREEK NEAR DE FUNIAK SPRINGS, FLA.

A=	65.6-SQ MI	S=	11.100-FT/MI	LAT=	30.62-DEC. DEG
LONG=	86.16-DEC. DEG	P=	66.70-IN	I24-2=	5.310-IN
Q2=	2390-CFS	Q5=	4290-CFS	Q10=	5910-CFS
Q25=	8380-CFS	Q50=	10600-CFS	Q100=	13100-CFS
WRC SKEW=	0.203-LOG10	WRC MEAN=	3.389-LOG10	WRC SD=	0.294-LOG10
YRSPK=	26-YEARS	YRSHISPK=	NONE-YEARS		

5-20 02367500 LIGHTWOOD KNOT CREEK AT BABBLE, ALA.

A=	113-SQ MI	S=	9.300-FT/MI	LAT=	31.27-DEC. DEG
LONG=	86.32-DEC. DEG	P=	56.00-IN	I24-2=	4.400-IN
Q2=	3290-CFS	Q5=	6160-CFS	Q10=	8700-CFS
Q25=	1 2700-CFS	Q50=	16400-CFS	Q100=	20800-CFS
WRC SKEW=	0.255-LOG10	WRC MEAN=	3.530-LOG10	WRC SD=	0.313-LOG10
YRSPK=	43-YEARS	YRSHISPK=	49-YEARS		

5-21 02367800 YELLOW RIVER NEAR WING, ALA.

A=	447-SQ MI	S=	5.500-FT/MI	LAT=	31.02-DEC. DEG
LONG=	86.53-DEC. DEG	P=	58.00-IN	I24-2=	5.200-IN
Q2=	6300-CFS	Q5=	9940-CFS	Q10=	13100-CFS
Q25=	18000-CFS	Q50=	22500-CFS	Q100=	27700-CFS
WRC SKEW=	0.699-LOG10	WRC MEAN=	3.824-LOG10	WRC SD=	0.219-LOG10
YRSPK=	11-YEARS	YRSHISPK=	46-YEARS		

5-22 02368000 YELLOW R AT MILLIGAN, FLA.

A=	624-SQ MI	S=	3.310-FT/MI	LAT=	30.75-DEC. DEG
LONG=	86.63-DEC. DEG	P=	58.60-IN	I24-2=	4.510-IN
Q2=	8340-CFS	Q5=	14900-CFS	Q10=	20500-CFS
Q25=	29100-CFS	Q50=	36700-CFS	Q100=	45400-CFS
WRC SKEW=	0.207-LOG10	WRC MEAN=	3.931-LOG10	WRC SD=	0.293-LOG10
YRSPK=	40-YEARS	YRSHISPK=	NONE-YEARS		

5-23 02369000 SHOAL RIVER NEAR CRESTVIEW, FLA.

A=	474-SQ MI	S=	4.040-FT/MI	LAT=	30.70-DEC. DEG
LONG=	86.57-DEC. DEG	P=	65.50-IN	I24-2=	4.900-IN
Q2=	7560-CFS	Q5=	12500-CFS	Q10=	16500-CFS
Q25=	22300-CFS	Q50=	27200-CFS	Q100=	32700-CFS
WRC SKEW=	0.188-LOG10	WRC MEAN=	3.887-LOG10	WRC SD=	0.255-LOG10
YRSPK=	40-YEARS	YRSHISPK=	NONE-YEARS		

5-24 02369800 BLACKWATER RIVER NEAR BRADLEY, ALA.

A=	86.8-SQ MI	S=	8.900-FT/MI	LAT=	31.03-DEC. DEG
LONG=	86.71-DEC. DEG	P=	60.00-IN	I24-2=	5.500-IN
Q2=	2230-CFS	Q5=	4630-CFS	Q10=	6800-CFS
Q25=	10200-CFS	Q50=	13400-CFS	Q100=	17000-CFS
WRC SKEW=	0.021-LOG10	WRC MEAN=	3.349-LOG10	WRC SD=	0.376-LOG10
YRSPK=	14-YEARS	YRSHISPK=	52-YEARS		

5-25 02370000 BLACKWATER R NR BAKER, FLA.

A=	205-SQ MI	S=	3.590-FT/MI	LAT=	30.83-DEC. DEG
LONG=	86.73-DEC. DEG	P=	61.20-IN	I24-2=	4.630-IN
Q2=	4310-CFS	Q5=	8820-CFS	Q10=	13000-CFS
Q25=	20000-CFS	Q50=	26600-CFS	Q100=	34500-CFS
WRC SKEW=	0.203-LOG10	WRC MEAN=	3.646-LOG10	WRC SD=	0.360-LOG10
YRSPK=	28-YEARS	YRSHISPK=	NONE-YEARS		

5-26 02371500 CONECUH RIVER AT BRANTLEY, ALA.

A=	492-SQ MI	S=	4.800-FT/MI	LAT=	31.57-DEC. DEG
LONG=	86.25-DEC. DEG	P=	54.00-IN	I24-2=	4.800-IN
Q2=	7300-CFS	Q5=	12400-CFS	Q10=	16000-CFS
Q25=	20600-CFS	Q50=	24000-CFS	Q100=	27300-CFS
WRC SKEW=	-0.395-LOG10	WRC MEAN=	3.844-LOG10	WRC SD=	0.292-LOG10
YRSPK=	53-YEARS	YRSHISPK=	116-YEARS		

5-27 02372000 PATSALIGA CREEK AT LUVERNE, ALA.

A=	249-SQ MI	S=	7.100-FT/MI	LAT=	31.73-DEC. DEG
LONG=	86.28-DEC. DEG	P=	54.00-IN	I24-2=	4.300-IN
Q2=	6240-CFS	Q5=	10800-CFS	Q10=	14100-CFS
Q25=	18600-CFS	Q50=	22100-CFS	Q100=	25700-CFS
WRC SKEW=	-0.248-LOG10	WRC MEAN=	3.783-LOG10	WRC SD=	0.293-LOG10
YRSPK=	42-YEARS	YRSHISPK=	116-YEARS		

5-28 02372500 CONECUH RIVER NEAR ANDALUSIA, ALA.

A=	1344-SQ MI	S=	4.100-FT/MI	LAT=	31.26-DEC. DEG
LONG=	86.60-DEC. DEG	P=	57.00-IN	I24-2=	5.200-IN
Q2=	14500-CFS	Q5=	24900-CFS	Q10=	33600-CFS
Q25=	46600-CFS	Q50=	57800-CFS	Q100=	70600-CFS
WRC SKEW=	0.220-LOG10	WRC MEAN=	4.171-LOG10	WRC SD=	0.273-LOG10
YRSPK=	62-YEARS	YRSHOSPK=	115-YEARS		

5-29 02372510 CATOE CREEK NEAR ANDALUSIA, ALA.

A=	2.46-SQ MI	S=	31.500-FT/MI	LAT=	31.16-DEC. DEG
LONG=	86.60-DEC. DEG	P=	56.00-IN	I24-2=	5.200-IN
Q2=	376-CFS	Q5=	598-CFS	Q10=	765-CFS
Q25=	997-CFS	Q50=	1190-CFS	Q100=	1390-CFS
WRC SKEW=	0.095-LOG10	WRC MEAN=	2.579-LOG10	WRC SD=	0.235-LOG10
YRSPK=	5-YEARS	YRSHISPK=	50-YEARS		

5-30 02373000 SEPULGA RIVER NEAR MCKENZIE, ALA.

A=	464-SQ MI	S=	6.800-FT/MI	LAT=	31.45-DEC. DEG
LONG=	86.78-DEC. DEG	P=	57.00-IN	I24-2=	4.700-IN
Q2=	9540-CFS	Q5=	16300-CFS	Q10=	21500-CFS
Q25=	28800-CFS	Q50=	34700-CFS	Q100=	41000-CFS
WRC SKEW=	-0.073-LOG10	WRC MEAN=	3.976-LOG10	WRC SD=	0.280-LOG10
YRSPK=	48-YEARS	YRSHISPK=	116-YEARS		

5-31 02373500 PIGEON CREEK NEAR THAD, ALA.

A=	296-SQ MI	S=	6.600-FT/MI	LAT=	31.48-DEC. DEG
LONG=	86.65-DEC. DEG	P=	57.00-IN	I24-2=	4.800-IN
Q2=	5470-CFS	Q5=	10400-CFS	Q10=	14400-CFS
Q25=	20500-CFS	Q50=	25700-CFS	Q100=	31400-CFS
WRC SKEW=	-0.045-LOG10	WRC MEAN=	3.736-LOG10	WRC SD=	0.332-LOG10
YRSPK=	41-YEARS	YRSHISPK=	105-YEARS		

5-32 02374500 MURDER CREEK NEAR EVERGREEN, ALA.

A=	170-SQ MI	S=	9.000-FT/MI	LAT=	31.42-DEC. DEG
LONG=	86.99-DEC. DEG	P=	58.00-IN	I24-2=	4.600-IN
Q2=	3600-CFS	Q5=	7330-CFS	Q10=	10900-CFS
Q25=	17200-CFS	Q50=	23300-CFS	Q100=	30900-CFS
WRC SKEW=	0.381-LOG10	WRC MEAN=	3.578-LOG10	WRC SD=	0.350-LOG10
YRSPK=	52-YEARS	YRSHISPK=	116-YEARS		

5-33 02428500 FLAT CREEK NEAR FOUNTAIN, ALA.

A=	245-SQ MI	S=	6.100-FT/MI	LAT=	31.62-DEC. DEG
LONG=	87.42-DEC. DEG	P=	57.00-IN	I24-2=	4.800-IN
Q2=	4670-CFS	Q5=	8930-CFS	Q10=	12900-CFS
Q25=	19600-CFS	Q50=	26000-CFS	Q100=	33700-CFS
WRC SKEW=	0.422-LOG10	WRC MEAN=	3.692-LOG10	WRC SD=	0.318-LOG10
YRSPK=	27-YEARS	YRSHISPK=	41-YEARS		

5-34 02429000 LIMESTONE CREEK NEAR MONROEVILLE, ALA.

A=	117-SQ MI	S=	16.400-FT/MI	LAT=	31.56-DEC. DEG
LONG=	87.35-DEC. DEG	P=	58.00-IN	I24-2=	4.800-IN
Q2=	3890-CFS	Q5=	7420-CFS	Q10=	10600-CFS
Q25=	15900-CFS	Q50=	20800-CFS	Q100=	26700-CFS
WRC SKEW=	0.321-LOG10	WRC MEAN=	3.607-LOG10	WRC SD=	0.320-LOG10
YRSPK=	22-YEARS	YRSHISPK=	44-YEARS		

5-35 02429595 LITTLE RIVER NEAR URIAH, ALA.

A=	99.2-SQ MI	S=	14.300-FT/MI	LAT=	31.01-DEC. DEG
LONG=	87.61-DEC. DEG	P=	61.00-IN	I24-2=	5.200-IN
Q2=	2800-CFS	Q5=	4940-CFS	Q10=	6790-CFS
Q25=	9720-CFS	Q50=	12400-CFS	Q100=	15500-CFS
WRC SKEW=	0.372-LOG10	WRC MEAN=	3.465-LOG10	WRC SD=	0.279-LOG10
YRSPK=	11-YEARS	YRSHISPK=	24-YEARS		

5-36 02468000 ALAMUCHEE CREEK NEAR CUBA, ALA.

A=	63.0-SQ MI	S=	16.100-FT/MI	LAT=	32.43-DEC. DEG
LONG=	88.33-DEC. DEG	P=	53.00-IN	I24-2=	4.400-IN
Q2=	1220-CFS	Q5=	2630-CFS	Q10=	4140-CFS
Q25=	7020-CFS	Q50=	10100-CFS	Q100=	14300-CFS
WRC SKEW=	0.655-LOG10	WRC MEAN=	3.128-LOG10	WRC SD=	0.368-LOG10
YRSPK=	16-YEARS	YRSHISPK=	77-YEARS		

5-37 02469000 KINTERBISH CREEK NEAR YORK, ALA.

A=	91.4-SQ MI	S=	11.400-FT/MI	LAT=	32.32-DEC. DEG
LONG=	88.18-DEC. DEG	P=	53.00-IN	I24-2=	4.600-IN
Q2=	1550-CFS	Q5=	2730-CFS	Q10=	3790-CFS
Q25=	5510-CFS	Q50=	7120-CFS	Q100=	9030-CFS
WRC SKEW=	0.496-LOG10	WRC MEAN=	3.213-LOG10	WRC SD=	0.277-LOG10
YRSPK=	16-YEARS	YRSHISPK=	77-YEARS		

5-38 02469500 TUCKABUM CREEK NEAR BUTLER, ALA.

A=	112-SQ MI	S=	9.100-FT/MI	LAT=	32.18-DEC. DEG
LONG=	88.17-DEC. DEG	P=	54.00-IN	I24-2=	4.600-IN
Q2=	3550-CFS	Q5=	6980-CFS	Q10=	10000-CFS
Q25=	14900-CFS	Q50=	19400-CFS	Q100=	24500-CFS
WRC SKEW=	0.139-LOG10	WRC MEAN=	3.558-LOG10	WRC SD=	0.342-LOG10
YRSPK=	25-YEARS	YRSHISPK=	77-YEARS		

5-39 02469700 OKATUPPA CREEK AT GILBERTOWN, ALA.

A=	151-SQ MI	S=	11.400-FT/MI	LAT=	31.89-DEC. DEG
LONG=	88.31-DEC. DEG	P=	56.00-IN	I24-2=	4.700-IN
Q2=	3410-CFS	Q5=	4840-CFS	Q10=	5880-CFS
Q25=	7300-CFS	Q50=	8430-CFS	Q100=	9630-CFS
WRC SKEW=	0.296-LOG10	WRC MEAN=	3.542-LOG10	WRC SD=	0.174-LOG10
YRSPK=	14-YEARS	YRSHISPK=	68-YEARS		

5-40 02469736 LITTLE SOUWILPA CREEK AT BOLINGER, ALA. SP

A=	7.25-SQ MI	S=	25.800-FT/MI	LAT=	31.77-DEC. DEG
LONG=	88.33-DEC. DEG	P=	58.00-IN	I24-2=	4.800-IN
Q2=	721-CFS	Q5=	1200-CFS	Q10=	1580-CFS
Q25=	2100-CFS	Q50=	2550-CFS	Q100=	3010-CFS
WRC SKEW=	0.063-LOG10	WRC MEAN=	2.861-LOG10	WRC SD=	0.260-LOG10
YRSPK=	6-YEARS	YRSHISPK=	50-YEARS		

5-41 02469800 SATILPA CREEK NEAR COFFEEVILLE, ALA.

A=	166-SQ MI	S=	8.700-FT/MI	LAT=	31.74-DEC. DEG
LONG=	88.02-DEC. DEG	P=	57.00-IN	I24-2=	4.800-IN
Q2=	5440-CFS	Q5=	9540-CFS	Q10=	12900-CFS
Q25=	18100-CFS	Q50=	22500-CFS	Q100=	27600-CFS
WRC SKEW=	0.189-LOG10	WRC MEAN=	3.745-LOG10	WRC SD=	0.282-LOG10
YRSPK=	26-YEARS	YRSHISPK=	79-YEARS		

5-42 02470100 EAST BASSETT CREEK AT WALKER SPRINGS, ALA.

A=	188-SQ MI	S=	9.300-FT/MI	LAT=	31.54-DEC. DEG
LONG=	87.79-DEC. DEG	P=	57.00-IN	I24-2=	4.900-IN
Q2=	5540-CFS	Q5=	9010-CFS	Q10=	11400-CFS
Q25=	14400-CFS	P50=	16600-CFS	Q100=	18900-CFS
WRC SKEW=	-0.356-LOG10	WRC MEAN=	3.728-LOG10	WRC SD=	0.266-LOG10
YRSPK=	21-YEARS	YRSHISPK=	77-YEARS		

5-43 02471026 WATSON CREEK NEAR STOCKTON, ALA. SP

A=	2.25-SQ MI	S=	43.800-FT/MI	LAT=	31.03-DEC. DEG
LONG=	87.84-DEC. DEG	P=	64.00-IN	I24-2=	5.500-IN
Q2=	384-CFS	Q5=	599-CFS	Q10=	758-CFS
Q25=	977-CFS	Q50=	1150-CFS	Q100=	1340-CFS
WRC SKEW=	0.075-LOG10	WRC MEAN=	2.587-LOG10	WRC SD=	0.227-LOG10
YRSPK=	6-YEARS	YRSHISPK=	50-YEARS		

5-44 02473500 TALLAHALA CREEK AT LAUREL, MISS.

A=	233-SQ MI	S=	3.200-FT/MI	LAT=	31.68-DEC. DEG
LONG=	89.12-DEC. DEG	P=	57.00-IN	I24-2=	4.700-IN
Q2=	5790-CFS	Q5=	11600-CFS	Q10=	16300-CFS
Q25=	23300-CFS	Q50=	29400-CFS	Q100=	35500-CFS
WRC SKEW=	-0.207-LOG10	WRC MEAN=	3.750-LOG10	WRC SD=	0.368-LOG10
YRSPK=	39-YEARS	YRSHISPK=	98-YEARS		

5-45 02474500 TALLAHALA CREEK NEAR RUNNELSTOWN, MISS.

A=	612-SQ MI	S=	2.500-FT/MI	LAT=	31.33-DEC. DEG
LONG=	89.11-DEC. DEG	P=	58.00-IN	I24-2=	4.700-IN
Q2=	7790-CFS	Q5=	12900-CFS	Q10=	17000-CFS
Q25=	23000-CFS	Q50=	28100-CFS	Q100=	33900-CFS
WRC SKEW=	0.220-LOG10	WRC MEAN=	3.901-LOG10	WRC SD=	0.253-LOG10
YRSPK=	37-YEARS	YRSHISPK=	92-YEARS		

5-46 02475500 CHUNKY RIVER NEAR CHUNKY, MISS.

A=	368-SQ MI	S=	5.100-FT/MI	LAT=	32.33-DEC. DEG
LONG=	88.91-DEC. DEG	P=	51.00-IN	I24-2=	4.500-IN
Q2=	8590-CFS	Q5=	16000-CFS	Q10=	22200-CFS
Q25=	31700-CFS	Q50=	40000-CFS	Q100=	49300-CFS
WRC SKEW=	0.071-LOG10	WRC MEAN=	3.938-LOG10	WRC SD=	0.318-LOG10
?RSPK=	38-YEARS	YRSHISPK=	NONE-YEARS		

5-47 02479420 WHITES BRANCH NEAR ESCATAWPA, ALA.

A=	2.56-SQ MI	S=	26.300-FT/MI	LAT=	31.30-DEC. DEG
LONG=	88.38-DEC. DEG	P=	65.00-IN	I24-2=	5.000-IN
Q2=	370-SQ MI	Q5=	593-CFS	Q10=	763-CFS
Q25=	1000-CFS	Q50=	1200-CFS	Q100=	1410-CFS
WRC SKEW=	0.134-LOG10	WRC MEAN=	2.573-LOG10	WRC SD=	0.238-LOG10
YRSPK=	5-YEARS	YRSHISPK=	50-YEARS		

5-48 02479500 ESCATAWPA RIVER NEAR WILMER, ALA.

A=	506-SQ MI	S=	2.700-FT/MI	LAT=	30.86-DEC. DEG
LONG=	88.42-DEC. DEG	P=	63.00-IN	I24-2=	5.100-IN
Q2=	9320-CFS	Q5=	14800-CFS	Q10=	19200-CFS
Q25=	25900-CFS	Q50=	31700-CFS	Q100=	38200-CFS
WRC SKEW=	0.425-LOG10	WRC MEAN=	3.985-LOG10	WRC SD=	0.227-LOG10
YRSPK=	30-YEARS	YRSHISPK=	NONE-YEARS		

5-49 02479583 FLAT CREEK NEAR WILMER, ALA.

A=	6.30-SQ MI	S=	25.500-FT/MI	LAT=	30.78-DEC. DEG
LONG=	88.40-DEC. DEG	P=	62.00-IN	I24-2=	5.600-IN
Q2=	657-CFS	Q5=	1090-CFS	Q10=	1430-CFS
Q25=	1900-CFS	Q50=	2300-CFS	Q100=	2710-CFS
WRC SKEW=	0.044-LOG10	WRC MEAN=	2.819-LOG10	WRC SD=	0.260-LOG10
YRSPK=	6-YEARS	YRSHISPK=	50-YEARS		

5-50 02480500 TUXACHANIE CREEK NEAR BILOXI, MISS.

A=	92.4-SQ MI	S=	6.800-FT/MI	LAT=	30.51-DEC. DEG
LONG=	88.91-DEC. DEG	P=	67.00-IN	I24-2=	5.600-IN
Q2=	4150-CFS	Q5=	6390-CFS	Q10=	8280-CFS
Q25=	11200-CFS	Q50=	13900-CFS	Q100=	17000-CFS
WRC SKEW=	0.732-LOG10	WRC MEAN=	3.643-LOG10	WRC SD=	0.206-LOG10
YRSPK=	24-YEARS	YRSHISPK=	71-YEARS		

HYDROLOGIC AREA 6

6-01 02370500 BIG COLDWATER CREEK NEAR MILTON, FLA.

A=	237-SQ MI	S=	6.030-FT/MI	LAT=	30.71-DEC. DEG
LONG=	86.97-DEC. DEG	P=	62.80-IN	I24-2=	4.740-IN
Q2=	5150-CFS	Q5=	10200-CFS	Q10=	14900-CFS
Q25=	22600-CFS	Q50=	29900-CFS	Q100=	38600-CFS
WRC SKEW=	0.272-LOG10	WRC MEAN=	3.727-LOG10	WRC SD=	0.340-LOG10
YRSPK=	39-YEARS	YRSHISPK=	NONE-YEARS		

6-02 02375000 BIG ESCAMBIA CREEK AT FLOMATON, ALA.

A=	323-SQ MI	S=	9.300-FT/MI	LAT=	31.02-DEC. DEG
LONG=	87.25-DEC. DEG	P=	60.00-IN	I24-2=	5.500-IN
Q2=	7250-CFS	Q5=	13600-CFS	Q10=	19800-CFS
Q25=	30900-CFS	Q50=	42200-CFS	Q100=	56600-CFS
WRC SKEW=	0.758-LOG10	WRC MEAN=	3.898-LOG10	WRC SD=	0.299-LOG10
YRSPK=	37-YEARS	YRSHISPK=	125-YEARS		

6-03 02376000 PINE BARREN CREEK NEAR BARTH, FLA.

A=	75.3-SQ MI	S=	11.900-FT/MI	LAT=	30.80-DEC. DEG
LONG=	87.37-DEC. DEG	P=	63.30-IN	I24-2=	4.670-IN
Q2=	2650-CFS	Q5=	6640-CFS	Q10=	11000-CFS
Q25=	19000-CFS	Q50=	27400-CFS	Q100=	38200-CFS
WRC SKEW=	0.202-LOG10	WRC MEAN=	3.439-LOG10	WRC SD=	0.462-LOG10
YRSPK=	26-YEARS	YRSHISPK=	NONE-YEARS		

6-04 02376500 PERDIDO RIVER NR BARINEAU PARK, FLA.

A=	394-SQ MI	S=	5.510-FT/MI	LAT=	30.69-DEC. DEG
LONG=	87.44-DEC. DEG	P=	64.20-IN	I24-2=	4.530-IN
Q2=	6880-CFS	Q5=	11700-CFS	Q10=	15700-CFS
Q50=	21600-CFS	Q50=	26800-CFS	Q100=	32600-CFS
WRC SKEW=	0.233-LOG10	WRC MEAN=	3.848-LOG10	WRC SD=	0.266-LOG10
YRSPK=	37-YEARS	YRSHISPK=	NONE-YEARS		

6-05 02377500 STYX RIVER NEAR LOXLEY, ALA.

A=	93.2-SQ MI	S=	8.900-FT/MI	LAT=	30.66-DEC. DEG
LONG=	87.64-DEC. DEG	P=	67.00-IN	I24-2=	5.650-IN
Q2=	3100-CFS	Q5=	6220-CFS	Q10=	9350-CFS
Q25=	14900-CFS	Q50=	20600-CFS	Q100=	27800-CFS
WRC SKEW=	0.558-LOG10	WRC MEAN=	3.522-LOG10	WRC SD=	0.338-LOG10
YRSPK=	24-YEARS	YRSHISPK=	52-YEARS		

6-06 02378500 FISH RIVER NEAR SILVER HILL, ALA.

A=	55.1-SQ MI	S=	9.800-FT/MI	LAT=	30.55-DEC. DEG
LONG=	87.80-DEC. DEG	P=	68.00-IN	I24-2=	6.200-IN
Q2=	1540-CFS	Q5=	3500-CFS	Q10=	5460-CFS
Q25=	8850-CFS	Q50=	12200-CFS	Q100=	16300-CFS
WRC SKEW=	0.149-LOG10	WRC MEAN=	3.197-LOG10	WRC SD=	0.417-LOG10
YRSPK=	17-YEARS	YRSHISPK=	20-YEARS		

6-07 02471001 CHICKASAW CREEK NEAR KUSHLA, ALA.

A=	125-SQ MI	S=	8.100-FT/MI	LAT=	30.80-DEC. DEG
LONG=	88.14-DEC. DEG	P=	66.00-IN	I24-2=	5.800-IN
Q2=	4550-CFS	Q5=	8610-CFS	Q10=	12300-CFS
Q25=	18300-CFS	Q50=	23800-CFS	Q100=	30400-CFS
WRC SKEW=	0.308-LOG10	WRC MEAN=	3.674-LOG10	WRC SD=	0.317-LOG10
YRSPK=	30-YEARS	YRSHISPK=	52-YEARS		

6-08 02480150 FRANKLIN CREEK NEAR GRAND BAY, ALA.

A=	16.4-SQ MI	S=	13.500-FT/MI	LAT=	30.47-DEC. DEG
LONG=	88.39-DEC. DEG	P=	64.00-IN	I24-2=	5.400-IN
Q2=	878-CFS	Q5=	1630-CFS	Q10=	2250-CFS
Q25=	3170-CFS	Q50=	3960-CFS	Q100=	4830-CFS
WRC SKEW=	-0.013-LOG10	WRC MEAN=	2.943-LOG10	WRC SD=	0.320-LOG10
YRSPK=	20-YEARS	YRSHISPK=	21-YEARS		

STREAMS WITH DRAINAGE AREAS GREATER THEN 1500 SQUARE MILES

BDA-01 02365500 CHOCTAWHATCHEE R AT CARYVILLE, FLA.

A=	3499-SQ MI	S=	2.000-FT/MI	LAT=	30.78-DEC. DEG
LONG=	85.83-DEC. DEG	P=	54.00-IN	I24-2=	4.400-IN
Q2=	31500-CFS	Q5=	50600-CFS	Q10=	66200-CFS
Q25=	89700-CFS	Q50=	110000-CFS	Q100=	133000-CFS
WRC SKEW=	0.407-LOG10	WRC MEAN=	4.514-LOG10	WRC SD=	0.233-LOG10
YRSPK=	51-YEARS	YRSHISPK=	NONE-YEARS		

BDA-02 02374000 CONECUH RIVER NEAR BROOKLYN, ALA.

A=	2460-SQ MI	S=	3.700-FT/MI	LAT=	31.17-DEC. DEG
LONG=	86.80-DEC. DEG	P=	58.00-IN	I24-2=	4.750-IN
Q2=	31500-CFS	Q5=	43700-CFS	Q10=	57800-CFS
Q25=	79000-CFS	Q50=	97300-CFS	Q100=	118000-CFS
WRC SKEW=	0.287-LOG10	WRC MEAN=	4.432-LOG10	WRC SD=	0.252-LOG10
YRSPK=	29-YEARS	YRSHISPK=	115-YEARS		

BDA-03 02375500 ESCAMBIA RIVER NEAR CENTURY, FLA.

A=	3817-SQ MI	S=	2.170-FT/MI	LAT=	30.96-DEC. DEG
LONG=	87.23-DEC. DEG	P=	57.90-IN	I24-2=	4.300-IN
Q2=	39000-CFS	Q5=	64700-CFS	Q10=	86100-CFS
Q25=	118000-CFS	Q50=	147000-CFS	Q100=	179000-CFS
WRC SKEW=	0.355-LOG10	WRC MEAN=	4.606-LOG10	WRC SD=	0.250-LOG10
YRSPK=	44-YEARS	YRSHISPK=	50-YEARS		

BDA-04 02397000 COOSA RIVER NEAR ROME, GA.

A=	4036-SQ MI	S=	4.300-FT/MI	LAT=	34.20-DEC. DEG
LONG=	85.26-DEC. DEG	P=	52.50-IN	I24-2=	3.600-IN
Q2=	33500-CFS	Q5=	42500-CFS	Q10=	49000-CFS
Q25=	57000-CFS	Q50=	63000-CFS	Q100=	70000-CFS
WRC SKEW=	0.478-LOG10	WRC MEAN=	4.534-LOG10	WRC SD=	0.116-LOG10
YRSPK=	61-YEARS	YRSHISPK=	88-YEARS		

BDA-05 02400500 COOSA RIVER AT GADSDEN, ALA.

A=	5800-SQ MI	S=	3.700-FT/MI	LAT=	34.01-DEC. DEG
LONG=	86.00-DEC. DEG	P=	53.00-IN	I24-2=	3.950-IN
Q2=	48800-CFS	Q5=	58300-CFS	Q10=	66000-CFS
Q25=	77500-CFS	Q50=	90000-CFS	Q100=	102000-CFS
WRC SKEW=	2.410-LOG10	WRC MEAN=	4.715-LOG10	WRC SD=	0.077-LOG10
YRSPK=	91-YEARS	YRSHISPK=	95-YEARS		

BDA-06 02407000 COOSA RIVER AT CHILDERSBURG, ALA.

A=	8390-SQ MI	S=	1.000-FT/MI	LAT=	33.29-DEC. DEG
LONG=	86.36-DEC. DEG	P=	55.00-IN	I24-2=	4.150-IN
Q2=	72000-CFS	Q5=	89000-CFS	Q10=	105000-CFS
Q25=	130000-CFS	Q50=	150000-CFS	Q100=	171000-CFS
WRC SKEW=	1.190-LOG10	WRC MEAN=	4.879-LOG10	WRC SD=	0.113-LOG10
YRSPK=	91-YEARS	YRSHISPK=	95-YEARS		

BDA-07 02411000 COOSA R AT JORDAN DAM NEAR WETUMPKA, ALA.

A=	10200-SQ MI	S=	1.200-FT/MI	LAT=	32.61-DEC. DEG
LONG=	86.26-DEC. DEG	P=	56.00-IN	I24-2=	4.200-IN
Q2=	146000-CFS	Q5=	197000-CFS	Q10=	228000-CFS
Q25=	266000-CFS	Q50=	293000-CFS	Q100=	326000-CFS
WRC SKEW=	0.080-LOG10	WRC MEAN=	5.166-LOG10	WRC SD=	0.145-LOG10
YRSPK=	56-YEARS	YRSHISPK=	95-YEARS		

BDA-08 02414500 TALLAPOOSA RIVER AT WADLEY, ALA.

A=	1660-SQ MI	S=	4.100-FT/MI	LAT=	33.12-DEC. DEG
LONG=	85.57-DEC. DEG	P=	52.00-IN	I24-2=	3.900-IN
Q2=	30900-CFS	Q5=	44000-CFS	Q10=	52000-CFS
Q25=	61300-CFS	Q50=	67800-CFS	Q100=	73800-CFS
WRC SKEW=	-0.437-LOG10	WRC MEAN=	4.476-LOG10	WRC SD=	0.196-LOG10
YRSPK=	58-YEARS	YRAHISPK=	62-YEARS		

BDA-09 02416000 TALLAPOOSA RIVER AT STURDIVANT, ALA.

A=	2460-SQ MI	S=	4.300-FT/MI	LAT=	32.91-DEC. DEG
LONG=	85.87-DEC. DEG	P=	52.00-IN	I24-2=	4.000-IN
Q2=	50800-CFS	Q5=	74600-CFS	Q10=	90500-CFS
Q25=	111000-CFS	Q50=	126000-CFS	Q100=	141000-CFS
WRC SKEW=	-0.144-LOG10	WRC MEAN=	4.702-LOG10	WRC SD=	0.202-LOG10
YRSPK=	26-YEARS	YRSHISPK=	32-YEARS		

BDA-10 02418500 TALLAPOOSA RIVER BELOW TALLASSEE, ALA.

A=	3320-SQ MI	S=	2.900-FT/MI	LAT=	32.51-DEC. DEG
LONG=	85.89-DEC. DEG	P=	50.00-IN	I24-2=	4.400-IN
Q2=	30200-CFS	Q5=	59100-CFS	Q10=	84600-CFS
Q25=	125000-CFS	Q50=	160000-CFS	Q100=	202000-CFS
WRC SKEW=	0.090-LOG10	WRC MEAN=	4.484-LOG10	WRC SD=	0.343-LOG10
YRSPK=	52-YEARS	YRSHISPK=	95-YEARS		

BDA-11 02420000 ALABAMA RIVER NEAR MONTGOMERY, ALA.

A=	15100-SQ MI	S=	1.480-FT/MI	LAT=	32.41-DEC. DEG
LONG=	86.41-DEC. DEG	P=	52.00-IN	I24-2=	4.550-IN
Q2=	120000-CFS	Q5=	164000-CFS	Q10=	196000-CFS
Q25=	238000-CFS	Q50=	272000-CFS	Q100=	307000-CFS
WRC SKEW=	0.340-LOG10	WRC MEAN=	5.086-LOG10	WRC SD=	0.156-LOG10
YRSPK=	54-YEARS	YRSHISPK=	167-YEARS		

BDA-12 02423000 ALABAMA RIVER AT SELMA, ALA.

A=	17100-SQ MI	S=	1.370-FT/MI	LAT=	32.40-DEC. DEG
LONG=	87.02-DEC. DEG	P=	52.00-IN	I24-2=	4.550-IN
Q2=	119000-CFS	Q5=	156000-CFS	Q10=	178000-CFS
Q25=	204000-CFS	Q50=	222000-CFS	Q100=	240000-CFS
WRC SKEW=	-0.258-LOG10	WRC MEAN=	5.068-LOG10	WRC SD=	0.146-LOG10
YRSPK=	82-YEARS	YRSHISPK=	165-YEARS		

BDA-13 02427500 ALABAMA RIVER NEAR MILLERS FERRY, ALA.

A=	20700-SQ MI	S=	1.250-FT/MI	LAT=	32.12-DEC. DEG
LONG=	87.40-DEC. DEG	P=	54.00-IN	I24-2=	4.750-IN
Q2=	112000-CFS	Q5=	150000-CFS	Q10=	174000-CFS
Q25=	203000-CFS	Q50=	224000-CFS	Q100=	245000-CFS
WRC SKEW=	-0.123-LOG10	WRC MEAN=	5.044-LOG10	WRC SD=	0.154-LOG10
YRSPK=	64-YEARS	YRSHISPK=	166-YEARS		

BDA-14 02429500 ALABAMA RIVER AT CLAIBORNE, ALA.

A=	22000-SQ MI	S=	1.400-FT/MI	LAT=	31.55-DEC. DEG
LONG=	87.51-DEC. DEG	P=	58.00-IN	I24-2=	5.000-IN
Q2=	134000-CFS	Q5=	175000-CFS	Q10=	199000-CFS
Q25=	228000-CFS	Q50=	248000-CFS	Q100=	267000-CFS
WRC SKEW=	-0.262-LOG10	WRC MEAN=	5.120-LOG10	WRC SD=	0.144-LOG10
YRSPK=	88-YEARS	YRSHISPK=	NONE-YEARS		

BDA-15 02424500 CAHABA RIVER AT SPROTT, ALA.

A=	1378-SQ MI	S=	4.370-FT/MI	LAT=	32.48-DEC. DEG
LONG=	87.13-DEC. DEG	P=	52.00-IN	I24-2=	4.500-IN
Q2=	24900-CFS	Q5=	44400-CFS	Q10=	61300-CFS
Q25=	87900-CFS	Q50=	112000-CFS	Q100=	140000-CFS
WRC SKEW=	0.342-LOG10	WRC MEAN=	4.413-LOG10	WRC SD=	0.285-LOG10
YRSPK=	32-YEARS	YRSHISPK=	67-YEARS		

BDA-16 02425000 CAHABA RIVER NEAR MARION JUNCTION, ALA.

A=	1768-SQ MI	S=	3.000-FT/MI	LAT=	32.44-DEC. DEG
LONG=	87.18-DEC. DEG	P=	54.00-IN	I24-2=	4.600-IN
Q2=	26300-CFS	Q5=	46800-CFS	Q10=	61900-CFS
Q25=	81900-CFS	Q50=	97200-CFS	Q100=	113000-CFS
WRC SKEW=	-0.350-LOG10	WRC MEAN=	4.401-LOG10	WRC SD=	0.315-LOG10
YRSPK=	53-YEARS	YRSHISPK=	79-YEARS		

BDA-17 02462500 BLACK WARRIOR R AT BANKHEAD L&D NEAR BESSEMER, ALA.

A=	3979-SQ MI	S=	2.900-FT/MI	LAT=	33.46-DEC. DEG
LONG=	87.35-DEC. DEG	P=	54.00-IN	I24-2=	4.200-IN
Q2=	83000-CFS	Q5=	104000-CFS	Q10=	118000-CFS
Q25=	135000-CFS	Q50=	147000-CFS	Q100=	159000-CFS
WRC SKEW=	0.089-LOG10	WRC MEAN=	4.921-LOG10	WRC SD=	0.117-LOG10
YRSPK=	16-YEARS	YRSHISPK=	53-YEARS		

BDA-18 02465000 BLACK WARRIOR RIVER AT NORTHPORT, ALA.

A=	4828-SQ MI	S=	2.800-FT/MI	LAT=	33.21-DEC. DEG
LONG=	87.58-DEC. DEG	P=	53.00-IN	I24-2=	4.200-IN
Q2=	115000-CFS	Q5=	154000-CFS	Q10=	179000-CFS
Q25=	208000-CFS	Q50=	228000-CFS	Q100=	248000-CFS
WRC SKEW=	-0.203-LOG10	WRC MEAN=	5.056-LOG10	WRC SD=	0.155-LOG10
YRSPK=	53-YEARS	YRSHISPK=	107-YEARS		

BDA-19 02466000 BLACK WARRIOR RIVER NEAR EUTAW, ALA.

A=	5797-SQ MI	S=	1.270-FT/MI	LAT=	32.82-DEC. DEG
LONG=	87.82-DEC. DEG	P=	52.00-IN	I24-2=	4.500-IN
Q2=	75100-CFS	Q5=	112000-CFS	Q10=	139000-CFS
Q25=	179000-CFS	Q50=	211000-CFS	Q100=	246000-CFS
WRC SKEW=	0.339-LOG10	WRC MEAN=	4.886-LOG10	WRC SD=	0.196-LOG10
YRSPK=	34-YEARS	YRSHISPK=	107-YEARS		

BDA-20 03584500 ELK RIVER NEAR PROSPECT, TENN.

A=	1784-SQ MI	S=	2.850-FT/MI	LAT=	35.03-DEC. DEG
LONG=	86.95-DEC. DEG	P=	54.00-IN	I24-2=	3.500-IN
Q2=	34000-CFS	Q5=	51100-CFS	Q10=	65000-CFS
Q25=	84000-CFS	Q50=	100000-CFS	Q100=	115000-CFS
WRC SKEW=	0.148-LOG10	WRC MEAN=	4.537-LOG10	WRC SD=	0.215-LOG10
YRSPK=	52-YEARS	YRSHISPK=	55-YEARS		

LITTLE RIVER BASIN

LR-01 02399000 LITTLE RIVER NEAR JAMESTOWN, ALA.

A=	121-SQ MI	S=	28.200-FT/MI	LAT=	34.40-DEC. DEG
LONG=	85.63-DEC. DEG	P=	53.00-IN	I24-2=	3.800-IN
Q2=	10100-CFS	Q5=	14900-CFS	Q10=	18100-CFS
Q25=	22200-CFS	Q50=	25100-CFS	Q100=	28100-CFS
WRC SKEW=	-0.229-LOG10	WRC MEAN=	3.995-LOG10	WRC SD=	0.210-LOG10
YRSPK=	38-YEARS	YRSHISPK=	NONE-YEARS		

LR-02 02399200 LITTLE RIVER NEAR BLUE POND, ALA.

A=	194-SQ MI	S=	32.500-FT/MI	LAT=	34.29-DEC. DEG
LONG=	85.68-DEC. DEG	P=	53.00-IN	I24-2=	3.900-IN
Q2=	13300-CFS	Q5=	19000-CFS	Q10=	23000-CFS
Q25=	28600-CFS	Q50=	32900-CFS	Q100=	37500-CFS
WRC SKEW=	0.225-LOG10	WRC MEAN=	4.129-LOG10	WRC SD=	0.179-LOG10
YRSPK=	25-YEARS	YRSHISPK=	38-YEARS		

STREAMS WITH NO FLOOD FREQUENCY DETERMINATIONS

NFQ-01 02339500 CHATTAHOOCHEE RIVER AT WEST POINT, GA.

A= 3550-SQ MI S= 2.800-FT/MI LAT= 32.89-DEC. DEG
LONG= 85.18-DEC. DEG P= 52.50-IN I24-2= 3.450-IN

NFQ-02 02341500 CHATTAHOOCHEE RIVER AT COLUMBUS, GA.

A= 4670-SQ MI S= 2.900-FT/MI LAT= 32.46-DEC. DEG
LONG= 85.00-DEC. DEG P= 52.00-IN I24-2= 3.550-IN

NFQ-03 02343500 CHATTAHOOCHEE RIVER AT COLUMBIA, GA.

A= 8040-SQ MI S= 3.100-FT/MI LAT= 31.29-DEC. DEG
LONG= 85.10-DEC. DEG P= 51.00-IN I24-2= 3.750-IN

NFQ-04 02344000 CHATTAHOOCHEE RIVER AT ALAGA, ALA.

A= 8340-SQ MI S= 3.100-FT/MI LAT= 31.12-DEC. DEG
LONG= 85.05-DEC. DEG P= 51.00-IN I24-2= 3.750-IN

NFQ-05 02430000 MACKEYS CREEK NEAR DENNIS, MISS.

A= 66.8-SQ MI S= 8.200-FT/MI LAT= 34.53-DEC. DEG
LONG= 88.32-DEC. DEG P= 53.00-IN I24-2= 3.900-IN

NFQ-06 02431000 TOMBIGBEE RIVER NEAR FULTON, MISS.

A= 612-SQ MI S= 3.500-FT/MI LAT= 34.26-DEC. DEG
LONG= 88.44-DEC. DEG P= 53.00-IN I24-2= 3.900-IN

NFQ-07 02437500 TOMBIGBEE RIVER AT ABERDEEN, MISS.

A= 2169-SQ MI S= 1.800-FT/MI LAT= 34.82-DEC. DEG
LONG= 88.52-DEC. DEG P= 53.00-IN I24-2= 4.000-IN

NFQ-08 02441500 TOMBIGBEE RIVER AT COLUMBUS, MISS.

A= 4490-SQ MI S= 1.400-FT/MI LAT= 33.49-DEC. DEG
LONG= 88.43-DEC. DEG P= 53.00-IN I24-2= 4.000-IN

NFQ-09 02444500 TOMBIGBEE RIVER NEAR COCHRANE, ALA.

A= 5990-SQ MI S= 1.200-FT/MI LAT= 33.08-DEC. DEG
LONG= 88.24-DEC. DEG P= 52.00-IN I24-2= 4.400-IN

NFQ-10 02449000 TOMBIGBEE RIVER AT GAINESVILLE, ALA.

A= 8700-SQ MI S= 1.100-FT/MI LAT= 32.82-DEC. DEG
LONG= 88.16-DEC. DEG P= 52.00-IN I24-2= 4.500-IN

NFQ-11 02467000 TOMBIGBEE R DEMOPOLIS L&D NR DEMOPOLIS, ALA.

A= 15400-SQ MI S= 0.900-FT/MI LAT= 32.52-DEC. DEG
LONG= 87.88-DEC. DEG P= 52.00-IN I24-2= 4.600-IN

NFQ-12 02469761 TOMBIGBEE R COFFEEVILLE L&D NR COFFEEVILLE, ALA.

A= 18500-SQ MI S= 0.760-FT/MI LAT= 31.76-DEC. DEG
LONG= 88.13-DEC. DEG P= 52.00-IN I24-2= 4.500-IN

NFQ-13 02470000 TOMBIGBEE RIVER NEAR LEROY, ALA.

A= 19100-SQ MI S= 0.690-FT/MI LAT= 31.57-DEC. DEG
LONG= 88.02-DEC. DEG P= 52.00-IN I24-2= 4.500-IN

NFQ-14 03571850 TENNESSEE RIVER AT SOUTH PITTSBURG, TENN.

A= 22620-SQ MI S= 3.100-FT/MI LAT= 35.01-DEC. DEG
LONG= 85.70-DEC. DEG P= 52.00-IN I24-2= 3.750-IN

NFQ-15 03573500 TENNESSEE RIVER AT GUNTERSVILLE, ALA.

A= 24340-SQ MI S= 3.000-FT/MI LAT= 34.37-DEC. DEG
LONG= 86.29-DEC. DEG P= 52.50-IN I24-2= 3.850-IN

NFQ-16 03575500 TENNESSEE RIVER AT WHITESBURG, ALA.

A= 25610-SQ MI S= 3.000-FT/MI LAT= 34.57-DEC. DEG
LONG= 86.56-DEC. DEG P= 52.00-IN I24-2= 3.800-IN

NFQ-17 03589500 TENNESSEE RIVER AT FLORENCE, ALA.

A= 30810-SQ MI S= 2.820-FT/MI LAT= 34.79-DEC. DEG
LONG= 87.67-DEC. DEG P= 49.90-IN I24-2= 4.300-IN

SUPPLEMENTARY DATA II

Location, Basin Characteristics File, and Peak Flow File.

BASIN AND STREAMFLOW CHARACTERISTICS FILE

MASTER LIST OF VARIABLES

Data type, identifier for data which is based
on synthetic record.

SP - Floodflow characteristics based on
synthetic or combined synthetic and
observed data.

FILE CODE	DESCRIPTION
A	Drainage area, in square miles, that contributes to surface runoff.
S	Main channel slope, in feet per mile, is the average slope of the main channel between 10 and 85 percent of the total channel length from the gaging station to the basin divide.
L	Stream length, in miles, measured along channel from gage to basin divide.
E	Mean basin elevation, in feet above sea level National Geodetic Vertical Datum of 1929, measured from topo- graphic maps by transparent grid sampling method (20 to 80 points in basin area were sampled).
ST	Area of the basin that is occupied by swamps, ponds, or reservoirs, in percent.
F	Forested area, in percent of contributing drainage area, measured by the grid sampling methods.
Si	Soils index, in inches, a relative measure of po- tential infiltration (soil water storage), from Soil Conservation Service.
LAT	Latitude of gaging station, in decimal degrees.
LONG	Longitude of gaging station, in decimal degrees.
P	Mean annual precipitation, in inches, from U.S. Weather Bureau series, "Climates of States;" grid sampling methods used if isohyetal map is available, otherwise anomaly map constructed (WSP 1580-D).

FILE CODE	DESCRIPTION
I24-2	Precipitation intensity; 24-hour rainfall, in inches, expected on the average once each 2 years. (Estimated from U.S. Weather Bureau Technical Paper 40).
Q2	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 2.
Q5	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 5.
Q10	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 10.
Q25	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 25.
Q50	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 50.
Q100	Annual flood peak, in cfs, of T-year recurrence interval, defined by log-Pearson Type III fitting, from computer program no. J407. T = 100.
WRC SKEW	WRC skew of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments and generalized skew weighting, from computer program no. J407.
WRC MEAN	WRC mean of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments, from computer program no. J407.
WRC SD	WRC standard deviation of logarithms, base 10, of annual peak discharges after outlier and historic-peak adjustments, from computer program no. J407.
YRSPK	Number of years of systematic peak-flow data.
YRSHISPK	Number of consecutive years used for historic-peak adjustment to flood frequency data.

1-03 02398300 CHATTOOGA RIVER ABOVE GAYLESVILLE, ALA.

LOCATION.--Lat 34°17'25", long 85°30'33", In NW1/4 sec. 5, T. 9 S., R. 11 E., Cherokee County, Hydrologic Unit 03150105, on left bank 10 ft (3.0 m) upstream from bridge on county road, 600 ft (183 m) downstream from Mills Creek and 3.5 mi (5.6 km) northeast of Gaylesville.

PERIOD OF RECORD.--January 1959 to September 1967. October 1967 to September 1971 (annual maximum only).

October 1971 to September 1981 (flood hydrograph only).

GAGE.--Water-stage recorder. Datum of gage is 562.11 ft (171.331 m) National Geodetic Datum of 1929 (Alabama Power Company benchmark). December 4, 1958, to July 20, 1959, non-recording gage at same site and datum.

CONTDA=	368.000-SQ MI	SLOPE=	7.000-FT/MI	LENGTH=	42.500-MI	ELEV=	780.000-FT
STORAGE=	0.000E+00-%	FOREST=	60.000-%	SOIL INF=	4.700-IN	LAT GAGE=	34.290-DEC. DEG
LNG GAGE=	85.509-DEC. DEG	PRECIP=	52.000-IN	I24,2=	3.920-IN	P2=	9570.000-CFS
P5=	14400.000-CFS	P10=	17700.000-CFS	P25=	22100.000-CFS	P50=	25500.000-CFS
P100=	29000.000-CFS	WRC SKEW=	-0.055-LOG10	WRC MEAN=	3.979-LOG10	WRC SD=	0.211-LOG10
YRSPK=	51.000-YEARS	YRSH1SPK=	63.000-YEARS				

WATER YEAR	DATE	* PEAK DISCHARGE (CFS)	DISCHARGE CODES	GAGE HEIGHT (FT)	GAGE HT CODES	HIGHEST SINCE	MAX GAGE HEIGHT (FT)	DATE	GAGE HT CODES	NUMBER OF PARTIAL PEAKS
1929	/ /29	11500.00	2							0
1930	/ /30	12800.00	2							0
1931	/ /31	7700.00	2							0
1932	/ /32	10700.00	2							0
1933	/ /33	16600.00	2							0
1934	/ /34	9600.00	2							0
1935	/ /35	7200.00	2							0
1936	/ /36	17800.00	2							0
1937	/ /37	12100.00	2							0
1938	04/09/38	11800.00	2							0
1939	03/01/39	8350.00	2							0
1940	03/14/40	4090.00	2							0
1941	07/16/41	5990.00	2							0
1942	02/17/42	7420.00	2							0
1943	12/30/42	13200.00	2							0
1944	03/29/44	10300.00	2							0
1945	02/15/45	6670.00	2							0
1946	02/11/46	17700.00	2							0
1947	01/21/47	13900.00	2							0
1948	02/14/48	11800.00	2							0
1949	11/29/48	31200.00	2							0
1950	03/14/50	12400.00	2							0
1951	03/30/51	32900.00	2	23.48		1916				0
1952	03/12/52	14300.00	2							0
1953	02/22/53	8390.00	2							0
1954	01/17/54	9470.00	2							0
1955	12/06/54	6620.00	2							0
1956	04/17/56	6790.00	2							0
1957	02/02/57	10900.00	2							0
1958	11/20/57	11200.00	2							0
1959	04/20/59	4220.00	2							0
1960	03/30/60	4050.00		13.93						0

WATER YEAR	DATE	* PEAK DISCHARGE (CFS)	DISCHARGE CODES	GAGE HEIGHT (FT)	GAGE HT CODES	HIGHEST SINCE	MAX GAGE HEIGHT (FT)	DATE	GAGE HT CODES	NUMBER OF PARTIAL PEAKS
1961	02/23/61	12000.00		20.56						0
1961	02/23/61	12000.00		20.56						0
1962	01/28/62	11500.00		20.34						3
	12/13/61	10400.00		19.78						
	12/19/61	10600.00		19.90						
	04/16/62	9260.00		19.21						
1963	04/30/63	13600.00		20.95						2
	03/07/63	7570.00		18.32						
	03/13/63	5120.00		16.17						
1964	03/26/64	13300.00		20.85						5
	01/26/64	6800.00		17.87						
	03/16/64	12200.00		20.42						
	04/08/64			17.98						
	04/14/64			16.05						
	05/04/64			18.85						
1965	03/27/65	10100.00		18.94						2
	03/25/65	5710.00		16.45						
	03/30/65	5270.00		15.88						
1966	03/05/66	16500.00		22.03						2
	02/14/66	5310.00		15.94						
	05/14/66	6500.00		17.07						
1967	02/21/67	4460.00		14.60						0
1968	12/19/67	9000.00	2							0
1969	02/02/69	7000.00	2							0
1970	04/27/70	5620.00		16.35						0
1971	02/05/71	5620.00		16.36						0
1972	05/15/72	9820.00		18.96						1
	01/05/72	5270.00		15.88						
1973	03/18/73	12500.00		20.25						1
	05/29/73	8500.00		18.32						
1974	04/05/74	7200.00		17.56						2
	11/29/73	5400.00		16.07						
	01/01/74	6040.00		16.72						
1975	09/24/75	9100.00		18.60						3
	01/25/75	7580.00		17.80						
	03/15/75	7180.00		17.55						
	03/31/75	5940.00		16.65						
1976	01/27/76	4680.00		14.97						0
1977	04/05/77	2800.00		20.42						0
1978	11/07/77	8720.00		18.43						3
	10/27/77	5350.00		13.06						
	01/26/78	5390.00		15.01						
	05/09/78	6590.00		16.61						
1979	04/13/79	16800.00		22.17						1
	03/05/79	15400.00		21.58						

* PEAK BASE DISCHARGE 5000 CFS

EXPLANATION OF PEAK DATA CODES

DISCHARGE QUALIFICATION CODES:

- 1...DISCHARGE IS A MAXIMUM DAILY AVERAGE
- 2...DISCHARGE IS AN ESTIMATE
- 3...DISCHARGE AFFECTED BY DAM FAILURE
- 4...DISCHARGE LESS THAN INDICATED VALUE, WHICH IS MINIMUM RECORDABLE DISCHARGE AT THIS SITE
- 5...DISCHARGE AFFECTED TO UNKNOWN DEGREE BY REGULATION OR DIVERSION
- 6...DISCHARGE AFFECTED BY REGULATION OR DIVERSION
- 7...DISCHARGE IS AN HISTORIC PEAK
- 8...DISCHARGE ACTUALLY GREATER THAN INDICATED VALUE
- 9...DISCHARGE DUE TO SNOWMELT, HURRICANE, ICE-JAM OR DEBRIS DAM BREAKUP

- A...YEAR OF OCCURRENCE IS UNKNOWN OR NOT EXACT
- B...MONTH OR DAY OF OCCURRENCE IS UNKNOWN OR NOT EXACT
- C...BASE DISCHARGE CHANGED DURING THIS YEAR
- D...ONLY ANNUAL MAXIMUM PEAK AVAILABLE FOR THIS YEAR

GAGE HEIGHT QUALIFICATION CODES:

- 1...GAGE HEIGHT AFFECTED BY BACKWATER
- 2...GAGE HEIGHT NOT THE MAXIMUM FOR THE YEAR
- 3...GAGE HEIGHT AT DIFFERENT SITE AND/OR DATUM
- 4...GAGE HEIGHT BELOW MINIMUM RECORDABLE ELEVATION
- 5...GAGE HEIGHT IS AN ESTIMATE
- 6...GAGE DATUM CHANGED DURING THIS YEAR

SUPPLEMENTARY DATA III

Maximum discharge and elevation for gaging stations in Alabama
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			Maximum Flood Elev. A/ Elevation		
					Date	Elevation (ft) NGVD	Peak (ft ³ /s) / mi ²)	100-Yr Flood (ft ³ /s)	Date	Elevation (ft) NGVD
02340750	Osanippa Creek nr Fairfax	101	(a)	1949-81	2-25-61	16,08	12,800	127	11,000	
02342150	Uchee Creek nr Seale	134	275	1929-81	4-09-64	289,06	19,500	146	18,000	
02342200	Phelps Creek nr Opelia (b)	7.47	530	1959-74	4-08-64	539,85	3,030	406	3,210	
02342500	Uchee Cr nr Fort Mitchell	325	201.76	1929-81	4-09-64	228,21	55,100	170	36,300	
02342933	So. Fk. Cowkee Creek nr Batesville	114	200	1929-81	2-17-75	237,08	18,100	159	16,300	
02343275	Abbie Creek nr Abbeville	46.7	(a)	1929-80	5-04-53	10,31(a)	13,000	278	11,900	
02343300	Abbie Creek nr Haleburg	144	145.74	1952-80	3-31-70	169,58	7,590	52.7	10,000	
02343700	Stevenson Cr nr Headland (b)	12.4	150.39	1960-74	1-06-62	162,40	3,120	252	4,550	
02360000	West Fk. Choctawhatchee River at Blue Springs	84.7	289.24	1929-71	9-26-56	300,74	9,800	116	9,620	
02360275	Judy Creek near Ozark	102	(a)	1936-77	4-07-57	19,70(a)	13,500	132	19,200	
02360500	East Fk. Choctawhatchee River nr Midland City	297	179.10	1953-70	5-04-53	202,92	15,700	52.9	20,700	
	Choctawhatchee R nr Newton	683	138.56	1900-81	3-15-29	180,56	70,000	102	40,900	
8	Hurricane Cr nr Clayton (b)	4.40	410.00	1970-74	6-07-73	417,86	1,700	386	2,140	
	Pea River nr Arinton	492	246.72	1900-81	3- -29	271,72	53,000	108	41,000	
	Moore's Branch nr Victoria(b)	2.17	234.00	1973-81	12-31-73	238,40	510	235	1,340	
02364000	Pea River at Elba	966	159.24	1900-81	3- -29	202,74	90,000	93.2	59,600	
02364500	Pea River nr Samson	1,187	97.95	1900-81	3-15-29	143,25	85,000	71.6	51,600	
02365310	Grants Branch Trib. nr Fadette (b)									
02367500	Lightwood Knot Cr at Babbie	113	192.00	1971-80	9-10-73	201,80	1,360	907	975	
02367800	Yellow River nr Wing	447	185.00	1929-78	4-13-75	199,70	21,600	191	20,800	
				96.13	6-04-70	117,23	39,600	88.6	27,700	
02369800	Blackwater River nr Bradley	86.8	121.87	1929-81	6-04-70	146,07	19,500	225	17,000	
02371000	Conecuh River nr Troy	253	313.30	1865-1981	3- -29	--	27,000(c)	107	29,500	
02371200	Indian Creek nr Troy (b)	8.88	354.00	1959-80	2-17-75	361,75	3,950	445	3,510	
02371500	Conecuh River at Brantley	492	226.20	1865-1981	3- -29	252,20	25,000	50.8	27,300	
02372000	Patsaliga Cr at Luverne	249	267.53	1865-1981	3- -29	--	21,400(c)	85.9	25,700	
02372500	Conecuh R nr Andalusia	1,344	106.77	1865-1980	3-15-29	154,37	154,000	115	70,600	
02372510	Catoe Cr nr Andalusia(b)	2.46	192.00	1970-74	6-30-70	204,50	1,200	488	1,390	
	Sepulga River nr McKenzie	464	155.96	1865-1981	3- -29	188,96	65,000	140	41,000	
	Pegeon Creek nr Thad	296	172.58	1865-1970	3- -29	202,58	38,000	128	31,400	
	Conecuh River nr Brooklin	2,460	76.95	1865-1981	3-15-29	123,95	190,000	77.2	118,000	

Maximum discharge and elevation for gaging stations in Alabama--Continued
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			Maximum Flood Elev. A/ Elevation (ft) NGVD		
					Date	Elevation (ft)	Peak (ft ³ /s)	Peak (ft ³ /s) / mi ²	100-Yr Flood (ft ³ /s)	Date
02374500	Murder Cr nr Evergreen	170	178.29	1865-1981	3-	-29	204.89	70,000	412	30,900
02375000	Big Escambia Cr at Flomaton	323	52.40	1850-1975	3-	-29	81.90	100,000	310	56,600
02377500	Styx River nr Loxley	93.2	39.22	1926-77	9-	-26	61.20	25,000	268	27,800
02378500	Fish River nr Silver Hill	55.1	20.00	1951-71	12-06-53	37.04	8,570	156	16,300	
02398300	Chattooga R ab Gaylesville	368	562.11	1916-79	3-30-51	585.59	32,900	89.4	29,000	
02398500	Chattooga R at Gaylesville	377	549.56	1916-60	3-30-51	574.80	33,700	89.4	36,200	
02399000	Little River nr Jamestown	121	1,177.40	1929-67	3-03-66	1,191.23	25,000	207	28,100 (d)	
02399200	Little River nr Blue Pond	194	581.38	1943-81	3-04-66	595.83	32,000	165	37,500 (d)	
02399500	Coosa River at Leesburg	5,270	517.77	1886-1981	4- 1886	--	120,000 (c)	22.8	NA	
02399800	Little Terrapin Creek at Borden Springs (b)	15.9	781.00	1961-69	3-12-63	789.86	1,820	114	5,440	
02400000	Terrapin Creek nr Piedmont	115	649.79	1929-63	11-28-48	653.09	21,000	183	29,000	
02400033	Nances Cr nr White Plains (b)	4,60	770.00	1971-81	4-04-74	777.53	1,430	311	2,530	
02400100	Terrapin Cr nr Ellisville	258	539.07	1951-81	3-04-79	558.89	20,100	77.9	23,800	
02400500	Coosa River at Gadsden	5,800	485.97	1886-1981	4-06-86	523.87	115,000	19.8	102,000 (e)	
86	Jacks Cr nr Ft Payne (b)	6.76	804.00	1971-74	3-16-73	807.76	540	79.9	2,920	
02401000	Big Wills Cr nr Crudup	185	570.00	1884-1980	- 1984	586.30	18,000	97.3	19,100	
02401390	Big Canoe Cr at Ashville	148	535.00	1961-81	4-13-79	553.75	13,600	91.9	14,700	
02401500	Big Canoe Cr nr Gadsden	256	490.56	1884-1965	12-29-42	519.66	37,900	148	34,200	
02402500	Coosa River at Riverside	7,060	450.00	1886-1981	7-16-16	471.40	82,600	11.7	137,000 (e)	
02404000	Choccolocco Cr nr Jenifer	281	554.15	1886-1969	4-13-79	572.00	23,000	81.6	36,000	
02404245	Cheaha Cr nr Talladega	69.2	(a)	1936-79	3-29-51	20.20(a)	16,000	231	17,600	
02404400	Choccolocco Cr at Jackson									
	Shoals nr Lincoln	484	448.50	1886-1980	4-30-63	488.48	36,900	76.2	64,100 (f)	
02404500	Choccolocco Cr nr Lincoln	499	448.46	1886-1963	- 1886	475.96	66,000	132	64,100 (f)	
02405000	Coosa River nr Cropwell	7,663	420.68	1886-1961	3-30-51	444.38	126,000	16.4	NA	
02405500	Kelley Creek nr Vincent	192	404.09	1952-79	4-13-79	431.48	33,400	174	38,300	
02405800	Talladega Cr ab Talladega	67.3	630.00	1951-70	3-19-70	642.70	6,550	97.3	9,470	
02406000	Talladega Cr nr Talladega	98.4	500.00	1901-62	3- -51	519.00	33,000	335	20,900	
02406500	Talladega Cr at Alpine	148	431.34	1901-79	3-21-51	447.94	39,000	264	31,700	
02407000	Coosa R at Childersburg	8,390	382.45	1886-1981	4-14-79	411.44	149,000	17.8	171,000 (e)	
02407500	Yellowleaf Creek nr Wilsonville	97.2	430.56	1901-79	2-21-61	455.76	26,700	275	32,700	

Maximum discharge and elevation for gaging stations in Alabama--Continued
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			Maximum Flood Elev. A/ Elevation (ft) NGVD		
					Date	Elevation (ft)	Peak (ft ³ /s)	Peak ([ft ³ /s]/mi ²)	100-Yr Flood (ft ³ /s)	Date
02407900	Paint Cr nr Marble Valley(b)	13.5	480.00	1960-72	4-06-64	493.49	7,280	539	4,863	
02408340	Little Hatchet Cr nr Goodwater (b)	9.44	740.00	1967-79	4-13-79	753.71	6,580	697	3,860	
02408500	Hatchet Cr nr Rockford	244	450.00	1929-79	4-13-79	477.03	66,000	270	44,600	
02409000	Weogufka Cr nr Weogufka	73.4	593.08	1900-79	3-29-51	609.88	24,200	330	26,600	
02410000	Patterson Cr nr Central (b)	4.95	440.00	1952-81	8-02-69	450.10	4,310	871	2,480	
02411000	Coosa R at Jordan Dam at Wetumpka	10,200	141.60	1886-1981	4-13-79	189.27	316,000	31.0	326,000 (e)	
02412000	Tallapoosa R nr Heflin	444	830.00	1920-81	3-31-77	861.34	32,500	73.2	29,600	
02412300	Elder Cr nr Dempsey (b)	1.79	950.00	1973-81	3-04-79	960.18	953	532	1,220	
02412500	Tallapoosa R nr Ofelia	787	665.00	1886-1979	12- 19	686.00	41,000	52.1	43,900	
02413400	Wedowee Cr ab Wedowee (b)	6.5	1,050.00	1936-79	4-13-79	1,058.22	1,900	292	2,890	
02413475	Wedowee Cr nr Wedowee Little Tallapoosa River	51.1	(a)	1949-79	4-13-79	14.10(a)	4,870	95.3	6,220	
02413500	nr Wedowee	592	680.00	1919-81	12- 18	703.00	30,000	50.7	33,700	
02414500	Tallapoosa R at Wadley	1,660	599.87	1919-81	4-14-79	630.44	89,100	53.7	73,800	
02414800	Harbuck Creek nr Hackneyville (b)	6.70	710.00	1951-79	4-14-79	721.87	6,260	934	3,080	
02415000	Hillabee Cr nr Hackneyville	196	557.92	1951-79	4-14-79	586.02	26,400	135	26,500	
02416000	Tallapoosa R at Studivant	2,460	440.00	1888-1926	12-11-19	--	160,000	65.0	141,000 (d)	
02417400	Stearns C nr Semon (b)	1.28	650.00	1966-73	8-02-69	656.57	676	528	928	
02418500	Tallapoosa R b1 Tallassee	3,320	164.01	1886-1981	12- 19	--	177,000	53.3	202,000 (d)	3-15-29
02419000	Uphapee C nr Tuskegee	330	223.65	1901-81	- 29	253.65	42,000	127	50,200	215.36
02419625	Calebee C nr Tuskegee	126	222.05	1944-70	3-07-58	239.45	23,000	183	44,300	
02420000	Alabama R nr Montgomery	15,100	97.90	1814-1981	4-01-1886	160.00	322,000	21.3	307,000 (d)	
02420500	Autauga Cr nr Prattville	109	164.38	1885-1979	12-09-19	183.23	23,000	211	14,700	
02421000	Catoma Cr nr Montgomery	298	151.02	1949-81	2-25-61	179.67	48,600	163	62,200	
02421300	Ivy Creek at Mulberry (b)	10.5	210.00	1961-72	4-06-64	225.81	2,440	232	3,960	
02422000	Big Swamp Cr nr Lowndesboro	247	427.95	1920-73	11-27-48	149.25	37,000	150	41,600	
02422500	Mulberry Creek at Jones	208	165.23	1868-1981	4- 38	198.33	48,000	231	34,900	
02423000	Alabama River at Selma	17,100	61.80	1814-1981	3-01-61	119.77	248,000	14.5	233,000 (d)	
02423500	Cahaba River near Acton	230	375.00	1929-79	- 38	--	27,300 (c)	119	43,100	
02423800	L. Cahaba R nr Brierfield	148	325.00	1943-79	2-21-61	346.07	10,000	67.6	15,200	
02424000	Cahaba R at Centreville	1,029	180.74	1902-81	3-29-51	215.54	83,600	81.2	117,000	4-08-38
										217.37

Maximum discharge and elevation for gaging stations in Alabama--Continued
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			Maximum Flood Elev. A/ Elevation (ft) NGVD		
					Date	Elevation (ft)	Peak (ft ³ /s)	Peak ([ft ³ /s]/mi ²)	100-Yr Flood (ft ³ /s)	Date (ft)
02424010	Sandy Cr nr Centreville (b)	0.63	417.00	1971-78	3-14-80	425.14	490	778	551	551
02424500	Cahaba River at Sprott	1,378	129.51	1902-69	4-09-36	158.06	95,000	68.9	140,000(d)	
02425000	Cahaba River nr Marion Junction	1,768	86.72	1902-81	2-24-81	130.52	85,500	48.4	113,000(d)	
02425500	Cedar Creek at Minter	217	123.50	1902-81	2-17-75	150.03	99,800	460	42,900	
02425655	Mush Creek nr Selma	45.4	(a)	1920-75	4-11-55	20.10(a)	22,100	487	26,600	
02426000	Boquechitta Cr nr Browns	104	129.39	1929-81	12-28-42	150.09	19,000	183	20,900	
02427013	Caine Cr nr Safford (b)	2.67	130.00	1973-76	3-16-73	137.10	1,430	536	1,600	
02427300	Praire Cr nr Oak Hill (b)	9.73	220.00	1960-74	3-02-72	235.00	1,860	191	3,770	
02427500	Alabama River near Millers Ferry	20,700	26.82	1814-1981	3-03-61	86.82	284,000	13.7	245,000(d)	
02427700	Turkey Cr at Kimbrough	114	58.78	1929-81	12-10-61	83.80	39,600	347	29,200	
02427875	Pursley Cr nr Camden	60.2	(a)	1949-70	3-31-61	25.90(a)	11,400	189	13,000	
02428300	Tallahatchee Cr nr Vredenburg (b)	14.6	109.73	1959-74	3-01-72	123.00	9,650	661	4,990	
02428500	Flat Creek at Fountain	245	45.43	1929-70	11-27-48	68.63	26,000	106	33,700	
02429000	Limestone Cr nr Monroeville	117	104.88	1929-73	2-25-61	121.16	30,600	262	26,700	
02429500	Alabama R at Claiborne	22,000	0.40	1814-1981	3-25-29	55.00	270,000	12.3	267,000(d)	
02429595	Little River nr Uriah	99.2	130.00	1955-79	9-08-74	146.14	12,500	126	15,500	
02437800	Barn Cr nr Hackleburg (b)	12.9	575.00	1959-73	3-16-73	589.76	5,160	400	4,700	
02437900	Woods Cr nr Hamilton (b)	14.1	470.00	1960-72	12-18-67	484.22	2,500	177	4,970	
02438000	Buttahatchee R bl Hamilton	284	360.50	1916-81	3-16-73	395.99	49,500	174	38,800	
02439000	Buttahatchee R nr Sulligent	472	287.58	1916-81	3-17-73	304.89	60,100	127	46,400	
02442000	Luxapallila Cr nr Fayette	127	322.33	1900-79	1936	--	14,000(c)	110	14,400	
02444000	Coal Fire Creek nr Pickensville	131	148.50	1955-80	4-13-79	160.24	16,400	125	17,800	
02444500	Tombigbee R nr Cochrane	5,990	89.85	1874-1981	4-1892	140.05	255,000(c)	42.6	NA	
02445000	Lubbub Cr nr Carrollton	116	174.24	1955-79	4-13-79	190.24	19,200	166	17,500	
02445245	New River nr Winfield	55.6	387.80	1951-73	3-16-73	412.10	7,970	143	12,300	
02445500	Sipsey River at Fayette	276	296.72	1900-79	2-07-50	317.92	20,500	74.3	27,600	
02446000	Sipsey R at Moores Bridge	403	240.95	1900-58	1-10-46	257.73	23,600	58.6	30,900	
02446500	Sipsey River nr Elrod	518	197.81	1900-81	2-23-61	216.64	27,800	53.7	29,600	
02447000	Sipsey R nr Pleasant Ridge	753	105.13	1916-78	2-25-61	131.73	31,700	42.1	30,100	
02448500	Noxubee R nr Geiger	1,140	86.08	1892-1981	4-14-79	134.66	156,000	137	66,000	

Maximum discharge and elevation for gaging stations in Alabama--Continued
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			100-Yr Flood (ft ³ /s) / (mi ²)	Date	Maximum Flood Elev. ft NGVD
					Elevation (ft) NGVD	Peak (ft ³ /s)	Peak ([ft ³ /s] / mi ²)			
02449000	Tombigbee River at Gainesville	8,700	63.29	1818-1981	4-15-79	119.57	261,000	30.0	NA	- 1892 121.69
02449400	Jones Creek nr Epes (b)	11.7	103.00	1959-74	2-21-61	124.46	5,160	441	4,160	NA
02449500	Tombigbee River at Epes	8,970	52.15	1892-1979	4-16-79	108.92	247,000	27.5		
02450000	Mulberry Fork nr Garden City	365	380.54	1900-81	2-04-36	404.54	46,600	128	51,300	
02450200	Dorsey Cr nr Arkadelphia(b)	13.0	430.00	1918-74	4-13-64	440.02	2,850	219	4,670	
02450250	Sipsey Fork nr Grayson	90.1	540.00	1961-81	3-16-73	584.27	20,300	225	24,900	
02450500	Sipsey Fork nr Falls City	360	360.00	1900-60	1-08-46	389.60	48,400	134	NA	
02451000	Clear Creek at Falls City	149	460.00	1900-60	1-08-46	470.97	13,000	87.2	NA	
02451500	Sipsey Fork nr Arley	524	(a)	1900-60	1-08-46	62.10(a)	57,000	109	NA	
02451550	Jaybird Cr nr West Point(b)	1.42	847.00	1966-73	4-25-70	853.26	369	260	970	
02451750	Vest Creek nr Baldwin(b)	1.64	798.00	1964-72	4-07-64	806.07	1,300	793	1,150	
02453000	Blackwater Cr nr Manchester	188	401.04	1900-81	2-23-61	414.14	10,600	56.4	NA	
02453900	Cheatham Cr nr Carbon Hill	4.77	390.00	1968-74	3-30-73	397.54	854	179	2,340	
02454000	Lost Creek nr Oakman	134	280.00	1900-81	3-29-51	--	20,000(c)	149	22,900	2-23-61
02454200	Wolf Creek nr Oakman	85.0	270.00	1958-79	3-20-70	296.50	15,000	176	20,600	
02454500	Locust Fork below Snead	147	702.94	1951-71	4-30-63	732.81	12,200	83.0	16,700	
02455000	Locust Fork nr Cleveland	303	536.94	1900-81	12-28-42	556.14	47,000	155	43,000	
02455500	Locust Fork at Trafford	624	309.12	1872-1980	- 1908	369.12	62,400(c)	100	76,200	
02456000	Turkey Creek at Morris	80.9	345.18	1929-79	3-19-70	368.30	15,600	193	18,800	
02456500	Locust Fork at Sayre	885	258.64	1872-1981	- 1908	--	56,000(c)	63.3	68,500	2-23-61
02457700	Fivemile Creek at Linn Crossing	96.2	(a)	1943-75	3-20-70	21.73(a)	14,500	151	12,600	
02462000	Valley Creek nr Oak Grove	145	320.00	1900-81	7- -16	349.60	26,800	185	32,200	4-13-79
02462600	Blue Creek nr Oakman (b)	5.32	332.00	1960-81	3-19-70	339.70	4,250	746	2,730	
02462800	Davis Creek below Abernart	45.2	410.00	1916-78	2-21-61	428.30	5,800	128	5,320	
02463500	Hurricane Creek nr Holt	108	173.70	1953-69	2-21-61	196.03	16,800	156	20,600	
02464000	North River nr Sammantha	219	232.39	1916-81	3-20-70	267.47	25,500	116	24,800	
02465000	Black Warrior River nr Northport	4,828	83.35	1874-1981	4-13-79	150.20	272,000	56.3	248,000(d)	4-18-1900 151.05
02465205	Jay Creek nr Coker (b)	3.56	170.00	1964-74	2-21-71	177.20	1,470	413	1,880	
02465500	Fivemile Cr nr Greensboro	72.2	160.00	1936-79	2-22-61	169.84	7,200	99.7	8,090	
02466000	Black Warrior R nr Eutaw	5,797	53.11	1874-1981	4-16-79	--	222,000	38.3	246,000(d)	- 1961 113.41

Maximum discharge and elevation for gaging stations in Alabama--Continued
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi. ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			Maximum Flood Elev., A/ Elevation (ft) NGVD		
					Date	Elevation (ft)	Peak (ft ³ /s)	Peak (ft ³ /s)	100-Yr Flood (ft ³ /s)	Date (ft ³ /s)
02466500	Prairie Creek nr Gallion	169	(a)	1900-60	12-28-42	19.30(a)	25,100	149	54,600	
02467000	Tombigbee River at Demopolis Lock & Dam nr Coatopa	15,400	56.00	1818-1981	4-18-79	93.03	343,000	22.3	NA	- 1900 129.10
02467500	Sucarnoochee River at Livingston	606	90.04	1929-81	4-14-79	123.51	62,200	103	47,300	
02468000	Alamuchee Cr nr Cuba	63.0	161.50	1902-79	4-14-79	180.37	14,700	233	14,300	
02468500	Chickasaw Boque nr Linden	258	63.45	1902-81	3-04-79	93.63	38,700	150	47,000	3-26-45 93.78
02469000	Kinterbish Cr nr York	91.4	120.00	1902-79	4-06-64	143.00	15,000	164	9,030	
02469500	Tuckabum Cr nr Butler	112	(a)	1902-79	4-06-64	22.90 (a)	35,100	313	24,500	
02469550	Horse Creek nr Sweetwater	60.4	129.00	1865-1979	3-08-79	147.66	28,100	532	38,200	
02469700	Okatuppa Cr at Gilberttown	151	59.41	1902-70	2-21-61	76.81	15,000	99.3	9,630	
02469736	Little Souwilla Creek nr Bolinger(b)	7.25	150.00	1969-73	4-13-69	160.66	1,880	259	3,010	
02469761	Tombigbee River at Coffeeville lock & dam	18,500	14.00	1818-1980	4-22-79	79.46	290,000	15.7	NA	5-01-1874 81.40
90	02469800 Satilpa Cr nr Coffeeville	166	80.00	1902-81	7-08-56	98.37	25,600	154	27,600	
02470000	Tombigbee River nr Leroy	19,100	7.28	1874-1965	5--1874	--	280,000	14.7	NA	3-05-61 55.52
02470100	East Bassett Creek at Walker Springs	188	60.02	1902-79	7-08-56	72.27	19,300	103	18,900	
02471001	Cickasaw Cr nr Kushla	125	3.85	1929-81	4-13-55	--	42,000	336	30,400	4-13-80 23.55
02471026	Watson Cr nr Stockton(b)	2.25	52.00	1969-74	12-15-70	56.93	915	407	1,340	
02479420	Whites Branch nr Escatawpa(b)	2.56	175.00	1971-75	4-13-74	179.60	620	242	1,410	
02479500	Escatawpa River nr Wilmer	506	55.01	1946-75	6-02-59	79.67	30,000	59.3	38,200	
02479583	Flat Creek nr Wilmer (b)	6.30	112.00	1969-74	8-18-69	117.62	261	41.4	2,710	
02480150	Franklin Cr nr Grand Bay	16.4	0.00	1959-79	4-12-61	16.54	2,750	168	4,830	
03572900	Town Creek nr Geraldine	141	1,000.00	1929-80	4-29-63	1,021.70	17,700	126	20,600	
03573000	Short Creek nr Albertville	91.6	865.80	1943-69	12- -42	887.00	25,000	273	25,700	
03573506	Tennessee River at Guntersville	24,340	546.31	1867-1938	3-13-1867	594.3	--	--	NA	
03574405	Little Dry Cr nr Garth (b)	3.91	615.00	1971-74	3-10-17	583.71	350,000	14.4	2,360	
03574500	Paint Rock R nr Woodville	320	570.95	1867-1981	3-16-73	624.16	1,700	435	52,200	
03574796	Walker Branch nr Plevna	0.44	810.00	1902-75	4-01-70	820.8	454	1,032	331	
03575000	Flint River nr Chase	342	640.37	1867-1981	3-16-73	669.89	104,000	304	75,200	
03575340	Glover Cove Creek nr Owens Cross Roads	3.52	610.00	1902-74	7-14-49	618.68	767	218	893	
03575500	Tennessee River at Whitesburg	25,610	549.00	1867-81	3-15-1867	582.00	459,000	17.9	NA	
03575830	Indian Creek nr Madison	49.00	600.00	1867-1981	3-12-73	612.70	16,500	337	13,200	

Maximum discharge and elevation for gaging stations in Alabama--Continued
 (Note: A - where no values given same as those given in maximum known flood)

Number	Station Name	Drainage Area (mi ²)	Datum (ft) NGVD	Period of known floods*	Maximum Known Flood			Maximum Flood Elev./Elevation		
					Date	Elevation (ft) NGVD	Peak (ft ³ /s) / mi ²	100-Yr Flood (ft ³ /s)	Date	Elevation (ft) NGVD
03576148	Cotaco Creek at Florrette	136	570.00	1963-81	12-26-73	587.89	19,500	143	23,100	
03576250	Limestone Creek nr Athens	119	626.34	1867-1981	3-16-73	643.62	45,800	385	30,800	
03576400	Piney Creek nr Athens	55.8	655.00	1951-70	3-12-63	668.38	12,900	231	12,100	
03576500	Flint Creek nr Falkville	86.3	572.59	1867-1981	3-16-73	588.44	12,500	145	14,100	
03577000	W Fk Flint Cr nr Oakville	87.6	576.59	1941-69	1- -49	--	7,800	89.0	10,900	12-18-67 599.91
				1- -50	--	7,800				
03577110	W Fk Flint Cr nr Hartselle	158	553.67	1967-1958	1-05-49	571.65	10,300	65.2	13,800	
03585300	Sugar Cr nr Good Springs	152	575.00	1955-69	1-07-50	571.65	10,300			
03585380	W Fk Anderson Creek nr Lexington (b)	5.92	732.00	1969-75	3-08-61	588.37	35,000	230	40,700	
03586500	Big Nance Cr at Courtland	166	537.60	1867-1981	3-16-73	562.57	27,200	164	2,570	
03589500	Tennessee R at Florence	30,810	401.12	1867-1981	3-19-1897	433.62	444,000	14.4	16,600	
										NA
03590000	Cypress Creek nr Florence	209	423.78	1867-1979	3-24-55	453.76	50,000	239	35,200	
03591570	Bear Creek at Posey Hill	26.8	791.45	1867-1979	3-16-73	817.83	4,200	157	3,780	
03591800	Bear Creek nr Hackleburg	143	646.50	1867-1981	3-16-73	685.50	24,000	168	23,800	
03592000	Bear Creek nr Red Bay	263	506.42	1867-1981	3-17-73	524.04	34,800	132	25,500	
03592200	Cedar Cr nr Pleasant Site	189	482.67	1867-1979	3-16-73	510.69	27,100	143	25,500	
03592300	Little Bear Cr nr Halltown	78.2	499.30	1867-1979	3-16-73	517.48	20,400	261	11,300	
03592500	Bear Creek at Bishop	667	419.91	1867-1979	3-17-73	440.03	60,800	91.2	52,800	

* See appendix A for type of record

(a) No recording gage or datum

(b) Small streams station

(c) Estimate

(d) Observed 100-yr flood peak

(e) COE figures

(f) Choccolocco Creek at Jackson Shoals and at Lincoln Combined

SUPPLEMENTARY DATA IV

Maximum discharge and elevation for urban streams in Alabama

Number	Station Name	Drainage Area (mi ²)	Impervious Area (%)	Datum (ft)	Period of known floods	Maximum Known Flood		
						Elevation (ft)	NGVD (ft ³ /s)	[ft ³ /s] / mi ²) (ft ³ /s)
02361093	Trib to Beaver Cr in Dothan	1.81	30.5	251	1977-81	4-13-78	256.6	631
02416032	Sugar Cr at Alexander City	1.67	20.2	601	1977-80	7-15-78	610.7	1,100
02419975	Three Mile Branch at Biltmore Ave.	in Montgomery	7.30	16.0	197	1974-76	5-09-78	205
02420987	Hannon Slough at Bear Bear School in Montgomery	1.35	42.9	212	1976-76	5-09-78	221.5	1,660
02423580	Shades Creek at Homewood	20.8	16.3	621.07	1966-76	3-19-70	656.39	6,990
02423630	Shades Creek nr Greenwood	72.3	8.3	480.37	1964-81	4-13-79	493.56	10,900
02457000	Fivemile Creek at Ketonaw	23.9	17.0	546.70	1953-79	3-19-70	561.64	4,970
02458200	Village Cr at Apalachee St in Birmingham	15.6	33.3	561.31	1971-77	3-19-70	581.7	4,330
02458300	Village Cr at 24th St in Birmingham	26.0	25.0	537.25	1974-77	4-13-79	554.37	5,400
02458450	Village Cr at Avenue W in Ensley	33.5	25.0	505.16	1975-81	9-07-77	518.42	5,730
02460500	Village Cr nr Adamsville	83.5	18.0	340	1959-81	3-19-70	363.15	19,700
02461299	Valley Cr at Cleburn Ave in Birmingham	20.1	36.8	495.68	1974-77	4-04-77	515.88	4,940
02465286	Cribbs Mill Cr at 2nd Ave in Tuscaloosa	2.75	28.9	183.20	1977-81	4-13-79	186.41	1,270
02471042.15	Woodcock Creek at Airport Blvd. in Mobile	1.85	25.0	14.31	1978-80	4-13-80	16.36	954
02471065	Montlimar Creek at Mobile	8.57	30.4	20.04	1962-81	5-06-81	33.21	5,530
03575686	Aldridge Creek at Dunsmore St. in Huntsville	1.15	10.2	676.87	1971-75	3-16-73	683.87	805
03575696	Aldridge Cr nr Lily Flagg	13.9	8.4	579.80	1971-74	3-16-73	591.73	5,250
03575880	Five Points Ditch at Howe St. in Huntsville	0.62	20.0	624	1971-74	3-16-73	634.14	313
03575890	Pinhook Cr at Clinton Ave. in Huntsville	22.5	12.0	592.58	1967-73	3-16-73	609.08	9,400
03575910	Pinehaven Ditch at Gayhart Drive in Huntsville	0.16	20.0	715	1971-75	3-20-74	725.13	231
03575930	Broglan Branch at Holmes Ave. in Huntsville	8.87	19.3	605	1971-75	3-16-73	617.55	4,240
03575950	Huntsville Spring Branch at Johnson Rd. in Huntsville	41.8	21.4	589.77	1967-74	3-16-73	602.27	11,000
03589450	Sweetwater Cr at Florence	4.92	24.1	420	1977-80	5-08-78	431.6	2,820
								573
								4,520

1,150
9,080
1,210
11,500
368
5,830
263
19,800
573

SUPPLEMENTARY DATA V

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Maximum known flood		
				Date	Elevation (ft) NGVD	Discharge ft ³ /s (ft ³ /s)/mi ²
02343000	Barbour Creek nr Eufaula	98.3	1953-58	Sep. 25, 1956	163.27	6,250 67.0
02343595	Poplar Spring Branch at Dothan	1.00		Aug. 11, 1978	285.02	-- --
--	East Fork Choctawhatchee River nr Pinckarel (sec. 27, T 5N, R 25E)	306		Mar. 1929	190.4	
95	Choctawhatchee River nr Bellwood	1,260		Mar. Jan. 1929, 1925	108 --	26,600 21.1
02361500	Panther Creek nr Hacoda	26.5	1974-81	Apr. 10, 1975	155.3	3,590 135
02364570	Conecuh River at Dozier	578		Mar. 1929	215.7	-- --
02371600	Conecuh River at River Falls	1,260	1929-72	Mar. 1929	173.3	-- --
02372430	Sepulga River nr Cohassett	510		Mar. 1929	172.8	
	Pigeon Creek at Higgins Mill nr Cohassett	344		Mar. 1929	168.3	
02373800	Sepulga River at Brooklyn	1,000		Mar. 1929	138.3	
02398195	Mills Creek nr Chesterfield	9.56	1978-81	Mar. 4, 1979	730.5	1,270 133
	Big Wills Cr 3 mi west of Ft Payne	63.0		Jan. 5, 1949	--	6,600 105

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Date	Maximum known flood		Discharge ft ³ /s (ft ³ /s)/mi ²
					Elevation (ft)	NGVD	
	Little Wills Cr at AGS RR bridge in Ft Payne	3.4		Mar. 3, 1966	---	2,200	647
	Little Wills Cr at Sewage Treatment Plant in Ft Payne	4.2		Mar. 3, 1966	---	2,140	510
	Little Wills Cr at I-65 in Ft Payne	4.9		Mar. 3, 1966	---	3,650	745
	Little Wills Cr 2 mi south of Keener	14.1		Jan. 5, 1949	---	3,200	227
96	02401370 Big Canoe Creek nr Springville	45.0	1978-81	Mar. 17, 1980	612.3	4,130	91.8
	02401460 Gulf Creek nr Ashville	14.2	1978-81	Apr. 13, 1979	571.5	3,060	215
	02401700 Ohatchee Cr at Reads	44.2		Feb. 21, 1961	539.08	--	--
	02401800 Tallahatchee Creek nr Wellington	88.6		Feb. 21, 1961	541.38	--	--
	Choccolocco Creek nr White Plains	104		Aug. 25, 1967	---	27,900	268
	02403200 Choccolocco Creek at Choccolocco	129		Aug. 25, 1967	660.28	32,000	248
	02407470 N. Fk. Yellowleaf Creek at Chelsea	23.2		Feb. 21, 1961	---	7,800	336
	02407510 Muddy Prong Creek at Westover	18.0		Feb. 21, 1961	---	4,870	271
	02412290 Chulafinnee Creek at Hollis	28.5		Aug. 25, 1967	---	13,900	488
	Unnamed Trib. to Ketchepedrake Creek nr Delta	5.45		Aug. 25, 1967	---	4,170	765

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Area (mi ²)	Drainage Period of known floods	Maximum known flood		
				Date	Elevation (ft) NGVD	Discharge ft ³ /s (ft ³ /s)/mi ²
02416483	Sandy Creek at State Hwy 49 nr Dadeville	156	Feb. 24, 1961	--	25,000	160
02420990	Hannon Slough at U.S. Hwy 231 in Montgomery	2.06	Aug. 31, 1961	203.3	2,760	1,640
	Baldwin Slough (Branch of) at U.S. Hwy 231 in Montgomery	1.12	Aug. 31, 1961	210.0	2,000	1,790
02421135	Pintalla Cr at Hwy 31 nr Davenport	189	Feb. 21, 1961	185.4	--	--
02421175	Pintalla Cr at U.S. Hwy 80 nr Monggomery	257	Feb. 25, 1961	164.88	54,300	211
	Big Swamp Cr nr Letohatchee (sec. 11, T 13 N, R 15E)	43.7	Nov. 27, 1948 Feb. 1961	210.0 --	-- 29,500	-- 675
02423130	Cahaba River nr Trussville	19.8	Mar. 19, 1970	--	3,580	181
02423425	Cahaba River nr Cahaba Heights	201	1975-81 Apr. 13, 1979	438.9	23,400	116
02423555	Cahaba River nr Helena	334	Mar. 19, 1970	390.20	21,000	62.9
02423570	Shades Creek at Irondale	7.26	Mar. 19, 1970	706.02	2,200	304
02423571	Shades Cr at Elder St in Springdale	9.46	Mar. 13, 1975	689.7	1,300	137
02423620	Little Shad Creek nr Bessemer	7.55	Mar. 19, 1970	525.48	2,610	346
02427350	Pine Barren Cr at St Hwy 41, 8 mi NE of Camden	345	Nov. 1948	103.2	--	--

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage area known floods (mi ²)	Period of known floods	Date	Maximum known flood		
					Elevation (ft)	NGVD	ft ³ /s (ft ³ /s)/mi ²
02427714	Graham (Mud) Cr at Hwy 5 in Lamison	7.89		Dec. 10, 1961	126.2	6,940	880
02427715	Martin & Goose Creeks 2 mi SW of Lamison	26.0		Dec. 10, 1961	108.3	10,800	415
02427718	Goose Cr 1/2 mi E of Anne Marie Pigeon Cr at Hwy 82 in Gosport	48.6		Dec. 10, 1961	91.6	18,400	379
		22.6		July 8, 1956	--	18,800	832
	Bush Creek at Hwy 82 in Gosport	4.5		July 8, 1956	--	4,050	900
98	Buttahatchee River nr Henson Springs	330		Dec. 19, 1967	--	22,000	66.7
02438550	Luxapallila Creek at Millport	241		Feb. 22-23, 1961	257.86	6,700	27.8
02442500	Lubbub Cr at Hwy 82 nr Reform	63.8		Apr. 13, 1979	--	12,800	201
02444850	Trib. to Lubbub Cr at Hwy 82 1.5 miles W of Reform	1.2		Apr. 13, 1979	--	1,660	1,383
02445050	Little Lubbub Cr nr Carrollton	20.0		Mar. 1951	193.0	--	--
02445107	Little Bear Cr 1.2 mi NE of Gordo	8		Apr. 13, 1979	--	3,760	470
02445327	Boxes Creek nr Howard	1.72	1980-83	Mar. 5, 1983	--	2,350	1,366
02449245	Brush Creek nr Eutaw	42.7	1971-81	Apr. 13, 1979	128.60	8,560	200
02450215	Dorsey Cr below Arkadelphia	26.0	1978-81	Mar. 21, 1980	317.4	3,180	122
02453815	Trinity Creek nr Carbon Hill	2.61	1978-81	Jan. 1, 1979	439.6	705	270

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Date	Discharge	
					Elevation (ft) NGVD	ft ³ /s (ft ³ /s)/mi ²
Maximum known flood						
02454793	Turkey Creek nr Crossston	29.8		Mar. 19, 1970	--	4,300 144
02456330	Crooked Creek at Morris	16.2	1976-81	Apr. 4, 1977	337.1	5,040 311
02456900	Fivemile Creek nr Huffman	9.70		Sep. 7, 1977	679.18	2,675 276
02458150	Village Cr at East Lake in Birmingham	4.82		Mar. 19, 1970	641.10	1,930 400
02461400	Valley Cr at Woodward Iron Co. in Birmingham	32.7		Mar. 19, 1970	477.13	7,330 224
99	02461500 Valley Creek nr Bessemer	51.0		Feb. 1936	457.24	-- --
				Apr. 13, 1979	455.70	11,300 222
02462040	Rock Creek nr Hopkins	30.3		Mar. 19, 1970	--	6,680 220
02462990	Yellow Creek nr Northport	8.23	1976-81	Apr. 12, 1979	381.10	1,180 143
02463900	Bear Creek nr Samantha	15.05	1977-81	Apr. 12, 1979	293.2	8,360 555
02464500	North River nr Tuscaloosa	366		Feb. 22, 1961	188.34	26,200 71.6
	Twomile Cr at Hwy 82 in Northport	--		Apr. 13, 1979	158.29	1,200 --
	Mill Creek trib. #1 at Main St in Northport	--		Apr. 13, 1979	180.18	697 --
	Cribbs Mill Cr at Kicker Rd in Tuscaloosa	--		Apr. 13, 1979	230.77	830 --
	Cribbs Mill Cr at 15th St E in Tuscaloosa	--		Apr. 13, 1979	266.81	580 --

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Date	Maximum known flood		
					Elevation (ft) NGVD	ft ³ /s (ft ³ /s)/mi ²	Discharge
	Trib. #6 Cribbs Mill Cr at I-59 in Tuscaloosa	0.5		Apr. 13, 1979	239.51	455	910
02465291	Cribbs Mill Cr at Kauoosa Ave. in Tuscaloosa	10.7		Apr. 13, 1979	152.79	3,070	287
	Cypress Cr at old Hwy 69 S of Tuscaloosa	--		Apr. 13, 1979	166.53	1,200	
02465400	Big Sandy Creek at Duncanville	56.0		Feb. 1961	182.69	7,000	125
100	Elliot's Creek nr Moundville	31.2		Apr. 13, 1979	--	3,000	96.1
02465900	Big Creek nr Greensboro	125		Mar. 1951	150.0	--	--
02468500	Chickasaw Boque at St. L&S RR bridge nr Linden	268		Mar. 26, 1945	89.1		
02469600	Bashi Creek nr Campbell	86.3		Dec. 10, 1961	--	20,600	239
	Ulkinash Cr, 2 mi NW of Coffeeville	31.1		July 8, 1956	--	25,500	820
	Jackson Creek, 2 mi SW of Winn	42.7		July 8, 1956	--	34,000	796
	Sister Creek at U.S. Hwy 43, 6 mi S of Mt. Vernon	4.11		Apr. 13, 1955	--	4,110	1,000
	Cold Creek at U.S. Hwy 43, 3 mi N of Avis	16.2		Apr. 13, 1955	--	5,160	319
02470675	Mobile River at U.S.I-65 nr Creole	--		Mar. 11, 1961	--	515,000	--
02470805	Bayou Sara at U.S. Hwy 43 at Saraland	23.4		Apr. 13, 1955	--	15,000	641

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Date	Maximum known flood	
					Elevation (ft) NGVD	Discharge ft ³ /s (ft ³ /s)/mi ²
02470810	Norton Cr at U.S. Hwy 43 1/2 mi S of Saraland	4.15		Apr. 13, 1955	--	6,030 1,450
02471010	Mobile River at Hwy 90 in Prichard	44,000		Mar. 10, 1961	--	533,000 12.1
02471041	Eslava Creek at Mobile	1.55		Apr. 13, 1980	21.73	1,510 974
02471075	Halls Mill Creek nr Mobile	27.2		Apr. 13, 1980	13.69	6,100 224
02480010	Big Creek near Mobile	103		Apr. 13, 1955	--	25,800 250
03572940	Black Oak 1 mi SE of Grove Oak	20.4		Dec. 28, 1942	--	5,700 279
101	Morris Branch nr Torey	1.43		Mar. 16, 1973	821.2	840 587
03574842	Straight Ditch at Huntsville	0.17		Mar. 16, 1973	732.63	141 829
03575100	Flint River near Gurley	375		Mar. 12, 1963	626.6	81,100 216
03575200	Hurricane Creek nr Gurley	63.8		Mar. 12, 1963	618.0	16,200 254
03575700	Aldridge Creek nr Farley	14.1		Mar. 12, 1963	590.28	2,200 156
				Apr. 27, 1964	591.13	-- --
03575867	Fagon Cr at Gallaton St in Huntsville	3.90		Mar. 16, 1973	--	3,030 777
03575872	West Fork Pinhook Cr at Blue Springs Rd in Huntsville	2.39		Mar. 16, 1973	--	2,040 854
03575874	Pinhook Cr at Manchester Lake Rd in Huntsville	8.50		Mar. 16, 1973	--	6,930 815

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Maximum known flood	
				Date	Elevation (ft) NGVD
02575875	Normal Branch at U.S. Hwy 72 in Huntsville	4.39	Mar. 16, 1973	--	1,480 337
03575876	Dallas Branch 600 ft above Haynes Ave. in Huntsville	0.47	Mar. 16, 1973	--	680 1,147
03575877	Dallas Branch at Mayessville Rd in Huntsville	2.32	Mar. 16, 1973	--	1,400 603
	Pinhook Cr at Taylor Island in Huntsville	21.4	Dec. 18, 1967	--	5,070 237
102	Pinhook Cr at Southern RR above Holmes Ave. in Huntsville	21.5	Mar. 16, 1973	--	8,170 380
03575879	Dry Cr at Grizzard Rd in Huntsville	2.42	Mar. 16, 1973	--	800 331
	Brogan Branch at SW Seminole Dr. in Huntsville	9.62	Dec. 18, 1967	--	1,870 194
03575940	Huntsville Spring Branch at Drake Ave. in Huntsville	37.3	Mar. 16, 1973	--	10,000 268
03575974	McDonald Cr at Technology Drive	2.05	Mar. 16, 1973	--	600 293
03575976	Sherwood Cr 200 ft below Wynn Dr. in Huntsville	3.01	Mar. 16, 1973	--	804 267
	McDonald Cr at Redstone Arsenal (Gate #8) in Huntsville	8.47	Dec. 18, 1967	--	3,300 390

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Period of known floods	Date	Discharge		Maximum known flood
				Drainage Area (mi ²)	Elevation (ft) NGVD ft ³ /s (ft ³ /s)/mi ²	
03575983	McDonnell Cr above Centour Blvd. in Huntsville	10.1	Mar. 16, 1973	--	3,340	331
03576280	Limestone Cr at St. Hwy 20 nr Mooresville	142	Mar. 16, 1973	--	48,800	344
03576398	Vinson Branch nr Athens	0.91	Mar. 16, 1973	--	840	923
03576403	Johnson Branch nr Athens	5.06	Mar. 16, 1973	--	3,030	599
03576800	Blowing Springs Branch nr Wren	1.26	Apr. 27, 1964	689	219	172
03576810	Elam Creek nr Wren	6.69	Apr. 27, 1964	671	2,500	374
	Rock Cr at Hwy 31 nr Cullman	5.68	Feb. 1961	--	2,100	370
	Unnamed Trib. at Shoal Cr 2-1/2 mi E of Hartselle	0.23	July 23, 1963	--	198	861
	Unnamed Trib. to Shoal Cr at Hartselle	1.03	July 23, 1963	--	1,180	1,150
	Town Branch 400 ft upstream from mouth at Hartselle	1.60	July 23, 1963	--	980	612
03585500	Elk River nr Rogersville	2,239	Mar. 25, 1929	540.7	61,600	27.5
03595639	Simpson Branch 2.4 mi W of Rogersville	3.07	July 8, 1940	--	6,400	2,085
03585900	Second Cr 2-1/2 mi NE of Elgin	46.1	Mar. 21, 1955	--	21,500	466

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Date	Maximum known flood	
					Elevation (ft) NGVD	Discharge ft ³ /s (ft ³ /s)/mi ²
03576983	McDonnell Cr above Centour Blvd. in Huntsville	10.1		Mar. 16, 1973	--	3,340 331
03576280	Limestone Cr at St. Hwy 20 nr Mooresville	142		Mar. 16, 1973	--	48,800 344
03576398	Vinson Branch nr Athens	0.91		Mar. 16, 1973	--	840 923
03576403	Johnson Branch nr Athens	5.06		Mar. 16, 1973	--	3,030 599
03576800	Blowing Springs Branch nr Wren	1.26		Apr. 27, 1964	689	219 172
104	Elam Creek nr Wren	6.69		Apr. 27, 1964	671	2,500 374
	Rock Cr at Hwy 31 nr Cullman	5.68		Feb. 1961	--	2,100 370
	Unnamed Trib. at Shoal Cr 2-1/2 mi E of Hartselle	0.23		July 23, 1963	--	198 861
	Unnamed Trib. to Shoal Cr at Hartselle	1.03		July 23, 1963	--	1,180 1,150
	Town Branch 400 ft upstream from mouth at Hartselle	1.60		July 23, 1963	--	980 612
03585500	Elk River nr Rogersville	2,239		Mar. 25, 1929	540.7	61,600 27.5
03595639	Simpson Branch 2.4 mi W of Rogersville	3.07		July 8, 1940	--	6,400 2,085
03585900	Second Cr 2-1/2 mi NE of Elgin	46.1		Mar. 21, 1955	--	21,500 466

Maximum discharge and elevation for short-term (less than 10 yrs) gaging stations
and miscellaneous sites in Alabama--Continued

Station number	Stream and place of determination	Drainage Area (mi ²)	Period of known floods	Maximum known flood		
				Elevation (ft) NGVD	Date	Discharge ft ³ /s (ft ³ /s)/mi ²
03587000	Big Nance Cr at Red Bank (NE1/4 sec. 28, T 3S, R 8W)	188	1935-40	Apr. 3, 1936	525	4,930 26.2
03589960	Cox Creek nr Florence	15.3		Mar. 21, 1955	--	8,000 523
03590499	Spring Creek at Tuscmibia	83.9		Mar. 1929	--	17,000 203
03591775	Austin Branch nr Bear Creek	2.0		May 26, 1963	760.83	2,080 1,040

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