

CHAPTER 9 .

ORBITAL VELOCITIES IN IRREGULAR WAVES

by

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1 ABSTRACT

Experimental and theoretical study to determine the applicability of linear wave theory for the description of the velocity field in irregular waves. A comparison between theory and measurement was executed both in frequency and in time domain. In frequency domain by means of the experimentally and theoretically determined frequency response functions of wave motion to orbital velocity, and in time domain by means of the measured and computed time records of the velocities. The time records for the velocities were computed from the measured waterlevel fluctuations by using the impuls response function method. The orbital velocities were measured contactless with laser-doppler equipment.

2 INTRODUCTION

In order to determine the applicability of linear wave theory for conversion of irregular wave motion to wave induced orbital velocities near the bottom, the Delft Hydraulics Laboratory has carried out in the past experimental investigation where the velocities were measured by means of a propeller-type flowmeter. The results of this study were used for the computation of wave forces on submarine pipelines. Although in general a reasonable agreement was found between theory and measurement, also significant discrepancies were observed. However, these discrepancies had to be ascribed to the distortion of the flow by the flowmeter.

The development of the laser-doppler velocity meter made it possible to measure accurately the orbital velocities in an extremely small area, without influence of the meter. With this accurate instrument new experiments in irregular waves were carried out, where apart from the bottom velocity also the horizontal and vertical orbital velocities at half water depth and just beneath the deepest trough were measured. The program

comprised waves of moderate steepness at intermediate depth/wavelength ratios ($0.008 < h/g T_p^2 < 0.069$, $0.0009 < H_s/g T_p^2 < 0.0064$). It appeared that the discrepancies mentioned above were indeed due to inaccuracies of the flowmeter and that the agreement with linear wave theory was even better than was found in the first set of experiments.

3 NOTATION

a	distance from the bottom where the velocities were measured
a_{cs}	significant crest of the waves
a_{c1}	crest exceeded by 1% of all the crests of the wave record
a_{ts}	significant trough of the waves
a_{t1}	trough exceeded by 1% of all the troughs of the wave record
g	acceleration of gravity
h	water depth
$H(\omega)$	computed frequency response function
$\hat{H}(\omega)$	"measured" frequency response function
$h(\tau)$	impuls response function
H_s	significant wave height
H_1	wave height exceeded by 1% of all the heights of the wave record
i	$\sqrt{-1}$
k	wave number
$S_{xy}(\omega)$	cross-spectrum
$S_{xx}(\omega)$	auto-spectrum
u_{cs}	significant crest velocity of the horizontal velocity component u
u_{c1}	crest velocity exceeded by 1% of all the crest velocities of the record of the horizontal velocity component u
u_{ts}	significant trough velocity of the horizontal velocity component u
u_{t1}	trough velocity exceeded by 1% of all the trough velocities of the record of the horizontal velocity component u
v_{cs}	significant crest velocity of the vertical velocity component v
v_{c1}	crest velocity exceeded by 1% of all the crest velocities of the record of the vertical velocity component v
v_{ts}	significant trough velocity of the vertical velocity component v
v_{t1}	trough velocity exceeded by 1% of all the trough velocities of the record of the vertical velocity component v
ϵ	relative error

4 THEORETICAL APPROACH

In Appendix A it is shown that according to the linear wave theory the water level fluctuations $\eta(t)$ and the horizontal and vertical orbital velocity components $u(t)$ and $v(t)$ at distance a from the bottom are mutually related by

$$u(t) = \int_0^{\infty} h_u(\tau) \{ \eta(t-\tau) + \eta(t+\tau) \} d\tau \quad (1)$$

$$v(t) = \int_0^{\infty} h_v(\tau) \{ \eta(t-\tau) - \eta(t+\tau) \} d\tau, \quad (2)$$

where $h_u(\tau)$ and $h_v(\tau)$ are impuls response functions. The corresponding frequency response functions are given by

$$H_u(\omega) = \frac{\omega \cosh ka}{\sinh kh} \quad (3)$$

$$H_v(\omega) = i \frac{\omega \sinh ka}{\sinh kh} . \quad (4)$$

The "measured" frequency response functions $\hat{H}_u(\omega)$ and $\hat{H}_v(\omega)$ can be obtained from the measured data by using the relation

$$\hat{H}(\omega) = \frac{S_{xy}(\omega)}{S_{xx}(\omega)}, \quad (5)$$

where $S_{xy}(\omega)$ is the cross-spectrum of input (wave motion) and output (orbital velocity component) and $S_{xx}(\omega)$ is the auto-spectrum of input (see e.g. Bendat and Piersol, 1971).

5 MEASUREMENTS

5.1 Experimental Set-up

The tests were carried out in the 2 meter wide windwave flume of the Delft Hydraulics Laboratory. In this flume irregular waves can be generated up to a height of 30 cm by means of wind and/or a hydraulically driven wave generator. A detailed description of the wave generator is given in (DHL, 1976). For the actual tests no wind was applied. The water level fluctuations were measured by means of a resistance type wave height meter, consisting of two vertical metal rods, and the orbital velocities were measured by means of a laser-doppler equipment which could simultaneously measure horizontal and vertical component. A description of the laser-doppler velocity meter is given in (Godefroy, 1978). In order to measure the undisturbed velocities with this instrument, the walls of the flume had to be translucent for transmitting the laser beams. As this was not the case, the flume was partially provided with an inner flume with perspex walls (see Figure 1). For absorption of the waves, beaches were used with a slope of 1:10 for minimal reflection.

Wave motion and orbital velocities were simultaneously measured in the same cross-section of the flume at a mutual distance of 20 cm. This distance proved to be sufficient for avoiding disturbance of the velocity measurement by the wave height meter. As the waves were long-crested this solution could be used.

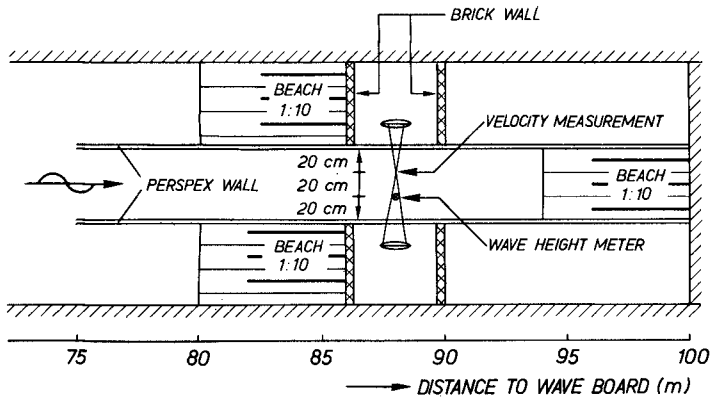


Fig. 1 Lay-out of the 2 m wide wind wave flume

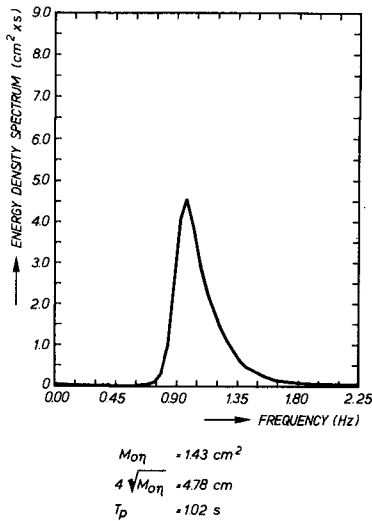


Fig. 2 Wave spectrum at test 16

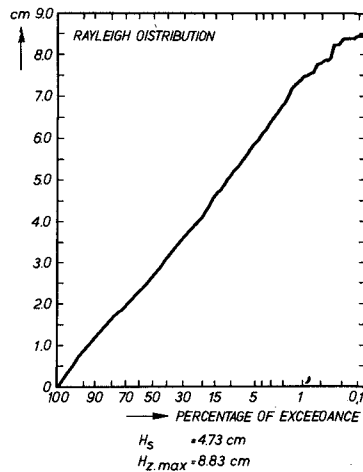


Fig. 3 Wave height distribution at test 16

For elaboration with the computer the measured signals were digitized with sample frequency of 25 Hz.

5.2 Test Program

The test program comprised irregular waves of moderate steepness at intermediate depth/wave length ratios, which are the normal working conditions for offshore structures. The wave parameters are given in Tables I and II. The wave heights, defined as the maximum difference of the record between two succeeding zero-down crossings, appeared to be nearly Rayleigh distributed. As an example the energy density spectrum of a wave record and the corresponding cumulative frequency distribution of the wave heights at test 16 are presented in Figures 2 and 3. Velocities were measured at three locations along the vertical: the horizontal bottom velocity just outside the boundary layer, and the horizontal and vertical components at mid-depth and just beneath the deepest trough (see Tables I and II).

TABLE I TEST PROGRAM FOR
BOTTOM VELOCITIES

TEST	$\frac{1000 h}{g T_p^2}$	$\frac{1000 H_s}{g T_p^2}$	ϵ_u %
1	7.8	0.89	1.4
2	11.8	0.88	5.2
3	19.2	1.75	2.1
4	19.2	2.49	2.3
5	24.4	2.02	1.7
6	24.4	2.81	2.2
7	26.3	3.13	4.8
8	26.3	3.12	2.8
9	33.5	2.69	1.2
10	33.5	3.94	2.6
11	43.6	3.85	5.8
12	43.6	5.62	2.9

TABLE II TEST PROGRAM FOR VELOCITIES
AT DISTANCE a FROM THE BOTTOM

TEST	$\frac{1000 h}{g T_p^2}$	$\frac{1000 H_s}{g T_p^2}$	$\frac{a}{h}$	ϵ_u %	ϵ_v %
13	13.3	1.3	0.500	2.0	4.4
14	13.3	1.3	0.873	2.9	2.1
15	16.3	2.0	0.500	4.7	7.6
16	16.3	2.0	0.825	1.4	3.5
17	17.0	1.5	0.500	3.5	3.1
18	20.6	1.6	0.850	3.0	2.6
19	39.2	4.8	0.500	3.2	7.1
20	39.2	4.6	0.825	1.4	3.4
21	53.9	5.2	0.500	5.2	5.4
22	53.9	5.0	0.873	0.6	2.1
23	68.6	5.2	0.500	4.6	2.8
24	68.6	5.3	0.850	1.0	1.7
25	35.9	6.4	0.810	1.0	1.2

6 COMPARISON OF THEORY AND MEASUREMENT

The comparison of the measured velocities in irregular waves with the linear wave theory has been carried out in frequency domain and, except for the bottom velocities, also in time domain.

In frequency domain theory and measurement were compared by defining a relative error ϵ by

$$\epsilon = \frac{1}{N} \sum_{i=1}^N \left| \frac{|\hat{H}(\omega_i)| - |H(\omega_i)|}{|H(\omega_i)|} \right|,$$

where the "measured" frequency response functions $\hat{H}(\omega_i)$ were determined by (5), and $H(\omega_i)$ by means of (3) or (4). The notation $|\cdot|$ denotes the

amplitude of the (complex) number. The relative errors ϵ have been evaluated only for those frequencies ω_i for which both the spectral density functions of input and output were greater than 10% of their peak value, as for smaller values the spectral densities are relatively inaccurate. All spectra were computed using a Fast Fourier Transform method. The relative errors are given in Tables I and II. The indices u and v have been attached for distinction between horizontal and vertical velocity.

Figure 4 presents most amplitudes of the "measured" and computed frequency response functions.

A comparison in time domain was executed for the velocities above the bottom by computing the time records of the orbital velocity components, for which either (1) or (2) was used, and by comparing them with the measured ones. The significant and the 1% crest and trough velocities are presented in Tables III and IV. Crest and trough velocities are defined in accordance with crests and troughs of the wave motion by the maximum positive and maximum negative velocity between two succeeding zero-down crossings. A positive velocity means for the horizontal velocity a velocity in direction of propagation of the waves, and for the vertical velocity a velocity in upwards direction.

An example of measured and computed time record is presented in Figure 5 and in more detail in Figure 6.

TABLE III WAVE MOTION AND MEASURED AND COMPUTED HORIZONTAL ORBITAL VELOCITY

TEST	WAVE MOTION MEASURED				HORIZONTAL ORBITAL VELOCITY							
	a_{cs} cm	a_{cl} cm	a_{ts} cm	a_{tl} cm	MEASURED				COMPUTED			
					u_{cs} cm/s	u_{cl} cm/s	u_{ts} cm/s	u_{tl} cm/s	u_{cs} cm/s	u_{cl} cm/s	u_{ts} cm/s	u_{tl} cm/s
13	2.5	3.9	2.4	3.5	9.0	12.6	9.4	14.2	9.0	12.7	9.9	14.9
14	2.4	3.6	2.7	3.5	11.7	16.5	12.0	18.3	11.6	16.8	12.2	18.4
15	2.6	3.9	2.4	3.6	7.6	11.5	7.7	13.1	6.8	10.6	8.1	13.0
16	2.5	4.0	2.3	3.6	12.0	17.8	12.4	20.8	12.0	18.6	12.2	18.6
17	2.8	4.0	2.6	4.0	9.6	13.2	9.5	16.0	9.5	13.3	9.6	16.1
18	2.8	3.8	2.7	4.1	12.6	17.0	11.5	17.2	12.6	17.0	12.0	17.2
19	2.9	4.8	2.5	4.0	5.9	9.1	5.9	10.0	5.6	9.4	5.9	9.3
20	2.7	4.5	2.5	3.8	12.0	19.1	12.9	20.4	12.7	21.0	12.3	18.8
21	3.5	5.2	3.2	4.6	10.1	14.6	10.1	16.2	9.6	14.9	9.8	14.5
22	3.0	4.5	2.8	4.0	10.8	16.1	11.5	16.9	11.8	17.4	11.0	16.0
23	2.7	4.6	2.7	4.1	4.2	6.1	4.1	6.8	4.1	6.5	4.3	6.7
24	2.9	4.5	2.6	3.8	10.6	15.3	11.8	18.4	11.2	17.0	11.1	16.4
25	7.3	13.1	6.2	9.6	22.0	34.0	25.1	41.2	23.1	36.2	24.6	40.5

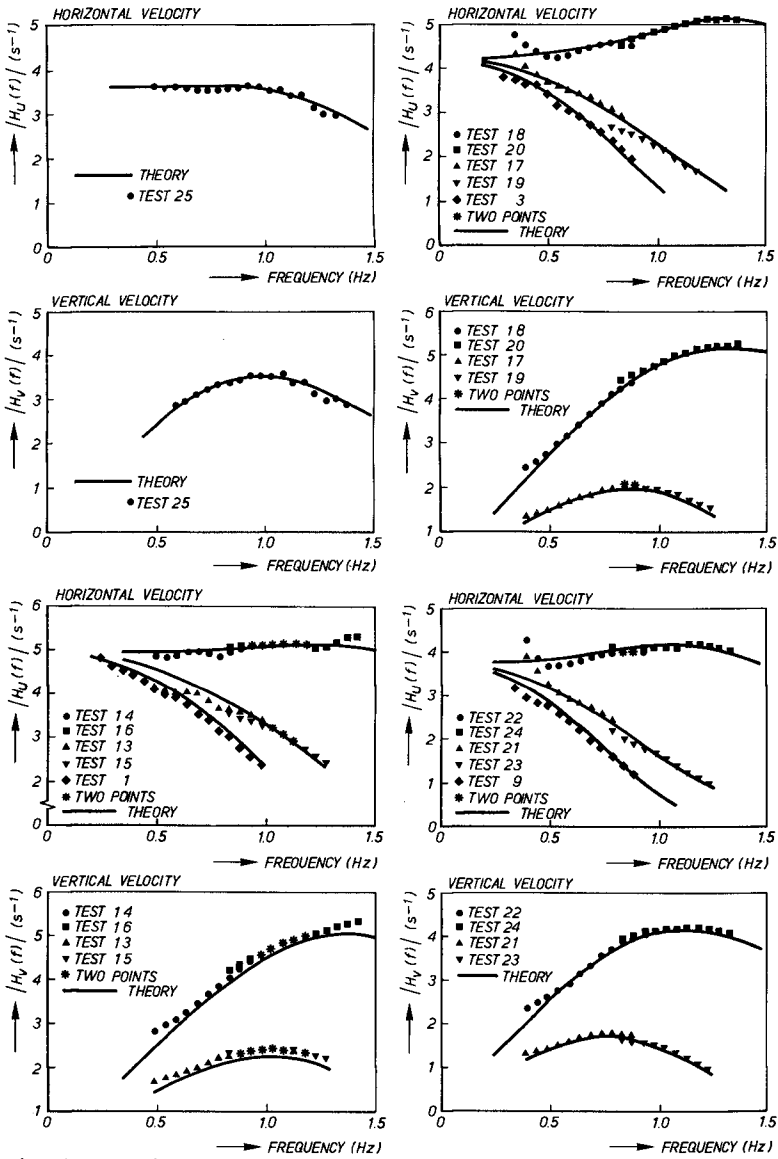


Fig. 4 Comparison of measured and computed frequency response function

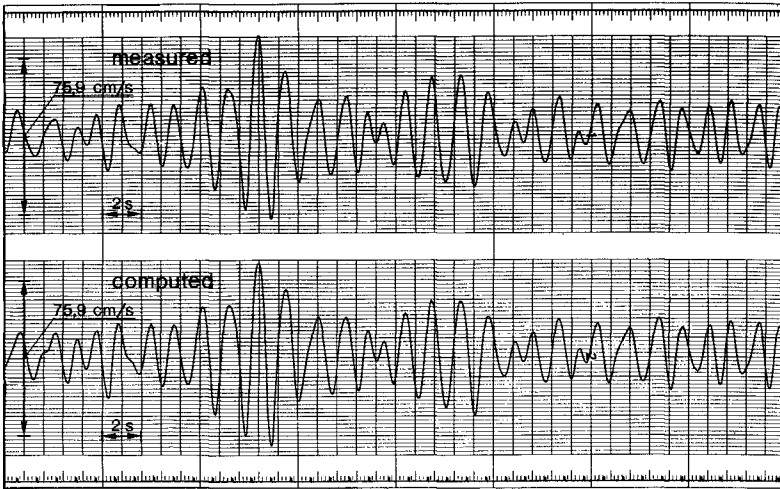


Fig. 5 Time records of measured and computed horizontal orbital velocity component at test 25

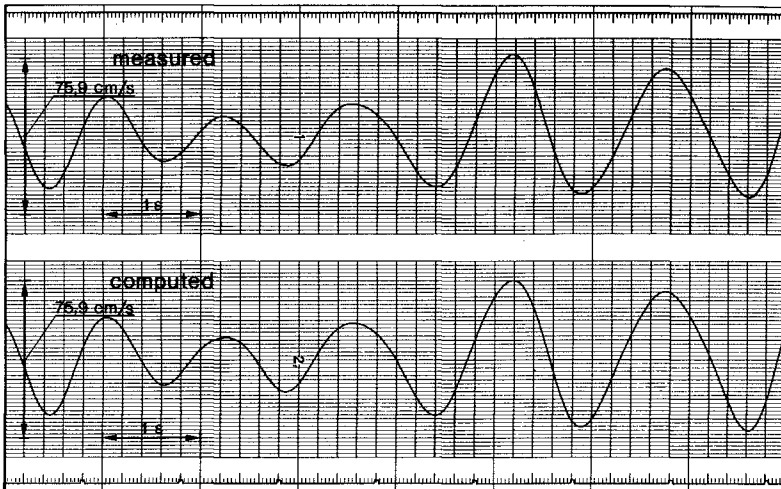


Fig. 6 Time records of measured and computed horizontal orbital velocity component at test 25 in more detail

TABLE IV MEASURED AND COMPUTED VERTICAL ORBITAL VELOCITY

TEST	VERTICAL ORBITAL VELOCITY							
	MEASURED				COMPUTED			
	v_{cs} cm/s	v_{cl} cm/s	v_{ts} cm/s	v_{tl} cm/s	v_{cs} cm/s	v_{cl} cm/s	v_{ts} cm/s	v_{tl} cm/s
13	5.2	7.8	5.2	7.5	4.9	7.0	4.8	7.0
14	9.2	14.2	9.0	13.3	9.0	13.2	8.7	12.9
15	5.7	9.0	5.8	8.9	5.3	8.3	5.3	8.1
16	11.2	18.3	11.4	17.8	11.0	17.7	11.1	17.9
17	4.4	6.3	4.6	6.6	4.2	6.1	4.3	6.4
18	8.7	12.9	8.4	12.6	8.6	12.6	8.8	12.4
19	4.7	8.0	4.9	7.9	4.6	7.5	4.6	7.3
20	12.2	19.9	12.3	19.4	12.1	19.9	12.1	19.2
21	4.9	7.6	5.6	7.8	5.0	7.6	5.0	7.3
22	9.1	14.5	9.0	13.9	9.0	14.3	8.9	13.4
23	3.7	5.7	3.7	5.8	3.6	5.7	3.6	5.8
24	11.0	16.4	11.0	17.0	10.8	16.5	10.8	16.8
25	21.0	34.7	20.9	32.8	20.8	35.6	21.2	33.0

7 DISCUSSION OF THE RESULTS

It is remarkable to see from the comparison in frequency domain that the velocities near the free surface match linear wave theory better than the velocities at mid-depth, whereas in view of test results in regular waves (see e.g. Le Méhauté, Divoky and Lin, 1968) just the opposite would be expected. This phenomenon was observed for both the orbital velocity components. The bottom velocities were indeed in more agreement with linear wave theory than the velocities at mid-depth. However, from the comparison in time domain, as far as executed, these tendencies could not be observed.

From Table III it appears that for the horizontal velocities the trough velocities are greater in magnitude than the crest velocities, contrary to the wave motion where the crests are higher than the troughs. The vertical velocities appeared to be distributed more symmetrically. It is important to note that these tendencies are followed by linear wave theory in most cases. However, when a higher order wave theory, such as the fifth order Stokes' method of De (1954), would be used for description of the velocity field in the individual waves of the irregular sea (the so-called deterministic approach) just the opposite would be found, apart from the fact that with respect to the crests of the waves only a "symmetrical" velocity distribution would be found. Also when linear wave theory is used in the random phase model, these tendencies will not be found, just as a consequence of the random phases by which equal crest and trough velocities will be found.

8 CONCLUSIONS AND RECOMMENDATIONS

In the present study low to moderately steep irregular waves have been

investigated, and it appeared that linear wave theory gives a reasonable description of the velocity field, in particular for the velocities near the free surface just beneath the deepest trough.

In view of these results it is advised to carry out further investigation of the applicability of linear wave theory to more severe wave conditions and to the velocities in the crests of the waves.

It will also be interesting to check linear wave theory for a spatial description of the velocity field. As for long-crested waves the correlation between water level fluctuations and orbital velocities in the same vertical has now been shown, this investigation can then be restricted to a check of the correlation in space of the wave motion at different locations. This was already investigated by Lundgren and Sand (1978), who found that the dispersion of the waves over limited distance is well described by linear wave theory, but that deviations increase with increasing distance. For directional seas research of the velocity field is necessary.

9 REFERENCES

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APPENDIX A. THEORY OF LINEAR SYSTEMS

Suppose that $x(t)$ is the input and $y(t)$ the output or response of a linear system and that $X(\omega)$ and $Y(\omega)$ are the corresponding Fourier transforms. Put $H(\omega)$ equal to the ratio $Y(\omega)/X(\omega)$; thus

$$Y(\omega) = H(\omega) \cdot X(\omega) \quad (\text{A.1})$$

Inverse Fouriertransform of this relation yields the well-known convolution expression

$$y(t) = \int_{-\infty}^{+\infty} h(\tau) x(t-\tau) d\tau, \quad (\text{A.2})$$

where $h(t)$ is the inverse Fouriertransform of $H(\omega)$, given by

$$h(t) = \frac{1}{2\pi} \int_{-\infty}^{+\infty} H(\omega) e^{i\omega t} d\omega \quad (\text{A.3})$$

Often $h(t)$ is denoted by impuls response function, as

$$h(t) = \int_{-\infty}^{+\infty} h(\tau) \delta(t-\tau) d\tau,$$

and $H(\omega)$ by frequency response function.

Expression (A.1) yields the relation between input and output of the linear system in frequency domain and (A.2) in time domain. Thus if the frequency response function $H(\omega)$ of the linear system is known, then the response $y(t)$ on any arbitrary input can be computed, either by using (A.3) followed by (A.2), or by using (A.1), in which case first the input has to be Fouriertransformed, resulting in $X(\omega)$, and the output $Y(\omega)$ has to be Fourier inverted in order to obtain $y(t)$.

The frequency response function between wave motion and orbital velocity component at distance a from the bottom (both in the same cross-section of the flume) can be found with linear wave theory by putting as input of the linear system the wave motion in frequency domain

$$X(\omega) = e^{i\omega t} = \cos\omega t + i \sin\omega t. \quad (\text{A.4})$$

(Note that $e^{i\omega t}$ is the Fouriertransform of the impulse on time $-t$; t has to be considered as a parameter.)

According to linear wave theory the response $Y(\omega)$ is then given by

$$Y(\omega) = \frac{\omega \cosh ka}{\sinh kh} e^{i\omega t} \quad (\text{A.5})$$

for the horizontal orbital velocity component, which is defined positive in wave direction, and by

$$Y(\omega) = i \frac{\omega \sinh ka}{\sinh kh} e^{i\omega t} \quad (\text{A.6})$$

for the vertical orbital velocity component, which is defined positive in upwards direction.

Then by using (A.1) the frequency response functions can be determined to be

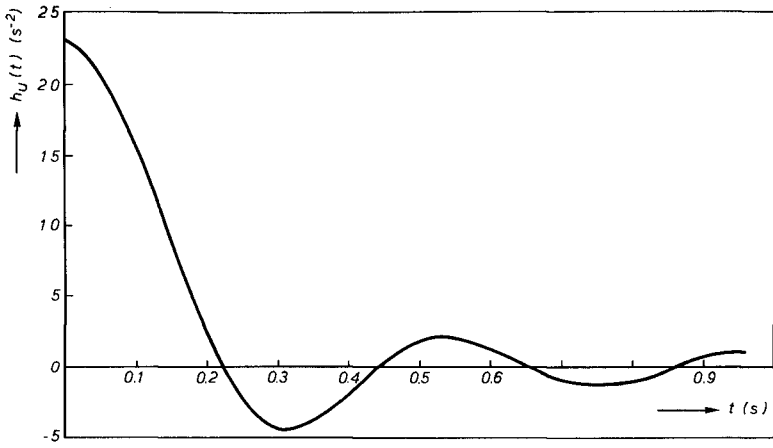


Fig. 7 Impuls response function for the horizontal orbital velocity component at test 14 and 16.
 $\omega_0 = 15 \text{ rad/s}$, $t_0 = 4.0 \text{ s}$

$$H_u(\omega) = \frac{\omega \cosh ka}{\sinh kh} \tag{A.7}$$

$$H_v(\omega) = i \frac{\omega \sinh ka}{\sinh kh}, \tag{A.8}$$

where indices u and v have been attached for distinction. With the aid of (A.3) it follows that

$$h_u(t) = \frac{1}{\pi} \int_0^\infty \left\{ \frac{\omega \cosh ka}{\sinh kh} \cos \omega t \right\} d\omega, \tag{A.9}$$

$$h_v(t) = \frac{1}{\pi} \int_0^\infty \left\{ \frac{\omega \sinh ka}{\sinh kh} \sin \omega t \right\} d\omega, \tag{A.10}$$

where also use was made of the property that the impuls response of a physical system is real. From these expressions it can be seen that $h_u(t)$ is an even and $h_v(t)$ an odd function of t . With these properties it follows from (A.2) that the horizontal and vertical orbital velocity components u and v at distance a from the bottom are respectively given by

$$u(t) = \int_0^\infty h_u(\tau) \{ \eta(t-\tau) + \eta(t+\tau) \} d\tau \tag{A.11}$$

$$v(t) = \int_0^\infty h_v(\tau) \{ \eta(t-\tau) - \eta(t+\tau) \} d\tau \tag{A.12}$$

It is remarkable to see from these relations that the response on time t depends on the wave motion $\eta(t)$ before and after t . Thus, not only the past but also the future of $\eta(t)$ determines the velocities at time t . Physically seen this is rather strange. However, it has to be realized that these relations are based on linear wave theory where it is assumed that the sinusoidal wave motion is present from $t = -\infty$ to $t = +\infty$. In view of this basic assumption, the results (A.11) and (A.12) could be expected.

In order to achieve existing integrals in (A.9) and (A.10), the constituent functions $H_u(\omega)$ and $H_v(\omega)$ have been put equal to zero for $\omega > \omega_0$, which implies that the response on input in this frequency range is zero. However, this interference is no limitation if ω_0 is chosen such that for higher frequencies the input signal does not contain energy.

For numerical computation of the integrals in (A.11) and (A.12), the functions $h_u(t)$ and $h_v(t)$, which approach to zero for $t \rightarrow \infty$, have been put equal to zero for $t > t_0$, where t_0 is chosen such that for $t > t_0$

$$|h(t)| < \epsilon \cdot \max_p |h(p)|$$

(e.g. is 0.01). An example of $h_u(t)$ is presented in Figure 7.