

## Primary resonance of traveling viscoelastic beam under internal resonance\*

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**Abstract** Under the 3:1 internal resonance condition, the steady-state periodic response of the forced vibration of a traveling viscoelastic beam is studied. The viscoelastic behaviors of the traveling beam are described by the standard linear solid model, and the material time derivative is adopted in the viscoelastic constitutive relation. The direct multi-scale method is used to derive the relationships between the excitation frequency and the response amplitudes. For the first time, the real modal functions are employed to analytically investigate the periodic response of the axially traveling beam. The undetermined coefficient method is used to approximately establish the real modal functions. The approximate analytical results are confirmed by the Galerkin truncation. Numerical examples are presented to highlight the effects of the viscoelastic behaviors on the steady-state periodic responses. To illustrate the effect of the internal resonance, the energy transfer between the internal resonance modes and the saturation-like phenomena in the steady-state responses is presented.

**Key words** traveling beam, nonlinear vibration, viscoelasticity, primary resonance, internal resonance

**Chinese Library Classification** O242

**2010 Mathematics Subject Classification** 74S05

## 1 Introduction

This paper presents a profound study for the nonlinear forced vibration of an axially traveling beam with 3:1 internal resonance. The main target of the present paper is to investigate the effects of the viscoelastic properties and the internal resonance on the steady-state response of the traveling beam. Axially traveling beams play an important role in the design of modern machinery. Therefore, the linear and nonlinear dynamics of the axially traveling beams have drawn many researchers' attention<sup>[1–3]</sup>. Marynowski and Kapitaniak<sup>[4]</sup> have reviewed the research progress of the dynamics of the axially traveling continua in the past sixty years.

Traditionally, the investigations were focused on the dynamics of the traveling elastic beams. Very recently, researchers have paid their attention on the viscoelastic behaviors of the traveling beam. Yao and Zhang<sup>[5]</sup> established the equations of motion for the viscoelastic moving belt

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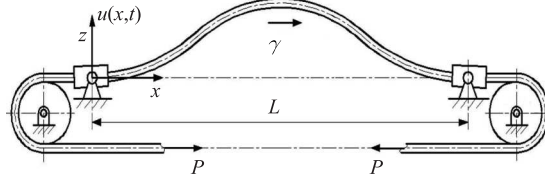
by use of the Kelvin-type viscoelastic constitutive law. Ozhan and Pakdemirli<sup>[6-7]</sup> and Yang and Zhang<sup>[8]</sup> adopted the Kelvin model containing the partial time derivative for describing the viscoelastic behavior of beam materials. To study the nonlinear vibration of the traveling viscoelastic beams, Ding and Zu<sup>[9]</sup>, Yan et al.<sup>[10]</sup>, and Tang et al.<sup>[11]</sup> proved that the material time derivative should be contained in the Kelvin model. The standard linear solid model has been employed in modelling axially traveling viscoelastic beams. Wang et al.<sup>[12]</sup> investigated the effect of an arbitrary varying length on the nonlinear free vibration of an axially translating viscoelastic beam. Chen and his co-workers worked on the steady-state responses of the axially traveling strings<sup>[13]</sup> and beams<sup>[14]</sup> with the standard linear solid model. Saksa and Jeronen<sup>[15]</sup> studied the characteristic behaviors of an axially traveling viscoelastic beam by modelling the viscoelasticity with the Poynting Thomson version of the standard linear solid. In the present paper, the effect of the standard linear solid model on the nonlinear dynamics of the traveling beams with internal resonance is firstly investigated.

Generally, internal resonance can complicate the dynamics of the nonlinear system. Therefore, the nonlinear vibration of the traveling beam under internal resonance has been widely concerned. With the Galerkin method, Ghayesh and Amabili<sup>[16]</sup> tuned the first two transverse modes to a 3:1 internal resonance, and studied the coupled longitudinal and transverse vibration of an axially traveling beam. By combining the incremental harmonic balance method with the Galerkin method, Chen and his co-workers discovered the planar motion<sup>[17]</sup> and the nonlinear vibration to periodic lateral force excitations<sup>[18]</sup> for an axially traveling beam. Wang et al.<sup>[19]</sup> considered the multimode dynamics of the inextensional beams on the elastic foundation with 2:1 internal resonance based on a similar process. Since the governing equations are discretized into ordinary differential equations, the research techniques in the above-mentioned literatures are called as the indirect perturbation method. The nonlinear dynamics of the traveling beams have also been studied by the direct perturbation method. By use of the direct multi-scale method, Sahoo et al.<sup>[20]</sup> focused on the nonlinear transverse vibration of an axially moving beam subjected to two frequency excitations in the presence of internal resonance. By use of the direct multi-scale method to establish the solvability conditions, Tang et al.<sup>[11]</sup> investigated the parametric and 3:1 internal resonance of the axially moving viscoelastic beams on the elastic foundation. Without internal resonance, the multi-scale method has been directly used to solve the nonlinear dynamics of the traveling beams. Forced vibrations are studied for traveling beams via the direct multi-scale analysis<sup>[21]</sup>. Liu et al.<sup>[22]</sup> derived the analytical expression of the first-order uniform expansion of the solution by directly using the multi-scale method. One thing should be mentioned, by use of the direct multi-scale method to study the nonlinear dynamics of traveling beams, all the above-mentioned studies establish the solvability conditions with complex modal functions. The reason is that it is difficult to obtain the real modal functions for the derived equation of the nonlinear continuous Gyro system.

In the present work, the forced periodic response of an axially traveling viscoelastic beam is investigated in the presence of 3:1 internal resonance. The standard linear solid model and the material time derivative are used to model the transverse vibration of the axially traveling beam. The effects of the viscoelastic property on the periodic response are determined. The approximate real modal functions are established based on the corresponding linear derived equation. Therefore, the direct multi-scale method is firstly used to establish the solvable conditions with the real modal functions. The approximate analytical stable steady-state response is confirmed by the Galerkin method.

## 2 Mathematical model

Consider a slender axially traveling beam with simply supported boundary conditions (see Fig. 1). The traveling beam is modelled as an Euler-Bernoulli viscoelastic beam. The transverse vibration of the traveling beam with the external harmonic excitation is governed



**Fig. 1** Traveling beam with simply supported ends

by<sup>[23]</sup>

$$\rho A(u_{,tt} + \dot{\gamma}u_{,x} + 2\gamma u_{,xt} + \gamma^2 u_{,xx}) + M_{,xx} = ((P + A\sigma)u_{,x})_{,x} + F, \quad (1)$$

where  $t$  is the time, and  $x$  is the neutral axis coordinate along the traveling beam. A comma preceding  $t$  or  $x$  denotes partial derivatives.  $u(x, t)$  presents the transverse displacement. Moreover,  $L$  and  $A$  are, respectively, the length and the cross-section area of the beam.  $F = b \sin(\omega t)$  denotes the transverse load, where  $b$  and  $\omega$  are the excitation amplitude and the frequency, respectively.  $P$  is the initial axial load.  $\sigma(x, t)$  presents the disturbed axial stress. For a slender traveling beam, the bending moment  $M(x, t)$  is expressed as follows:

$$M(x, t) = - \int_A z \sigma(x, t) dA. \quad (2)$$

The standard linear solid model with the material time derivative is used to characterize the viscoelastic property of the traveling beam. Therefore, the stress-strain relation is expressed in a differential form as follows:

$$(E_1 + E_2)\sigma + \alpha \frac{d\sigma}{dt} = E_1 E_2 \varepsilon_L + E_1 \alpha \frac{d\varepsilon_L}{dt}, \quad (3)$$

where  $E_1$  and  $E_2$  are the stiffness constants,  $\sigma$  denotes the normal stress due to that bending  $\alpha$  represents the dynamic viscosity, and  $\varepsilon_L(x, t)$  is the Lagrangian axial bending strain expressed by

$$\varepsilon_L(x, t) = \frac{1}{2} u^2_{,x}. \quad (4)$$

The total time derivative is given by

$$\frac{d}{dt} = \frac{\partial}{\partial t} + \gamma \frac{\partial}{\partial x}. \quad (5)$$

Therefore, substituting Eqs. (4) and (5) into Eq. (3) yields

$$(E_1 + E_2)\sigma + \alpha \sigma_{,t} + \alpha \gamma \sigma_{,x} = \frac{1}{2} (E_1 E_2 (u^2_{,x}) + E_1 \alpha (u^2_{,x})_{,t} + E_1 \alpha \gamma (u^2_{,x})_{,x}). \quad (6)$$

In this paper, only small deflections are considered. Therefore, the displacement-strain relationship can be written as follows:

$$\varepsilon_S(x, z, t) = -z u_{,xx}. \quad (7)$$

Therefore, for the standard linear solid, the viscoelastic material of the beam has the following constitution relationship:

$$(E_1 + E_2)M + \alpha M_{,t} + \alpha \gamma M_{,x} = (E_1 E_2 u_{,xx} + E_1 \alpha u_{,xxt} + E_1 \alpha \gamma u_{,xxx}) \int_A z^2 dA. \quad (8)$$

Due to bending and the bending moment, the normal stresses are, respectively, derived from Eqs. (6) and (8). In the following investigation, the simply supported boundary conditions of the traveling beam are considered as follows:

$$u(0, t) = u(L, t) = 0, \quad u_{,xx}(0, t) = u_{,xx}(L, t) = 0. \quad (9)$$

Incorporate the following dimensionless variables and parameters:

$$\left\{ \begin{array}{l} u \leftrightarrow \frac{u}{L}, \quad x \leftrightarrow \frac{x}{L}, \quad t \leftrightarrow t\sqrt{\frac{P}{\rho AL^2}}, \\ \gamma \leftrightarrow \gamma\sqrt{\frac{\rho A}{P}}, \quad \alpha \leftrightarrow \frac{\alpha}{E_1 + E_2}\sqrt{\frac{P}{\rho AL^2}}, \quad \omega \leftrightarrow \omega\sqrt{\frac{\rho AL^2}{P}}, \\ b \leftrightarrow \frac{bL}{P}, \quad I = \int_A z^2 dA, \quad E_a = \frac{IE_1E_2}{PL^2(E_1 + E_2)}, \\ E_b = \frac{IE_1}{PL^2}, \quad E_c = \frac{E_1E_2A}{P(E_1 + E_2)}, \quad E_d = \frac{E_1A}{P}, \end{array} \right. \quad (10)$$

where  $I$  denotes the area moment of inertial,  $\alpha$  denotes the dimensionless viscous coefficient,  $E_a$  and  $E_b$  are, respectively, represent the effects of the flexural stiffness of the traveling beam, and  $E_c$  and  $E_d$  are the nonlinear coefficients. The governing equation of the transverse vibration of the traveling beam and the boundary conditions are nondimensionalized as follows:

$$\begin{aligned} & u_{,tt} + 2\gamma u_{,xt} + (\gamma^2 - 1)u_{,xx} + E_a u_{,xxxx} + \alpha(E_b - E_a)(u_{,xxxxt} + \gamma u_{,xxxxx}) \\ &= \frac{3}{2}E_c u_{,x}^2 u_{,xx} - \alpha u_{,x}(E_c - E_d)(2u_{,xx}(u_{,xt} + \gamma u_{,xxx}) \\ &+ u_{,x}(u_{,xxt} + \gamma u_{,xxx})) + b \sin(\omega t), \end{aligned} \quad (11)$$

$$u(0, t) = u(1, t) = 0, \quad u_{,xx}(0, t) = u_{,xx}(1, t) = 0. \quad (12)$$

### 3 Natural frequencies and internal resonance condition

To construct the condition of the internal resonance of the traveling beam, the corresponding linear equation can be adopted to abstract the natural frequencies of the traveling beam. Omitting the nonlinear terms and viscosity terms of Eq. (11) yields

$$u_{,tt} + 2\gamma u_{,xt} + (\gamma^2 - 1)u_{,xx} + E_a u_{,xxxx} = 0. \quad (13)$$

The solution to the linear derived equation (13) can be expressed as follows:

$$u(x, t) = \sum_{n=1}^{+\infty} \Phi_n(x) e^{i\omega_n t} + \text{c.c.}, \quad n = 1, 2, \dots, \quad (14)$$

where c.c. represents the complex conjugate of all preceding terms on the right-hand side of the equation, and the modal function is assumed as follows:

$$\Phi_n(x) = \sum_{m=1}^{+\infty} c_{n,m} \sin(m\pi x), \quad \overline{\Phi_n}(x) = \sum_{m=1}^{+\infty} \overline{c_{n,m}} \sin(m\pi x). \quad (15)$$

Substitute Eqs. (14) and (15) into Eq. (13). Then, multiplying both sides of the obtained equation by  $\sin(m\pi x)$  ( $m = 1, 2, \dots$ ) and integrating it with respect to  $x$  from 0 to 1 yield a set of second-order ordinary differential equations. The non-triviality of the solutions to the set of the ordinary differential equations requires its determinant of coefficients to be zero. Therefore, the following equation is obtained:

$$|-\omega_n^2 \mathbf{M} + i\omega_n \mathbf{Z} + \mathbf{K}| = 0, \quad (16)$$

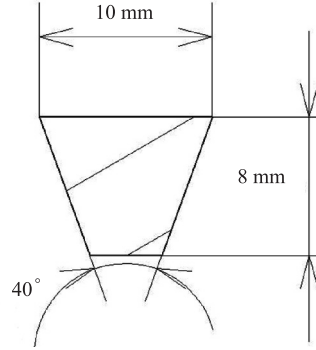
where  $\mathbf{M}$  represents the identity matrix, and  $\mathbf{Z}$  and  $\mathbf{K}$  are, respectively, the Coriolis acceleration matrix and the stiffness matrix defined by

$$Z_{ij} = 4\gamma \begin{cases} \frac{ij(1 - (-1)^{i+j})}{i^2 - j^2}, & i \neq j, \\ 0, & i = j, \end{cases} \quad (17)$$

$$K_{ij} = \begin{cases} i^4 \pi^4 E_a + i^2 \pi^2 (1 - \gamma^2), & i = j, \\ 0, & i \neq j. \end{cases} \quad (18)$$

The natural frequencies  $\omega_n$  can be obtained from Eq. (16).

In the following study, a V-belt is adopted as the prototype of the traveling beam. Figure 2 shows the shape of the cross-sectional area of the V-belt. Table 1 presents the physical parameters of the V-belt.



**Fig. 2** Shape of cross-sectional area of V-belt

**Table 1** Physical parameters of V-belt transmission

Item	Notation	Value
Cross-section area	$A$	$5.671 \times 10^{-5} \text{ m}^2$
Area moment of inertial	$I$	$2.775 \times 10^{-9} \text{ m}^4$
Initial axial tension	$P$	20 N
Length of belt	$L$	0.35 m
Modulus of elasticity	$E_1$	200 MPa
Creep modulus of elasticity	$E_2$	200 MPa

Based on Table 1, the dimensionless parameters can be determined based on Eq. (10). Therefore,

$$E_b = 2E_a = 0.058, \quad E_d = 2E_c = 567.2, \quad b = 0.003, \quad \alpha = 0.005.$$

If there is no special designation, the traveling beam can be considered with these parameter values in all the following examples. The first four natural frequencies are calculated and shown

in Table 2 with the dimensionless axial speed  $\gamma = 0.59829$ . As shown in Table 2, the internal resonance condition  $\omega_2 : \omega_1 = 3$  is satisfied.

**Table 2** First four natural frequencies of derived linear equation

Item	Notation	Value
First natural frequency	$\omega_1$	2.797 92
Second natural frequency	$\omega_2$	8.393 77
Third natural frequency	$\omega_3$	17.123 50
Fourth natural frequency	$\omega_4$	30.381 60

#### 4 Scheme of approximate analytical solution

In this section, the multi-scale method is used to solve the steady-state periodic responses of the axially traveling beam. In this work, only weak external excitation is considered. A non-dimensional bookkeeping parameter is introduced to distinguish the different orders of magnitude. Therefore, the following dimensionless variable and parameters are scaled:

$$b \leftrightarrow \varepsilon^3 b, \quad \alpha \leftrightarrow \varepsilon^2 \alpha, \quad u(x, t) \leftrightarrow \varepsilon u(x, t). \quad (19)$$

Then, the perturbation solution can be written as follows:

$$u(x, T_0, T_2) = u_0(x, T_0, T_2) + \varepsilon^2 u_2(x, T_0, T_2), \quad (20)$$

where  $T_0 = t$ ,  $T_2 = \varepsilon^2 t$ , and the following relationships are satisfied:

$$\frac{\partial}{\partial t} = \frac{\partial}{\partial T_0} + \varepsilon^2 \frac{\partial}{\partial T_2}, \quad \frac{\partial^2}{\partial t^2} = \frac{\partial^2}{\partial T_0^2} + 2\varepsilon^2 \frac{\partial^2}{\partial T_0 \partial T_2} + \varepsilon^4 \frac{\partial^2}{\partial T_2^2}. \quad (21)$$

Substituting Eqs. (19), (20), and (21) into Eq. (11) and equalizing the coefficients of  $\varepsilon^0$  and  $\varepsilon^2$  in the obtained equations yield

$$u_{0,T_0 T_0} + E_a u_{0,xxxx} + 2\gamma u_{0,xT_0} + u_{0,xx} \gamma^2 - u_{0,xx} = 0, \quad (22)$$

$$2u_{0,T_0 T_2} + u_{2,T_0 T_0} + 2\gamma u_{2,xT_0} + 2\gamma u_{0,xT_2} + \gamma^2 u_{2,xx} + \alpha \gamma (E_b - E_a) u_{0,xxxx} \\ - 3E_c u_{0,x}^2 u_{0,xx} / 2 + \alpha (E_b - E_a) u_{0,xxxx T_0} + E_a u_{2,xxxx} - u_{2,xx} + b \sin(\omega t) = 0. \quad (23)$$

The solution of Eq. (22) can be written as follows:

$$u_0(x, T_0, T_2) = A_1(T_2) \Theta_1(x) e^{i\omega_1 T_0} + A_2(T_2) \Theta_2(x) e^{i\omega_2 T_0} + \text{c.c.}, \quad (24)$$

where  $A_1(T_2)$  and  $A_2(T_2)$  are undetermined functions, c.c. represents the complex conjugate of the two preceding terms on the right hand of the equation, and

$$\begin{cases} \Theta_k(x) = p_{k,1} \sin(\pi x) + p_{k,2} \sin(2\pi x) + p_{k,3} \sin(3\pi x) + p_{k,4} \sin(4\pi x), \\ \overline{\Theta}_k(x) = \overline{p_{k,1}} \sin(\pi x) + \overline{p_{k,2}} \sin(2\pi x) + \overline{p_{k,3}} \sin(3\pi x) + \overline{p_{k,4}} \sin(4\pi x), \end{cases} \quad (25)$$

where  $k = 1, 2$ . Substitute Eqs. (25) and (24) into Eq. (22). Then,  $p_{k,r}$  and  $\overline{p_{k,r}}$  ( $k = 1, 2$  and  $r = 1, 2, 3, 4$ ) can be solved by the undetermined coefficient method, and the solution of Eq. (23) can be written as follows:

$$u_2(x, T_0, T_2) = Q_1(x, T_2) e^{i\omega_1 T_0} + Q_2(x, T_2) e^{i\omega_2 T_0}, \quad (26)$$

where

$$Q_k(x, T_2) = q_{k,1}(T_2) \sin(\pi x) + q_{k,2}(T_2) \sin(2\pi x) + q_{k,3}(T_2) \sin(3\pi x) + q_{k,4}(T_2) \sin(4\pi x). \quad (27)$$

Introduce the detuning parameters  $\sigma_1$  and  $\sigma_2$ . Then, the nearnesses of  $\omega$  to  $\omega_1$  and  $\omega_2$  to  $3\omega_1$  can be, respectively, represented by

$$\omega_2 = 3\omega_1 + \varepsilon^2 \sigma_1, \quad \omega = \omega_1 + \varepsilon^2 \sigma_2. \quad (28)$$

Substituting Eqs. (26) and (28) into Eq. (23) yields

$$\begin{aligned} & 2(i\omega_1 A_{1,T_2} \Theta_1 e^{i\omega_1 T_0} + i\omega_2 A_{2,T_2} \Theta_2 e^{i\omega_2 T_0} + \text{c.c.}) + 2\gamma(A_{1,T_2} \Theta_{1,x} e^{i\omega_1 T_0} \\ & + A_{2,T_2} \Theta_{2,x} e^{i\omega_2 T_0} + \text{c.c.}) - 3E_c(A_1 \Theta_{1,x} e^{i\omega_1 T_0} + A_2 \Theta_{2,x} e^{i\omega_2 T_0} + \text{c.c.})^2 \\ & \cdot (A_1 \Theta_{1,xx} e^{i\omega_1 T_0} + A_2 \Theta_{2,xx} e^{i\omega_2 T_0} + \text{c.c.})/2 \\ & - \omega_1^2 Q_1 e^{i\omega_1 T_0} + \alpha(E_b - E_a)(i\omega_1 A_1 \Theta_{1,xxxx} e^{i\omega_1 T_0} + i\omega_2 A_2 \Theta_{2,xxxx} e^{i\omega_2 T_0} + \text{c.c.}) \\ & + ib(e^{i\omega T_0} - e^{-i\omega T_0})/2 - \omega_2^2 Q_2 e^{i\omega_2 T_0} + \alpha(E_b - E_a)\gamma(A_1 \Theta_{1,xxxx} e^{i\omega_1 T_0} \\ & + A_2 \Theta_{2,xxxx} e^{i\omega_2 T_0} + \text{c.c.}) + \gamma^2(Q_{1,xx} e^{i\omega_1 T_0} + Q_{2,xx} e^{i\omega_2 T_0}) \\ & + 2i\gamma(\omega_1 Q_{1,x} e^{i\omega_1 T_0} + \omega_2 Q_{2,x} e^{i\omega_2 T_0}) + E_a(Q_{1,xxxx} e^{i\omega_1 T_0} + Q_{2,xxxx} e^{i\omega_2 T_0}) \\ & - (Q_{1,xx} e^{i\omega_1 T_0} + Q_{2,xx} e^{i\omega_2 T_0}) = 0. \end{aligned} \quad (29)$$

Substituting Eqs. (24), (25), and (27) into Eq. (29), multiplying both sides of the obtained equation by  $\sin(m\pi x)$  ( $m = 1, 2, 3, 4$ ), integrating with respect to  $x$  from 0 to 1, and then abstracting the coefficients of  $\exp(i\omega_1 T_0)$  and  $\exp(i\omega_2 T_0)$  yield the following equation:

$$\begin{pmatrix} K_{11} - \omega_k^2 & -16i\gamma\omega_k/3 & 0 & -32i\gamma\omega_k/15 \\ 16i\gamma\omega_k/3 & K_{22} - \omega_k^2 & -48i\gamma\omega_1/5 & 0 \\ 0 & 48i\gamma\omega_k/5 & K_{33} - \omega_k^2 & -96i\gamma\omega_k/7 \\ 32i\gamma\omega_k/15 & 0 & 96i\gamma\omega_k/7 & K_{44} - \omega_k^2 \end{pmatrix} \begin{pmatrix} q_{k,1} \\ q_{k,2} \\ q_{k,3} \\ q_{k,4} \end{pmatrix} = \begin{pmatrix} H_{k,1} \\ H_{k,2} \\ H_{k,3} \\ H_{k,4} \end{pmatrix}, \quad (30)$$

where  $k = 1, 2$ .  $K_{11}$ ,  $K_{22}$ ,  $K_{33}$ , and  $K_{44}$  are defined in Eq. (18).  $H_{k,1}$ ,  $H_{k,2}$ ,  $H_{k,3}$ , and  $H_{k,4}$  are defined by

$$\left\{ \begin{aligned} H_{k,1} &= (B_1 + 8p_{k,2}\gamma C_1/3 + 16p_{k,4}\gamma C_2/15 + \text{c.c.}) + D_1(A_k(4p_{k,2}/15) + \text{c.c.})^2 + E, \\ H_{k,2} &= (B_2 + 8p_{k,1}\gamma C_5/3 - 24p_{k,3}\gamma C_6/5 + \text{c.c.}) \\ &\quad + D_2(A_1(4p_{k,1}/3 + 12p_{k,3}/5) + \text{c.c.})^2 + E, \\ H_{k,3} &= (B_3 + 24p_{k,2}\gamma C_3/5 - 48p_{k,4}\gamma C_4/7 + \text{c.c.}) \\ &\quad + D_3(A_k(12p_{k,2}/5 - 24p_{k,4}/7) + \text{c.c.})^2 + E, \\ H_{k,4} &= (B_4 + 16p_{k,1}\gamma C_5/15 + 48p_{k,3}\gamma C_6/7 + \text{c.c.}) \\ &\quad + D_4(A_1(8p_{k,2}/15 + 24p_{k,3}/7) + \text{c.c.})^2 + E, \end{aligned} \right. \quad (31)$$

where

$$\begin{cases} B_i = i\omega_1 p_{k,i}(A_{1,T_2} + (i\pi)^4 \alpha(E_b - E_a)A_1), & i = 1, 2, 3, 4, \quad k = 1, 2, \\ C_1 = A_{k,T_2} - 8\alpha(E_b - E_a)\pi^4 A_k, & C_2 = A_{k,T_2} - 128\alpha(E_b - E_a)\pi^4 A_k, \\ C_3 = A_{k,T_2} + 8\alpha(E_b - E_a)\pi^4 A_k, & C_4 = A_{k,T_2} + 128\alpha(E_b - E_a)\pi^4 A_k, \\ C_5 = A_{k,T_2} + \alpha(E_b - E_a)\pi^4 A_k/2, & C_6 = A_{k,T_2} + 81\alpha(E_b - E_a)\pi^4 A_k/2, \\ E = ib(e^{i\omega T_0} - e^{-i\omega T_0})/2, & D_i = 3E_c(i^2\pi^2 A_k p_{k,i} + \text{c.c.})/4. \end{cases} \quad (32)$$

Therefore, the following solvability condition is derived:

$$\begin{aligned} & H_{k,1}K_{22}K_{33}K_{44} - H_{k,1}K_{22}K_{33}\omega_k^2 - H_{k,1}K_{22}\omega_k^2 K_{44} + H_{k,1}K_{22}\omega_k^4 \\ & - 9216H_{k,1}K_{22}\gamma^2\omega_k^2/49 - H_{k,1}\omega_k^2 K_{33}K_{44} + H_{k,1}\omega_k^4 K_{33} + H_{k,1}\omega_k^4 K_{44} \\ & - H_{k,1}\omega_k^6 + \frac{343296}{1225}H_{k,1}\omega_k^4\gamma^2 - \frac{2304}{25}H_{k,1}\gamma^2\omega_k^2 K_{44} - \frac{32}{15}iH_{k,4}\gamma\omega_k^3 K_{33} - \frac{16}{3}iH_{k,2}\gamma\omega_k^3 K_{44} \\ & - \frac{32}{15}iH_{k,4}\gamma\omega_k^3 K_{22} + \frac{32}{15}iH_{k,4}\gamma\omega_k^5 - \frac{16}{3}iH_{k,2}\gamma\omega_k^3 K_{33} - \frac{256}{5}H_{k,3}\gamma^2\omega_k^2 K_{44} + \frac{768}{35}H_{k,3}\omega_k^4\gamma^2 \\ & + \frac{16}{3}iH_{k,2}\gamma\omega_k^5 + \frac{1024}{35}H_{k,3}K_{22}\gamma^2\omega_k^2 - \frac{1572864}{1225}iH_{k,2}\gamma^3\omega_k^3 + \frac{32}{15}iH_{k,4}\gamma\omega_k K_{22}K_{33} \\ & + \frac{16}{3}iH_{k,2}\gamma\omega_k K_{33}K_{44} - \frac{786432}{875}iH_{k,4}\gamma^3\omega_k^3 = 0. \end{aligned} \quad (33)$$

The solutions of  $A_1(T_2)$  and  $A_2(T_2)$  are expressed as follows:

$$\begin{cases} A_1(T_2) = \frac{1}{2}a_1(T_2)e^{i\theta_1(T_2)}, \\ A_2(T_2) = \frac{1}{2}a_2(T_2)e^{i\theta_2(T_2)}, \end{cases} \quad (34)$$

where  $a_1(T_2)$  and  $a_2(T_2)$  are, respectively, the amplitudes of the primary resonance responses, and  $\theta_1(T_2)$  and  $\theta_2(T_2)$  are the phase angles of the corresponding responses, respectively. Then, the following four equations are obtained:

$$\begin{cases} \Gamma_{11}a_1 + \Gamma_{12}a_1^2 a_2 \sin \beta_1 + \Gamma_{13}b \cos \beta_2 = 0, \\ \Gamma_{21}a_1 a_2^2 + \Gamma_{22}a_1 \sigma_2 + \Gamma_{23}a_1^2 a_2 \cos \beta_1 + \Gamma_{24}a_1^3 + \Gamma_{25}b \sin \beta_2 = 0, \\ \Gamma_{31}a_2 + \Gamma_{32}a_1^3 \sin \beta_1 = 0, \\ \Gamma_{41}a_2 \sigma_2 + \Gamma_{42}a_2 \sigma_1 + \Gamma_{43}a_1^3 \cos \beta_1 + \Gamma_{44}a_1^2 a_2 + \Gamma_{45}a_2^3 = 0, \end{cases} \quad (35)$$

where  $\Gamma_{11}, \Gamma_{12}, \Gamma_{13}, \Gamma_{21}, \Gamma_{22}, \Gamma_{23}, \Gamma_{24}, \Gamma_{25}, \Gamma_{31}, \Gamma_{32}, \Gamma_{41}, \Gamma_{42}, \Gamma_{43}, \Gamma_{44}$ , and  $\Gamma_{45}$  are constant coefficients, and

$$\beta_1 = T_2\sigma_1 - 3\theta_1 + \theta_2, \quad \beta_2 = T_2\sigma_2 - \theta_1. \quad (36)$$

The relationships between the detuning parameters and the modal amplitudes are built as



follows:

$$\begin{cases} a_2^2(\Xi_{11}\sigma_1 - \Xi_{12}\sigma_2 + \Xi_{13}a_1^2 + \Xi_{14}a_2^2)^2 + a_2^2\Xi_{15} = a_1^6, \\ (-\Xi_{21}a_1^2a_2^2 - \Xi_{22}a_1^2\sigma_2 + \Xi_{23}a_1^4 + \Xi_{24}a_2^2\sigma_2 \\ -\Xi_{25}a_2^2\sigma_1 - \Xi_{26}a_2^4)^2 + (\Xi_{27}a_1^2 + \Xi_{28}a_2^2)^2 = a_1^2b^2, \end{cases} \quad (37)$$

where  $\Xi_{mk}$  ( $m = 1, 2, 3, 4, 5$ , and  $k = 1, 2, \dots, 8$ ) are constant coefficients. The stability of the modal steady-state response amplitudes is determined by the Lyapunov stability theory.

## 5 Scheme of Galerkin method

The approximate analytic results are verified by the four-term Galerkin truncation. Therefore, the transverse displacement of the traveling beam is assumed as follows<sup>[24–25]</sup>:

$$u(x, t) = \sum_{m=1}^4 q_m(t) \sin(m\pi x), \quad m = 1, 2, 3, 4, \quad (38)$$

where  $q_m(t)$  ( $m = 1, 2, 3, 4$ ) are the generalized coordinates. Substituting Eq. (38) into Eq. (11), multiplying the resulting equation by the weighting function  $\sin(g\pi x)$  ( $g = 1, 2, 3, 4$ ), and integrating the product from 0 to 1 yield

$$\begin{aligned} & \ddot{q}_1 + C\dot{q}_1 - C_1\dot{q}_2 - C_2\dot{q}_4 + k_{11}q_1 - k_{12}q_2 - k_{14}q_4 \\ & - \alpha A_3 q_1 (q_1\dot{q}_1 + 4q_2\dot{q}_2 + 9q_3\dot{q}_3 + 16q_4\dot{q}_4) + 3E_c\pi^4 q_1 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2)/4 \\ & - \alpha E_h\pi^4 \dot{q}_1 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2)/2 - 12\alpha E_h\pi^4 q_2 q_3 \dot{q}_4 \\ & + \pi^4 \alpha E_h \gamma q_2 (7q_1^2/3 + 88q_2^2/21 + 88q_3^2/7 + 32\,608q_4^2/693)16/5 \\ & + 32\pi^4 \alpha E_h \gamma q_4 (59q_1^2/21 + 496q_2^2/63 + 277q_3^2/11 + 6\,016q_4^2/429)/5 \\ & + 288\pi^4 \alpha E_h \gamma q_1 q_2 q_3 /7 + 1\,216\pi^4 \alpha E_h \gamma q_1 q_3 q_4 /15 - 3\alpha E_h\pi^4 q_3 \dot{q}_1 q_1 /2 \\ & - 12\alpha E_h\pi^4 q_3 q_4 \dot{q}_2 + 3\alpha E_h\pi^4 q_1^2 \dot{q}_1 /4 - \alpha E_h\pi^4 4q_2 \dot{q}_4 q_1 \\ & - 3\alpha E_h\pi^4 \dot{q}_3 (q_1^2 + 4q_2^2 + 16q_4^2)/4 - 6\alpha E_h\pi^4 q_2 q_3 \dot{q}_2 - \alpha E_h\pi^4 4q_4 \dot{q}_2 q_1 \\ & - 4\alpha E_h\pi^4 q_2 q_4 \dot{q}_1 + 24E_c\pi^4 (12q_2^2 q_3 + 3q_1^2 q_3 + 48q_2 q_3 q_4 + 16q_1 q_2 q_4 - q_1^3) - b \sin(\omega t) = 0, \end{aligned} \quad (39a)$$

$$\begin{aligned} & \ddot{q}_2 + 16C\dot{q}_2 + C_1\dot{q}_1 - C_3\dot{q}_3 + k_{22}q_2 - k_{23}q_3 + k_{21}q_1 \\ & - 4\alpha E_h\pi^4 q_2 (q_1\dot{q}_1 + 4q_2\dot{q}_2 + 9q_3\dot{q}_3 + 16q_4\dot{q}_4) + 3E_c\pi^4 q_2 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2) \\ & - 2\alpha E_h\pi^4 \dot{q}_2 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2) + 3E_c\pi^4 q_1^2 q_4 \\ & + 8\pi^4 \alpha \gamma E_h q_3 (117q_1^2/7 + 196q_2^2/3 + 5\,589q_3^2/77 + 95\,408q_4^2/273)/5 - 18\alpha E_h\pi^4 q_3^2 \dot{q}_4 \\ & - 36\alpha E_h\pi^4 q_3 q_4 \dot{q}_3 + 8\pi^4 \alpha \gamma E_h q_1 (q_1^2/3 + 244q_2^2/21 + 127q_3^2/7 + 20\,432q_4^2/693)/5 \\ & - 2\alpha E_h\pi^4 q_1^2 \dot{q}_4 - 12\alpha E_h\pi^4 q_1 q_3 \dot{q}_4 + \pi^4 \alpha E_h \gamma q_2 q_4 (19\,584q_3/55 + 7\,552q_1/63) \\ & - 12\alpha E_h\pi^4 q_3 q_4 \dot{q}_1 + 3E_c\pi^4 (6q_1 q_3 q_4 + 3q_1 q_2 q_3 + 9q_3^2 q_4 - 2q_2^3) - 12\alpha E_h\pi^4 q_1 q_4 \dot{q}_3 \\ & - 4\alpha E_h q_1 \pi^4 q_4 \dot{q}_1 - 6\alpha \pi^4 E_h q_1 q_3 \dot{q}_2 + 4\alpha E_h \pi^4 q_2^2 \dot{q}_2 - 6\alpha E_h \pi^4 q_1 q_2 \dot{q}_3 \\ & - 6\alpha E_h \pi^4 q_2 q_3 \dot{q}_1 = 0, \end{aligned} \quad (39b)$$

$$\begin{aligned}
& \ddot{q}_3 + 81C\dot{q}_3 + C_3\dot{q}_2 - C_4\dot{q}_4 + k_{33}q_3 - k_{34}q_4 - 9\alpha E_h \pi^4 q_3 (q_1\dot{q}_1 + 4q_2\dot{q}_2 + 9q_3\dot{q}_3 + 16q_4\dot{q}_4) \\
& + 27E_c \pi^4 q_3 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2)/4 - 9\alpha E_h \pi^4 \dot{q}_3 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2)/2 \\
& - 6\alpha E_h q_1 \pi^4 q_2 \dot{q}_2 + 16\pi^4 \alpha E_h \gamma q_2 (9q_1^2/7 - 8q_2^2/3 + 2403q_3^2/77 + 2272q_4^2/91)/5 + k_{32}q_2 \\
& + 32\pi^4 \alpha E_h \gamma q_4 (67q_1^2/7 + 3312q_2^2/77 + 7479q_3^2/91 + 1664q_4^2/21)/5 + 243/4 \alpha E_h \pi^4 q_3^2 \dot{q}_3 \\
& - 12\alpha E_h \pi^4 q_1 q_4 \dot{q}_2 - 36\alpha E_h \pi^4 q_2 q_4 \dot{q}_3 - 3\alpha E_h \pi^4 q_1^2 \dot{q}_1/4 - 3\alpha E_h \pi^4 q_2^2 \dot{q}_1 - 12\alpha E_h \pi^4 q_1 q_2 \dot{q}_4 \\
& - 36\alpha E_h q_3 \pi^4 q_4 \dot{q}_2 + \pi^4 \alpha E_h \gamma q_1 q_3 (3552q_2/35 + 1344q_4/11) - b \sin(\omega t)/3 + 9E_c \pi^4 q_2^2 q_1/2 \\
& - 36\alpha E_h q_2 \pi^4 q_3 \dot{q}_4 - 12\alpha E_h \pi^4 q_2 q_4 \dot{q}_1 \\
& + 3E_c \pi^4 q_1^3/8 + 3E_c \pi^4 (18q_2 q_3 q_4 - 81q_3^3/8 + 6q_1 q_2 q_4) = 0, \tag{39c}
\end{aligned}$$

$$\begin{aligned}
& \ddot{q}_4 + 256C\dot{q}_4 + C_2\dot{q}_1 + C_4\dot{q}_3 + k_{44}q_4 + k_{41}q_1 + k_{43}q_3 + 12E_c \pi^4 q_4 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2) \\
& - 16\alpha E_h \pi^4 q_4 (q_1\dot{q}_1 + 4q_2\dot{q}_2 + 9q_3\dot{q}_3 + 16q_4\dot{q}_4) - 8\alpha E_h \pi^4 \dot{q}_4 (q_1^2 + 4q_2^2 + 9q_3^2 + 16q_4^2) \\
& + 16\pi^4 \alpha E_h \gamma q_3 (11q_1^2/21 - 1332q_2^2/77 - 2673q_3^2/91 + 1713q_4^2/21)/5 + 3E_c \pi^4 q_1^2 q_2 \\
& + 16\pi^4 \alpha E_h \gamma q_1 (-13q_1^2/21 + 428q_2^2/63 + 511q_3^2/11 + 22288q_4^2/429)/5 + 18q_1 q_2 q_3 E_c \pi^4 \\
& + 192\alpha E_h \pi^4 q_4^2 \dot{q}_4 + \pi^4 \alpha E_h \gamma q_2 q_4 (2521q_1/1155 + 547q_3/91)256/3 - 2\alpha E_h \pi^4 \dot{q}_2 (q_1 + 3q_3)^2 \\
& - 12\alpha E_h \pi^4 q_2 \dot{q}_3 (q_1 + 3q_3) - 4\alpha E_h \pi^4 \dot{q}_1 q_2 (q_1 + 3q_3) + 3(9q_3^2 q_2 - 32q_4^3) E_c \pi^4 = 0, \tag{39d}
\end{aligned}$$

where

$$\begin{cases} C = \alpha(E_b - E_a)\pi^4, & C_1 = 16\gamma/5, & C_4 = 96\gamma/\pi, \\ A_1 = \pi^4 \alpha \gamma E_b, & A_2 = \pi^4 E_h \gamma, & A_3 = \pi^4 E_h, & E_h = (E_c - E_d), \\ k_{12} = 128A_1/5, & k_{34} = 12288A_1/7, & k_{41} = 16A_1/15, & k_{21} = 8A_1/3, \\ k_{32} = 384A_1/5, & k_{43} = 3888A_1/7. \end{cases} \tag{40}$$

The set of the second-order ordinary differential equation (39) is numerically calculated by the fourth-order Runge-Kutta method. In the following numerical examples, the time step is set as 0.001<sup>[26-27]</sup>. Moreover, the initial conditions are set as follows:

$$q_1(t) = 0.001, \quad q_m(t) = 0, \quad m = 2, 3, 4, \quad \dot{q}_m(t) = 0, \quad m = 1, 2, 3, 4. \tag{41}$$

## 6 Numerical results

Figure 3 illustrates the comparisons of the steady-state periodic responses by use of the multi-scale method and based on the Galerkin method. Similar comparisons with the weaker flexural stiffness of the traveling beam, i.e.,  $E_b = 0.03$ , are shown in Fig. 4. Both Figs. 3 and 4 clearly depict that the second-order mode resonates when the excitation frequency is close to the first-order natural frequency. Therefore, the energy transfers from the second-order mode to the first-order mode. Figures 3 and 4 also demonstrate that the approximate analytical results and the numerical results are almost overlapping. Specially, the amplitude curves of resonance at the first-order mode based on the two approaches overlap completely. Compared with the results without the internal resonance in Refs. [10] and [21], the amplitude-frequency curves are

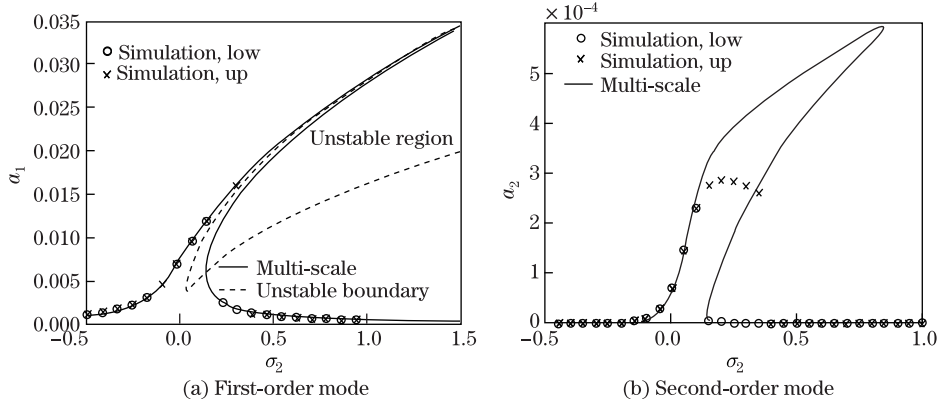


Fig. 3 Comparison of analytical and Galerkin method's results

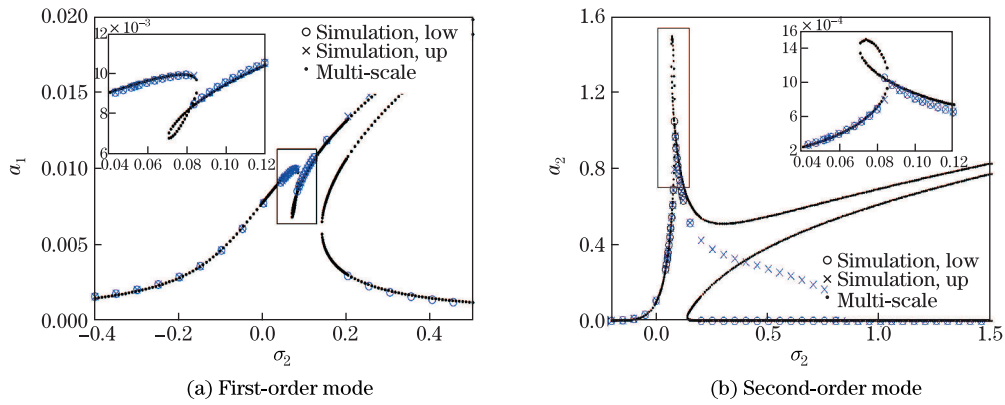


Fig. 4 Comparison of analytical and Galerkin method's results with weaker flexural stiffness

more complicate in the presence of the internal resonance, specially when the flexural stiffness of the beam is relatively soft.

Figure 5 shows the effects of the amplitude of the excitation on the primary resonance response with the internal resonance. The numerical results show the hysteresis phenomenon of

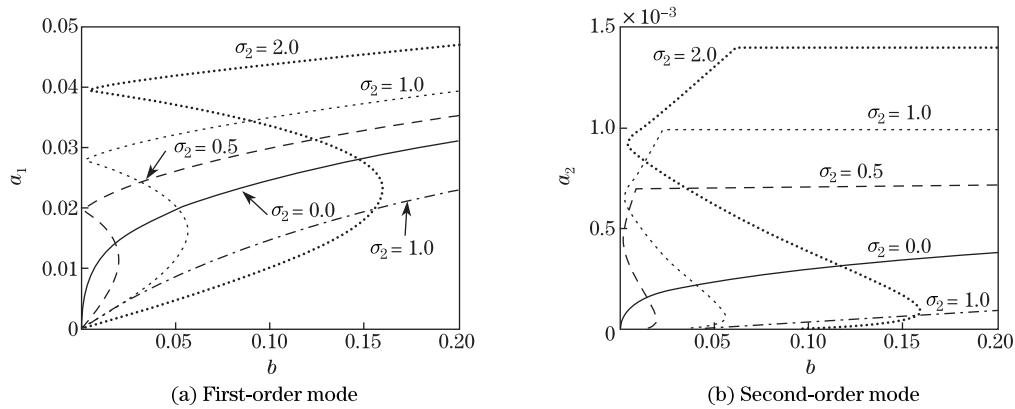
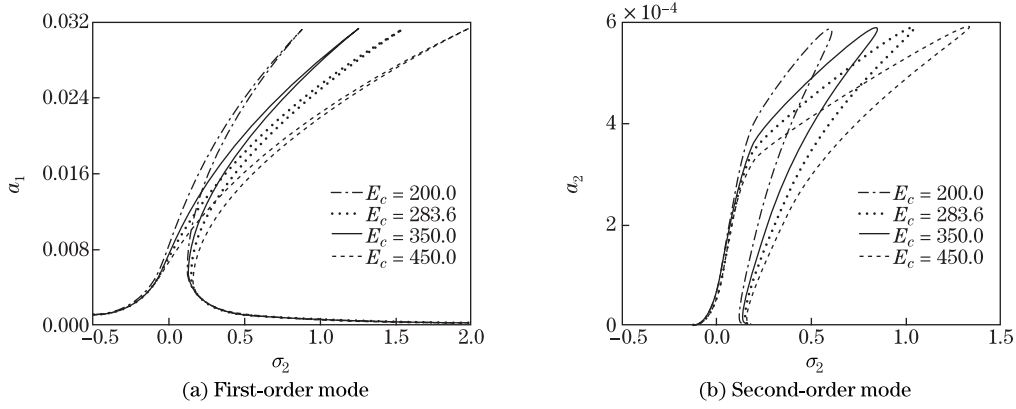


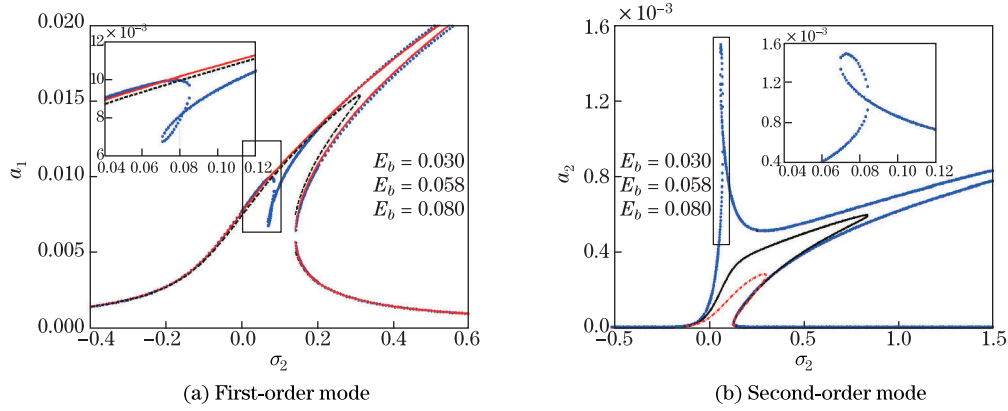
Fig. 5 Effects of excitation amplitude on resonance responses

the axially traveling beam. Under the conditions of 3:1 internal resonance between the first two modes and the first- and second-order mode resonates, the amplitude of the resonance response changes with the excitation amplitude. Moreover, the saturated phenomenon is observed in Fig. 5(b). This is an interesting phenomenon since the saturation of the second-order mode happens at the first-order primary resonance.

The effects of the system parameters on the amplitude-frequency curves are presented in Figs. 6, 7, and 8. The results indicate that the amplitudes of the resonance of the first two modes are very sensitive to the system parameters. From Fig. 6, one can easily find that the amplitude-frequency curves bend to the right with the increase in the nonlinear coefficient. The simulations shown in Figs. 7 and 8 depict that the amplitude of the resonance with the internal resonance increases with the decreases in the flexural stiffness and the viscous coefficient of the beam. Moreover, the nonlinear dynamics of the axially traveling beam becomes more complicate when the flexural stiffness and viscous coefficient of the beam become smaller.



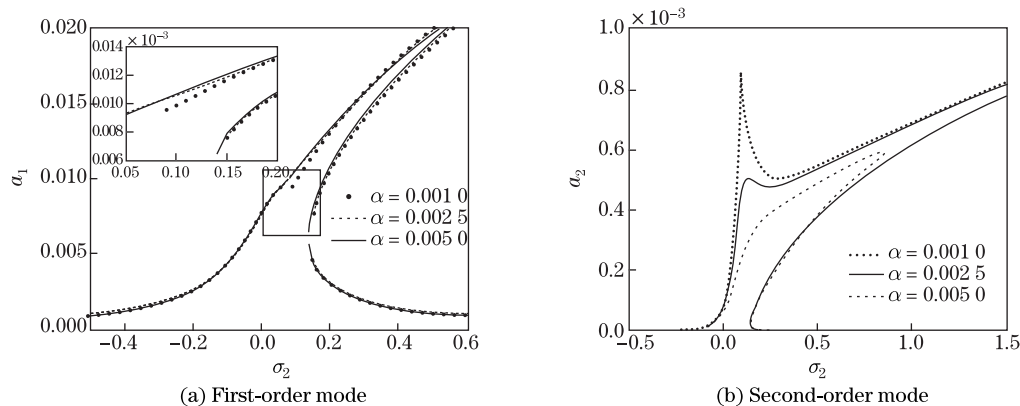
**Fig. 6** Effects of nonlinearity on resonance responses



**Fig. 7** Effects of flexural stiffness of traveling beam on resonance responses

## 7 Conclusions

The purpose of this paper is to investigate the effects of the internal resonance and the viscoelastic behaviors on the primary response of the traveling viscoelastic beam. The standard



**Fig. 8** Effects of viscous coefficient on resonance responses

linear solid model with the material time derivative is adopted to describe the viscoelastic properties of the axially traveling beam. The real modal functions of the linear derived equation are approximately established. Usually, the linear derived system of the nonlinear governing equation is solved by complex modal functions. The steady-state response amplitudes are firstly determined by the direct multi-scale method with the real modal functions. Then, the Galerkin method is used to verify the approximate solutions. The verification of the Galerkin method shows that the approximate analytical results are acceptable. Therefore, the direct multi-scale method with approximate real modal functions is a reliable approach for studying the nonlinear vibration of the traveling beam. The steady-state responses for the internal resonance modes show that the effect of the internal resonance cannot be ignored.

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