

1 **State-of-the-Art Review on FRP Sandwich Systems for Lightweight Civil Infrastructure**

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3 **Abstract**

4 Fiber reinforced polymer (FRP) sandwich systems as primary load-bearing elements are
5 relatively new concepts in lightweight civil infrastructure. These systems offer a combination
6 of light weight, high strength, thermal insulation for some types, and service-life benefits.
7 Recent developments and applications have demonstrated that these composite systems have
8 emerged as a cost-effective alternative, especially when each material component is
9 appropriately designed. Still, some issues and challenges need to be addressed if FRP systems
10 are to gain widespread use in civil infrastructure. This paper provides an overview of the state-
11 of-the-art research, development, and applications of FRP sandwich systems. It also identifies
12 the challenges and future opportunities for the broad use of these advanced systems in civil
13 engineering and construction.

14 **Authors keywords:** Sandwich systems; Fiber reinforced polymer; civil engineering;
15 developments; challenges; opportunities.

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29 **Introduction**

30 Fiber reinforced polymer (FRP) sandwich systems are special form of a laminated composite
31 fabricated by attaching two thin skins to a thick lightweight core (ASTM C274-99). These
32 systems are increasingly used in applications requiring high bending stiffness and strength,
33 combined with low weight (Belouettar et al. 2009). FRP sandwich systems have been widely
34 used in the aerospace, aircraft, and marine industries because of their many advantages such as
35 improving the efficiency in transportation vehicles (Bakis et al. 2002). Such systems have also
36 drawn considerable interest in the construction industry and are now emerging as effective
37 alternatives for use in niche civil infrastructure applications (Keller et al. 2007). The low weight
38 of FRP sandwich systems facilitates handling during assembly, while reducing installation and
39 transportation costs. This can significantly speed up construction, especially rebuilding in
40 disaster areas and in highly populated areas to reduce traffic interruptions. These systems also
41 offer benefits in structures built in rural and regional areas where relatively light construction
42 and lifting equipment are used. Moreover, the high insulating capacity of some FRP sandwich
43 systems is of great interest for energy-efficient buildings. In addition, the construction
44 industry's need for durable, cost-effective civil infrastructure has generated significant research
45 into developing new and innovative FRP sandwich systems.

46 The material combination coupled with the geometry of FRP sandwich systems, makes
47 it possible to optimize designs for specific applications. FRP skins are commonly used due to
48 their lightweight and high tensile strength. A great variety of core materials have been used,
49 such as balsa wood, polymeric foam core, honeycomb, and trussed core. Similarly, the bond
50 between the skin and core interface must be adequate in order to make the best use of the
51 effective mechanical properties of the skin and core materials. The evolution of FRP sandwich
52 systems through research and development has provided an opportunity to expand the
53 engineering application of these advanced material systems.

54 Recent developments in FRP sandwich systems have been very valuable in terms of
55 research and applications. The feasibility and potential of FRP sandwich systems in civil
56 engineering and construction have been successfully demonstrated for various applications
57 including structural roofs (Keller et al. 2008), floors (Van Erp and Rogers 2008), walls (Sharaf
58 and Fam 2011), and bridge decks (Keller et al. 2014) as well as composite railway sleepers
59 (Manalo and Aravinthan 2012a; Van Erp and McKay 2013), bridge beams (Primi et al. 2009;
60 Van Erp and McKay 2012), and floating and protective structures (Qiao et al. 2008; Liu et al.
61 2013). The potential of these systems has yet to be fully explored despite engineers having
62 access to a wide range of sandwich composites. This paper reviews the state-of-the-art research,
63 innovations, and developments related to FRP sandwich systems, including field
64 implementation in lightweight civil infrastructure. Challenges in using these emerging
65 composite systems are discussed and future prospects are presented to pave the way in
66 improving confidence in using FRP sandwich systems for civil infrastructure.

67 **Components of FRP Sandwich Systems**

68 This section describes the materials commonly used in manufacturing (as well as the production
69 methods) and joining FRP sandwich systems.

70 ***Top and Bottom Skins***

71 The top and bottom skins are largely responsible for the system's flexural strength and stiffness.
72 Steel, stainless steel and aluminium often serve as skin materials and have been extensively
73 studied and used in aerospace, automotive, and marine applications while FRP have emerged
74 as an excellent alternative skin material for structural engineering applications (Mathieson and
75 Fam 2014a). The high specific strength and stiffness of FRP significantly reduce the weight of
76 sandwich composites. According to the ACI recommendations for reinforcement of concrete
77 members using FRP materials, the fibres used in that important civil engineering application

78 can be glass, carbon or aramid bonded with epoxies, polyester, vinyl esters, or phenolics (ACI
79 440R 2007). The same type of fibres can be used in the skins of sandwich panels.

80 Glass fibers are the most commonly used due to their relatively lower comparative cost.
81 The use of basalt (Torres et al. 2013) and bio-based fibers is now also being explored for FRP
82 sandwich systems. Mak et al. (2015a) published results indicating that FRP sandwich systems
83 with three layers of flax fiber skins provide equivalent structural performance to composites
84 comprising a single layer of glass fibers skins. Interestingly, the sandwich composites with flax
85 fiber skins exhibited a more flexible failure than the glass fibers. Indeed, using these new fiber
86 types for the skins may result in more cost-effective and environmentally-friendly FRP
87 sandwich systems.

88 According to Barbero (1999), the strength and stiffness of FRP skins depend largely on
89 fiber amounts and the direction the fibers were laid. In general, skins fail by face yielding in
90 either tension or compression, face wrinkling, buckling due to core compressive failure or
91 adhesive-bond failure, and by dimpling in the case of sandwich systems with cellular cores.
92 Borsellino et al. (2004) and Belouttar et al. (2009) however indicated that the nature of the core
93 influences more the fracture mechanism of FRP sandwich systems than the different skin
94 arrangements. Gdoutos and Daniel (2008) suggested that core usually fails by shear cracking,
95 crushing, core indentation, and flexural tensile cracking. Thus, research and development have
96 focused more on developing effective core material systems so as to boost the performance of
97 FRP sandwich systems.

98 *Core Materials and Systems*

99 The lightweight core material of unreinforced cores provides the shear rigidity and strength of
100 FRP sandwich systems. Traditionally, sandwich composites consist of a simple foam core, as
101 shown in Fig. 1a. The two of the most commonly used materials are polyvinyl chloride (PVC)
102 foam in densities ranging from 30kg/m³ to 400kg/m³ and rigid polyurethane (PU) foam in

103 densities ranging from approximately 21 kg/m^3 up to 400 kg/m^3 . While structurally inferior to
104 denser cores, lower density PU foam core is desirable for its higher thermal insulation
105 (Mathieson and Fam 2014a). Another big advantage of PUR foams is their relatively low cost.
106 Polyethylene terephthalate (PET) foam is also increasingly being used for FRP sandwich
107 systems and is very interesting by being thermoplastic and recyclable. Balsa wood (Fig. 1b) is
108 another common core material because of its lightweight and good mechanical properties
109 (Grenestedt and Bekisli 2003). Usually, small blocks of balsa wood are glued together side by
110 side to form sheets of the so-called end-grain balsa, in which the grain is oriented parallel to the
111 thickness. As a core material, end-grain balsa improves sandwich panel resistance to local
112 indentation and face-sheet wrinkling (Kepler 2011). Furthermore, balsa has positive shear
113 properties and contributes significantly to bending stiffness (Keller et al. 2014). Table 1
114 provides the mechanical properties of the balsa wood and foam core commonly used for FRP
115 sandwich systems as reported by Beckwith (2008) and Manalo et al. (2013a).

116 Honeycomb and truss cores have received much attention in recent years to meet the
117 through-the-thickness compression requirements of FRP sandwich systems. Honeycombed
118 core FRP sandwich systems (Fig. 1c) may be low-density polymeric or metallic (aluminium)
119 honeycomb core (He and Hu 2008). FRP sandwich systems with a honeycomb core can perform
120 better than foam core in compression and shear at equivalent weight. On the other hand,
121 sandwich composites with trussed cores (see Fig. 1d) are highly efficient from a weight
122 standpoint and deliver good compression performance (Wicks and Hutchinson 2001).
123 However, trussed cores are very hard to produce properly, guaranteeing fibre continuity
124 between truss members and face sheets is challenging, and this type of solution is often poor
125 from a cost-effectiveness stand-point.

126 While various core materials are available, the nature of those in common use limits
127 their application in FRP sandwich systems for civil infrastructure. Fam and Sharaf (2010)

128 indicated that FRP sandwich systems with lightweight foam cores usually fail due to core
129 indentation and shear and that their low shear stiffness leads to early skin-core delamination
130 failure. Open-cell foam cores tend to absorb and retain moisture and have very low fire
131 resistance (Marsh 2007). The major problem with balsa wood is its susceptibility to water
132 penetration leading to swelling, debonding, and rotting (Grenestedt and Bekisli 2003).
133 Similarly, the closed cells in the honeycomb core are susceptible to entrapping moisture which
134 can lead to core and skin delamination (Kooistra and Wadley 2007). On the other hand, FRP
135 sandwich systems with truss cores are weak at bearing concentrated loads and very difficult to
136 join (Demelio et al. 2001). The presence of cavities between the skins of honeycomb and truss
137 core materials reduces the capacity of sandwich composites to hold mechanical connectors.
138 Moreover, the continued high cost of the honeycomb and truss core materials has restricted
139 their applications predominantly to the aerospace industry (Reis and Rizkalla 2008). Thus, the
140 evolution of FRP sandwich composites with lightweight and high-strength core materials will
141 help increase the use of these systems for civil engineering and construction.

142 ***Production Methods for FRP Sandwich Systems***

143 The adequate joining of the fibre composite skins and the core is one of the most difficult
144 aspects to achieve in the manufacturing of sandwich composites (Lee et al., 2004). In fact, many
145 of the failure modes of the FRP sandwich systems are closely related to the integrity of the
146 adhesion between the face and the core. Thus, strict quality control is required during their
147 production process to achieve a good adhesion. A description of the commonly used production
148 methods in manufacturing sandwich composites are presented in this section.

149 • *Adhesive bonding*

150 Adhesive bonding is a conventional method of manufacturing FRP sandwich systems wherein
151 composite faces are prepared and separately bonded to the core (Grunewald et al. 2015).

152 Adhesive layers are introduced between the faces and the core and the whole stack is subjected
153 to pressure using weights or a hydraulic press. For high-performance applications, the pressure
154 can be achieved using a vacuum bag and an autoclave. In this production method, it is normally
155 necessary to prepare the surfaces to be bonded in order to achieve a good enough bond. While
156 a time and labor intensive process, adhesive bonding is suitable for short production series of
157 small to medium-sized sandwich components for civil infrastructures.

158 • *Wet lay-up*

159 Wet lay-up is one the most commonly used methods to manufacture sandwich components with
160 composite faces (Zenkert 1995). During the wet lay-up process, the dry top and bottom fibres
161 are impregnated with a resin and laid out on the core. The wet-layup may be performed either
162 by hand lay-up or spray-up. Airborne volatile organic compounds (VOCs) content is very high
163 during this type of open mould production, which may pose safety and health concerns, forcing
164 workers to use appropriate protection gear. In case of vacuum assisted wet layup, the core is
165 placed in between the top and bottom fiber composite skins whereupon the vacuum is applied
166 to remove the excess resin. Rolling on top of the vacuum bags is common in order to minimise
167 voids in the skins and to produce a better surface finish. This method is very flexible yet labor
168 intensive and thus the method best suited for especially large and/or complex structures.

169 • *Vacuum assisted resin transfer moulding*

170 Vacuum assisted resin transfer moulding (VARTM) is a low cost manufacturing process that
171 has been employed to manufacture large sandwich structures such as turbine blades, boats, rail
172 cars and bridge decks since early 1980s (Mohamed et al. 2015). VARTM is capable of
173 producing geometrically complex sandwich structures in relatively short time, without creating
174 an unhealthy work environment since the process uses closed moulds. In this method, the dry
175 fibers are placed in the mould together with the core. Moreover, not only cores, but also inserts

176 and fasteners are easily integrated into the reinforcement of the core before impregnation. After
177 the mould is closed, the resin is introduced into the mould to impregnate the reinforcement
178 under vacuum. Sandwich composites manufactured through VARTM are characterized by good
179 to excellent mechanical properties due to low void contents in the skins and consistent laminate
180 quality.

181 • *Resin transfer moulding*

182 A two-sided mould is used for the Resin Transfer Moulding (RTM) (Barbero 1999). This
183 manufacturing method is different from VARTM wherein a top and rigid mould is used instead
184 of a flexible vacuum bag to form a vacuum-tight seal. Once the dry fibre composite skins and
185 the core are preformed and placed in the mould, the mould is closed and the resin is injected
186 into cavity. The two-sided mould permits the production of sandwich composites with good
187 surface finish on both sides. While this method is suited for high production volumes, the
188 necessity for top and bottom moulds prohibits its production to very large size sandwich panels.

189 • *Vacuum infusion*

190 The Vacuum Infusion (VI) process uses vacuum pressure to drive resin into a laminate (Zenkert
191 1995). In this manufacturing method, the dry fiber composite skins and the core materials are
192 placed in a mould. A vacuum is then applied through a flexible cover or vacuum bag tightly
193 sealed over the top of the mould. From that point, resin is infused using vacuum pressure. As
194 the VI process starts with none and pushes resin in, any excess resin that is introduced will
195 eventually be sucked out into the vacuum line. As a result, only the minimum amount of resin
196 is introduced producing a sandwich structure with a low weight and high strength. This
197 manufacturing method is well suited to large sandwich components in low to medium
198 production volume.

199 • *Co-curing*

200 Co-curing is a production method wherein the composite faces and the core are simultaneously
201 cured during the manufacturing (Lee et al. 2004). In this method, the dry fibres for the top and
202 bottom skins are infused with resin at the same time. The bottom skin is then laid in a mould,
203 the core is then placed, and finally the top skin. The assembled materials are then cured in one
204 operation ensuring a good bond between the skins and the core. Most of the co-curing methods
205 uses forming pressure and temperature in manufacturing and curing sandwich components. The
206 expansion of the foam core with temperature increase helps increase pressure within the mould.
207 Thus, the deformability and thermal expansion of the core especially the foam core material
208 systems are critical as their properties are known to decreased significantly at high curing
209 temperature.

210 ***Joining Techniques for FRP Sandwich Systems***

211 In civil engineering applications, connections are inevitable due to limitations on shape size and
212 the requirements of transportation and installation. For FRP sandwich systems, this requires the
213 consideration of connections between sandwich composites themselves as well as joints
214 between them and other structural components. Zhou and Keller (2005) reviewed the joining
215 techniques for FRP bridge decks in component, panel and structure levels. They indicated that
216 adhesive bonding has been widely used in component level connections as it is easier to design
217 and provides higher strength values than the bolted connections. Similarly, splicing–bonding
218 connections are well adapted for panel-to-panel connections. The splicing-bonding connections
219 is a type of connection wherein splice plate is fixed into the groove in the top and bottom of the
220 sandwich composites using an adhesive (Fig. 2a). Reising et al. (2004) reported that the
221 individual panels in the bridge decks that they have installed and evaluated were interconnected
222 using adhesively bonded tongue and groove systems, as it provides a more effective load
223 transfer and failure resistant capability than mechanical fixing. All-adhesive connections (Fig.

224 2b) are also preferable for deck-to-support connections when the supports are wide and flat
225 because their installation is simple and gives an even distribution of stresses in the joint (Zhou
226 and Keller 2005). In adhesively-bonded connections, however, the quality during on-site
227 installation and the fatigue and durability of the adhesive layer should be carefully ensured.

228 Recent studies presented other joining techniques for FRP sandwich systems. Dawood
229 and Peirick (2013) published their development work on connections for FRP sandwich
230 systems made of GFRP face skins and fiber reinforced foam core. They concluded that bonded
231 connections are more suitable for sandwich systems with a low-density foam core to effectively
232 utilise the high strength of FRP skins while bolted connection is more preferred for high-density
233 foam cored sandwich systems to achieve a higher capacity joint. Garrido et al. (2015a)
234 developed and investigated the behavior of a Z-shaped adhesive joint for connecting adjacent
235 sandwich deck panels (Fig. 3a). This connection is integrated into the sandwich panels during
236 production, and adhesively bonded on-site. These same authors also demonstrated the
237 effectiveness of angular steel sections as node connectors for sandwich composites with stiff
238 core materials such as balsa wood (Garrido et al. 2016). Manalo (2013) also presented a
239 connection systems that is composed of exterior studs with a groove for the shear key to connect
240 the adjacent prefabricated sandwich wall panels (Fig. 3b). The studs and the shear key are made
241 up of glass fiber reinforced rigid polyurethane (PU) foam. The results of the in-plane shear test
242 demonstrated that the rigid PU foam shear key provided a reliable attachment to connect two
243 adjacent panels. While some joining techniques are available, this issue has received
244 comparatively little research attention especially for FRP sandwich systems in load bearing
245 structures.

246 **Recent Developments in FRP Sandwich Systems**

247 Several researchers have contributed to the research and development of FRP sandwich systems
248 for structural purposes. These researchers have focused on enhancing core materials either

249 through cellular manipulation (CM), corrugated face (CF), fiber reinforcement (FR), geometric
250 arrangement (GA), introduction of an intermediate layer (IL) or hybrid (H) systems to
251 manufacture sandwich composites with higher performance. Table 2 provides a summary of
252 these developments as a companion to the detailed information presented in this section.

253 ***Cellular Manipulated Solid Core material***

254 Marsh (2007) suggested that cellular manipulation could be used to produce a sandwich panel
255 with a high-strength solid core. Accordingly, Van Erp and Rogers (2008) chemically modified
256 the plant-based phenolic resin to produce a lightweight but high-strength core material. The
257 solid phenolic core is consists of a proprietary lignin resin containing approximately 50% bio-
258 content. It was purposely designed for high compressive and shear strength to carry the
259 concentrated loads typical in bridges and other structural elements. It can be noted from Table
260 1 that the phenolic resin is twice as heavy as the heaviest PET or PUR foams. Extensive
261 characterisation of the mechanical properties of this novel core material was conducted and
262 reported by Manalo et al. (2013a). Investigation of the flexural (Manalo et al. 2010a) and shear
263 behavior (Manalo et al. 2010b) of the FRP sandwich systems with this core material indicated
264 that its strength and stiffness were suitable for civil engineering and construction. This FRP
265 sandwich panel has been used in several building and residential projects in Australia and its
266 use has already been explored for bridge infrastructure.

267 ***Corrugated Facing Sandwich Panels***

268 Kampner and Grenestedt (2007) introduced sandwich composites with corrugated skin and
269 foam core to improve shear capacity and reduce weight. The corrugated skin also increased the
270 wrinkling strength of compression-loaded sandwich composites. As a result, the corrugated
271 sandwich beams showed similar strength but weighed 10% to 20% less than their plain
272 counterparts.

273 ***Fiber Reinforced Core Material Systems***

274 Karlsson and Astrom (1997) suggested that reinforcing through thickness with fibers can
275 potentially significantly improve the structural integrity of sandwich composites. Thus, Reis
276 and Rizkalla (2008) developed a 3-D fiber reinforced composite sandwich panel (Fig. 4) which
277 increased the shear modulus and through-the-thickness compressive strength of the foam core
278 but caused a decrease in the tensile strength and stiffness of the skin due to the waviness created
279 by the stitched fibers.

280 *Geometrically Arranged Core Material*

281 A number of developments have focused on enhancing the structural performance of FRP
282 sandwich systems by geometrically modifying the arrangement of the core material. Grenestedt
283 and Bekisli (2003) analysed the behavior of a sandwich core with a new arrangement of balsa
284 blocks. This core consisted of an assembly of complex-shaped balsa blocks with the grains
285 oriented 30° to 60° with respect to the longitudinal direction and was predicted to have a 70%
286 higher average effective shear stiffness than that of end-grain balsa. Experimental verification
287 by Bekisli and Grenestedt (2004) of the behavior of the new balsa core with a grain orientation
288 of 45° showed a 30% higher average effective shear modulus while evidencing similar shear
289 strength to the end-grain balsa. Kepler (2011) provided a simpler concept for improving the
290 shear stiffness of balsa elements in which small strips of balsa wood were glued together at
291 alternating grain angles (see Fig. 5a). An approximately four-fold increase in the shear stiffness
292 was achieved when the grains were oriented at 45° compared to 90° with the failure strength
293 varying only moderately between the specimens.

294 Another approach to increasing performance in carrying applied loads is to modify the
295 geometrical orientation of FRP sandwich panels (Fig. 5b). Due to limitation in thickness of the
296 mass produced FRP sandwich systems with a phenolic core, Manalo et al. (2010c, 2013b) have
297 extensively investigated the behavior of glue-laminated beams made by bonding the sandwich
298 panels in the horizontal and vertical positions. They found that the beams laminated in the

299 vertical position were found to possess 25% higher bending strength (Manalo et al. 2010c) and
300 achieved over 200% higher shear strength (Manalo et al. 2013b) than in the horizontal position
301 suggesting a more effective use of the composite material. The introduction of the vertical fiber
302 composite skin inhibits the development of flexural and shear cracks in the core making the
303 FRP sandwich systems exhibit ductile failure behavior, which is important from the civil
304 engineering perspective.

305 Osei-Antwi et al. (2013; 2014a) optimised the load-bearing behavior of an FRP
306 sandwich deck by introducing multilayer core materials involving balsa with different densities,
307 i.e. structurally graded cores. In their concept, a high-density end-grain balsa was provided at
308 the upper face of FRP sandwich systems to prevent indentation and wrinkling and to provide
309 sufficient strength and stiffness in the deck's support region. Low-density balsa was used in the
310 less-stressed zones to minimize overall weight. Intermediate arch-shaped FRP laminates (Fig.
311 6) were then provided between the core layers to further increase the stiffness and strength. The
312 effectiveness of this core configuration was also assessed for FRP sandwich decks in bridge
313 construction (Onsei-Antwi et al. 2014b). They found a maximum span of approximately 19 m
314 long could be achieved with balsa-core sandwich-slab bridges with a deflection limit of
315 span/500, if a carbon FRP arch was integrated into the balsa core.

316 *Intermediate Layer*

317 The performance of FRP sandwich systems may be improved for specific applications by
318 introducing an intermediate layer between the top and bottom skins and the core material.
319 Mamalis et al. (2008) produced sandwich beams with an intermediate plywood layer between
320 the skin and the PVC foam core to improve the system's impact resistance (Fig. 7a). Fang et al.
321 (2015) developed sandwich beams containing an intermediate bamboo layer between the GFRP
322 skins and the wood core. In investigating the flexural behavior, they found that increasing the
323 thickness of the bamboo and GFRP layers significantly increased the system's stiffness and

324 ultimate load due to the good bonding of the intermediate bamboo layer to the GFRP skin and
325 paulownia wood core.

326 ***Hybrid Core Systems***

327 Hybrid core involves combining two or more different core materials to improve the structural
328 performance of FRP sandwich systems. Using this approach, Keller et al. (2008) combined PU
329 foam core of three different densities and strengths reinforced with orthogonal GFRP webs to
330 increase the shear strength of an FRP sandwich roof. Fam and Sharaf (2010) explored the
331 feasibility of fabricating and improving the performance of FRP sandwich systems with low-
332 density PU foam cores by providing internal and/or exterior GFRP ribs in the longitudinal
333 direction connecting the core and GFRP skins. They found that, depending on the rib
334 configuration, the flexural strength and stiffness of the sandwich panel could be increased by
335 44% to 140% compared to panels without ribs. Sharaf and Fam (2012) further estimated that
336 an increase in strength of up to 220% could be achieved if the ribs had an optimal spacing of
337 2.93 times the panel thickness, indicating the high potential of these sandwich composites for
338 structural applications. More recently, Mohamed et al. (2015) manufactured sandwich panels
339 made from PU foam core and E-glass fiber skins. Three types of sandwich panels with different
340 stiffeners and stitching orientations shown in Fig. 7b, i.e. closed cell (Type 1), trapezoidal shape
341 (Type 2), and web-core boxes (Type 3) were investigated. Their results showed that the
342 sandwich panel with trapezoidal shape stiffeners represents a feasible design for full scale
343 bridge decks.

344 **Case Studies and Field Applications of Sandwich Composites in Civil Engineering**

345 FRP sandwich systems have been effectively and economically applied for civil infrastructure
346 demonstrating their light weight and efficient load-carrying capacity. This section presents case
347 studies and several field applications of FRP sandwich composites.

348 ***Housing and Construction***

349 FRP sandwich composites offer promising material properties suitable for housing and
350 construction. A function-integrated GFRP sandwich roof structure for a main gate building
351 (Fig. 8) was designed and built in Switzerland by Keller et al. (2008). The face sheets of this
352 sandwich structure have thicknesses of 6 to 10.5 mm GFRP while the core is constructed using
353 up to 600 mm thick PU foam of three different densities in which a system of crossing GFRP
354 webs is inserted. The sandwich integrates structural, architectural and building physics
355 functions (thermal insulation, waterproofing and sound insulation). The prefabrication of large
356 and lightweight panels enabled easy transportation and rapid installation. Keller et al. (2010)
357 have furthermore proven the feasibility of encapsulating photovoltaic cells into almost
358 transparent GFRP skins, thus also providing energy supply functions.

359 The GFRP sandwich systems with ribs (Fig. 9) developed by Fam and Sharaf (2010)
360 were found to be applicable not only for thermal insulation but also as a structural cladding for
361 buildings. Their experimental investigation using large-scale sandwich panels (9.145 m high,
362 2.440 m wide, and 78 mm thick) under transverse loading showed that the sandwich composites
363 failed at 7.5 kPa or 2.6 times the factored design pressure for the windiest region in Canada
364 (Sharaf and Fam 2011). Furthermore, the deflection under the design wind pressure did not
365 exceed span/360.

366 ***Bridge and Pedestrian Decks***

367 The inherent advantages in strength and stiffness per unit weight as compared to traditional
368 steel-reinforced concrete decks make FRP sandwich decks a good alternative. As a result,
369 several variants of sandwich bridge decks, spanning transversely or longitudinally between
370 supporting elements (such as concrete, steel and timber beams) have been developed.

371 Keller et al. (2007) presented a new concept for a lightweight hybrid-FRP deck. This
372 deck had GFRP for the tensile skin, lightweight concrete for the core, and a thin layer of ultra-
373 high performance reinforced concrete for the compressive skin. Similarly, SAMPE (2010)

374 reported the first use of balsa cored FRP sandwich bridge deck (Fig. 10a) in Louisiana, USA.
375 In developing this bridge deck, high tensile strength steel reinforcement was combined with bi-
376 axial GFRP skins to achieve the required flexural stiffness. The core was then made up of end-
377 grain balsa wood with embedded fiber-optic strain gauges to monitor the long-term
378 performance of the deck. Composite Advantage LLC installed its FiberSPAN™ FRP bridge
379 deck on a new three-span steel superstructure at Wolf Trap National Park for the Performing
380 Arts in Virginia (United States). The deck was a moulded sandwich construction consisting of
381 thick GFRP top and bottom skins and a foam core with fiberglass shear webs in which the fibers
382 were oriented at $\pm 45^\circ$ angles (Reeve, 2013).

383 FRP sandwich decking also provides the opportunity to upgrade a bridge's load carrying
384 capacity. An FRP sandwich composites deck was used to replace the deteriorated concrete slab
385 on a bridge over Bennet's Creek in Steuben County, New York (Aref et al. 2005). Keller et al.
386 (2014) developed and installed the first GFRP-balsa sandwich bridge deck in Switzerland,
387 across the Avançon River in Bex. The new lightweight two-lane bridge replaced a one-lane
388 concrete bridge without increasing the total load on the stone abutments. The sandwich deck
389 was composed of three panels of 22-mm-thick GFRP face sheets and a 241-mm-thick balsa-
390 wood core, see Fig. 10b, manufactured with the vacuum infusion process. The balsa-wood
391 fibers in the core were oriented perpendicular to the face sheets to provide the required
392 resistance against indentation and shear. The panel-to-panel and deck-to-girder connections
393 were manufactured on site through an adhesive infusion process.

394 ***Bridge Beams***

395 The development of structural beams from FRP sandwich systems is gaining interest. Canning
396 et al. (1999) proposed a hybrid box section for beam application. The web of the beam is made
397 up of sandwich construction to prevent buckling with an upper layer of concrete in the
398 compression side. A similar structural concept was used by Primi et al. (2009) to build a new

399 FRP bridge in Spain wherein the beam's webs consisted of sandwich panels with polyurethane
400 core and glass-fiber skins produced by hand lay-up process. Likewise, the girders of the
401 Asturias Bridge in Spain have a trapezoidal cross-section and built by wrapping carbon fiber
402 prepreg around a stay in place polyurethane mould, thereby producing FRP sandwich bridge
403 systems (Hurtado et al., 2012). Each girder had been split in two trunks and was successfully
404 joined at the site using adhesive bonding.

405 Bridge beams made of FRP sandwich panels were developed in Australia for the
406 replacement of deteriorating timber girders (Van Erp and McKay, 2012). These beams
407 incorporated the sandwich panels oriented in the vertical position to provide the general shape,
408 shear strength and structural core of the girder, while the hybrid modules consisting of steel
409 reinforcing bars cast in pultruded FRP tubes provided additional flexural strength and stiffness
410 (Fig. 11). Furthermore, a solid glue-laminated sandwich panel was used mainly in the ends of
411 this beam for the drilling and installation of the fixing rods and to resist the high
412 compressive/crushing force at this location.

413 *Floating and Protective Structures*

414 The low weight, high-strength, and corrosion resistance of FRP sandwich systems make them
415 suitable for the development of floating and impact resistant structures. In China, floating and
416 energy absorptive elements made of FRP sandwich systems were developed for a collision-
417 avoidance structure. The outer shell of the sandwich structure is a thin GFRP skin while the
418 inside is a fiber-reinforced foam core (Liu et al. 2013). Theoretical investigation and full-scale
419 testing showed that the anti-collision structure around a bridge shown in Fig. 12a could reduce
420 the ship impact force by 40% (from 19.95MN to 13.16MN). There were more than 50
421 completed design projects, such as Fuzhou Wulong river bridge, Changzhou Xinmengge
422 bridge, Guangzhou-Shenzhen high-speed way along the Yangtze river bridge (Shenzhen
423 section), which used this structure. The concept of a floating collision-avoidance structure is

424 similar to the collision-protection/scarifying I-Lam system for concrete bridge girders shown
425 in Fig. 12b developed by Qiao et al. (2008). This system uses sandwich composites with a
426 crushable core. The system has smart sensors and actuators for remote sensing, triggering and
427 monitoring. Numerical simulations and full-scale impact tests on reinforced concrete beams
428 showed that about 60 to 70% of the kinetic energy was absorbed by crushing of the aluminium
429 core in the I-Lam system. 20 I-Lam panels were installed on the sides of a slab concrete bridge
430 (DEL-23-12.99) in Delaware, Ohio in 2006.

431 *Railway Sleepers*

432 A number of railway sleeper technologies have been developed using glued FRP sandwich
433 panels and combinations with other materials (Manalo and Aravinthan 2012a; Van Erp and
434 McKay 2013). These technologies are suitable for replacing and maintaining deteriorating
435 timber sleepers, including existing timber lines, turnouts, and transoms. One significant
436 advantage of FRP sandwich systems as railway sleepers is that they can be engineered to have
437 similar flexural stiffness to structural timber and to deform at the same magnitude as the existing
438 railway sleepers, which is very important in the sleeper maintenance works.

439 The timber-replacement sleeper made of glued FRP sandwich panels in the edgewise
440 position with top and bottom glass fiber reinforced polymer (GFRP) plates and coated with
441 epoxy-based polymer (Fig. 13a) was specifically designed to conform to the loading conditions
442 for mainline applications in which the sleeper is only loaded in two distinct locations (at the
443 rails) and does not need the same strength along its length. These sleepers can be drilled on site
444 similar to timber sleepers. Fifty units of this new sleeper have been installed in 2014 and are in
445 service on the standard railway line on the Queensland Rail Line (Australia). Sleeper
446 technologies with glue-laminated FRP sandwich panels as the main structural component were
447 also developed for railway turnouts and transoms. These sleepers have a prismatic rectangular
448 shape (Fig. 13b) and are reinforced with 2 layers of GFRP laminates at the bottom to enhance

449 flexural strength and stiffness. The composite transom sleepers are FRP sandwich panels
450 combined with steel reinforcement bars similar to the bridge beam concept in Fig. 11 to make
451 the sleepers exhibit ductile behavior when overloaded. This creates a very reliable structural
452 element that gives ample warning of failure. Australian Rail Track Corporation (ARTC)
453 installed twenty-two of these transoms in November 2007 on a railway bridge in the Hunter
454 Valley, Australia (Prasad, 2008). To date, the sleepers have been subjected to approximately
455 80 million load cycles and have been performing extremely well. They were recently approved
456 for general use on the ARTC rail network.

457 **Challenges and Opportunities**

458 While sandwich systems have emerged as a suitable solution in civil infrastructure, barriers still
459 need to be overcome for their continued acceptance and growing use. This section discusses the
460 emerging issues of FRP sandwich systems and presents future developments and opportunities
461 to accelerate their application in civil engineering and construction.

462 ***Core Material Development***

463 Beloultar et al. (2009) highlighted that the failure modes of the FRP sandwich systems depend
464 largely on the nature of the core material. As presented in Table 1, the commonly used foam
465 cores have low strength and modulus in shear and compression. As a result, many researchers
466 have dedicated significant efforts to develop and/or modify core material to produce sandwich
467 systems with higher performances (see Table 2). The localised strengthening and the
468 introduction of fibers to improve the properties of foam core involve a complex process such
469 as weaving or injection technology, which may further increase the production cost. It also
470 increases the amount of resin pickup during moulding, which adds to the panel's weight.
471 Similarly, the intermediate layer increases the amount of material used, which increases cost.
472 Furthermore, the increased stiffness in using a complex core structure is obtained at added
473 machining cost, and wasted materials. This approach also produces a core that is not

474 geometrically perfect, creating mismatches between blocks (Kepler, 2011). These important
475 aspects have to be addressed in developing new core materials in order to advance the use of
476 FRP sandwich systems in civil engineering applications.

477 The priority should be focusing on exploiting the potential of the core material for the
478 demanding requirements in civil infrastructure. Moreover, improving some aspects of the
479 properties in developing or modifying core material should not compromise the competitive
480 advantages of FRP sandwich systems. Attention should be paid to optimising core material
481 characteristics for stronger, lighter, more competitive, and more effective FRP sandwich
482 systems. For example, the effective use of existing core materials can be achieved by
483 strategically placing the high-strength core in locations where stress levels are high and low-
484 strength core in areas where stress levels are low. Similarly, different core material systems can
485 be combined together to achieve the desired stiffness and strength characteristics. Better
486 performing sandwich composites can be achieved even using simple concept and
487 manufacturing method but with some manipulation of the existing core material systems. From
488 the recently developed core material systems, the solution with the most promise seems to be
489 the use of FRP ribs/webs (hybrid cores) – it is cost effective and allows using cheaper and lower
490 strength core materials that are already available. These methods of core improvement can
491 result in FRP sandwich systems with higher structural performance while maintaining the
492 simplicity of the production process.

493 *Effective Joining Systems for FRP Sandwich Composites*

494 The design and manufacture of reliable joining systems is recognized by many researchers as
495 the major challenge in the development of FRP composite structures. This problem also exists
496 for FRP sandwich systems. Garrido et al. (2015a) highlighted that a significant part of the
497 current sandwich panel connection technology has been developed for non-structural or
498 secondary structural sandwich panels. While tongue and groove combined with adhesive

499 bonding is proven effective as panel-to-panel connection for pultruded FRP decks, Reising et
500 al. (2004) indicated that this can be an issue for FRP sandwich systems manufactured by
501 VARTM and hand lay-up process due to higher dimension tolerances. Moreover, this joining
502 technique may not be suitable for FRP sandwich systems with soft flexible foam cores and with
503 low shear properties as the adhesive needs to be applied to the contact surfaces between the
504 joints. Similarly, when joining adjacent sandwich panels with different core materials, Osei-
505 Antwi (2014c) highlighted the importance of appropriately joining core materials of different
506 properties to minimize material discontinuities and stress concentrations. For these
507 applications, node connections between sandwich panels are necessary. Similarly, FRP
508 sandwich systems with foam and balsa wood cores cannot directly support mechanical joints
509 such as bolts and rivets due to their low strength if they are to be connected to bridge girders or
510 other systems. In these cases, insert materials such as metal, stiffer foam core, wood patches or
511 polymeric materials can be used for local reinforcement (Zenkert 1995) to adequately transfer the
512 load to the FRP skins. Currently, only a few published works focus on inserts and primarily on
513 FRP sandwich systems in automobile and aerospace structures (Bozhevolnaya and Lyckegaard
514 2005). Likewise, the corner joints in lightweight honeycomb sandwich structures for aerospace
515 design presented by Heimbs and Pein (2009) can be adopted in panel-to-panel connections for
516 FRP sandwich systems used in structural walls. Clearly, the development of reliable joining
517 systems for FRP sandwich in civil engineering is important as they are subjected to high load
518 levels and their behavior should be determined to ensure the integrity of the entire structure.
519 Research activities in this area should be conducted to verify the efficiency of shear transfer
520 and constructability of inserts in FRP sandwich systems for civil infrastructure. Moreover,
521 investigation on the global behavior of sandwich composites with node connections made of
522 FRP sections shown in Fig. 14 should be assessed to determine reliable connection systems
523 between adjacent sandwich panels and at the corners. These connection techniques should be

524 adaptable for curved/free-form FRP sandwich panels for roof and facades, with varying skin
525 thickness and skew angles for bridge decks.

526 *Acoustic and Thermal Insulation Properties*

527 FRP sandwich systems are widely employed for sound absorption in various structural
528 applications including aircraft, spacecraft, automotive, and wind-turbine blades. When used in
529 building and construction, the sound and thermal insulation of sandwich composites are also
530 desirable (Patinha et al. 2015). However, information on the sound propagation of FRP
531 sandwich systems for civil engineering and construction are limited even though these materials
532 are already dominating in the commercial market (Zhu et al. 2014). This is because of the
533 natural difficulty in measuring this property both experimentally and numerically.

534 Patinha et al. (2015) indicated that FRP sandwich systems have superior noise reduction
535 capacity and have clear advantage over conventional panels and plasterboard materials of the
536 same weight due to their increased thickness. Zhu et al. (2014) identified the materials of the
537 core and skin, and the density, thickness and topology of the core as important design
538 parameters that determine the sound transmission loss in sandwich composites. While there is
539 an increasing attention on this topic, D'Alessandro et al. (2013) highlighted that the core
540 configuration is the most investigated parameter because it highly influences the acoustic
541 behavior. These authors further mentioned that the sound absorption coefficient of sandwich
542 composites increase as the density of the core material becomes higher. Similarly, the thickness
543 and the density of the core play a major role in the thermal insulation properties of FRP
544 sandwich systems. Kawasaki and Kawai (2006) indicated that insulation materials must be of
545 sufficient thickness and low density to provide high thermal resistance. These researchers found
546 that moderate (340 kg/m^3) to high density (410 kg/m^3) fiberboard core materials have high
547 warmth-keeping properties. The thermal conductivity of these sandwich panels are only 0.070
548 and $0.077 \text{ W/m}^\circ\text{K}$, respectively, which are suitable for building construction. Moreover, the

549 Federation of European Rigid Polyurethane Foam Associations (2006) reported that PU foam
550 is one of the most efficient, high performance thermal insulation materials. This material has a
551 thermal conductivity of around 0.023 W/m[°]K. In fact, they suggested the use of sandwich panels
552 with PU foam in buildings can cut average energy consumption by more than 50%. Moreover,
553 the results of their 15-year tests showed very minimal decrease in the thermal insulation
554 properties of PU foam.

555 Sound transmission and heat insulation may not be a significant issue for FRP sandwich
556 systems used in civil infrastructure as in this application, high density and thick core materials
557 are normally utilised for high load-bearing capacity. However, the lightweight of FRP sandwich
558 systems with low density foam core is a drawback as it results in poor acoustic performances
559 and may have negative effects on physiological and psychological health of building occupants.
560 This is also a concern for resin rich FRP sandwich systems as the sound absorption behavior of
561 the resin systems is low. As large variety of FRP sandwich systems arising from different
562 materials and geometrical combinations becomes available, the acoustic properties cannot be
563 neglected when designing them for use in housing and building construction. Similarly,
564 comprehensive evaluation of the thermal properties of the current variety of core materials
565 should be conducted to open up a broad field of applications for FRP sandwich systems.

566 ***Time Dependent Behavior***

567 FRP sandwich systems in civil infrastructure will support significant permanent loads as well
568 as will subject to repetitive loads. However, the creep performance and fatigue behavior of FRP
569 sandwich systems for civil infrastructure has received limited attention to date.

570 Barbosa (2014) indicated that the structural form of sandwich composites makes them
571 susceptible to creep. The primarily reason for this is the nonlinear viscoelastic behavior of
572 polymeric foams especially at higher levels of load. They suggested that PET sandwich panels
573 will exhibit lower creep deformation than PU sandwich panel due to their lower viscoelastic

574 properties. Jeon et al. (2014) found that sandwich beams with foam core and GFRP skins,
575 wherein the fibers are oriented mostly in the longitudinal (0°) direction experience lesser creep
576 deformation than sandwich beams with skins oriented in 45° and 90° due to the dominating
577 response of polymeric constituents. Similarly, Garrido et al. (2013) noted a 65% increase in the
578 deflection due to creep of the sandwich beam with PU core and GFRP skins after 5 months
579 under an applied load of 5.2 kPa (or a load level inducing 20% of the core shear strength).
580 Moreover, they highlighted the importance of considering the shear deformation in the creep
581 behavior of sandwich composites as it contributes up to 70% of the total deformation. Thus,
582 understanding the response of sandwich structures for a prolonged loading in time is critical as
583 the design of FRP for civil infrastructure is normally governed by serviceability rather than
584 strength. Moreover, simple models to account for the long-term effect of creep in the
585 deformation of FRP sandwich systems are needed.

586 Fatigue is a critical design concern in response to repetitive loads (Osgood 1982), but
587 only a few studies have investigated the behavior of FRP sandwich systems under fatigue.
588 Dawood et al. (2010) found that sandwich composites with stiffer cores under fatigue loading
589 generally evidenced greater degradation than those with lighter cores due to the higher induced
590 shear stresses at the same level of applied shear strain. Mathieson and Fam (2014a) indicated
591 that the fatigue life of sandwich composites with GFRP skins and a polyurethane foam core
592 was significantly short under full unloading and fully reversed loading. This also occurred with
593 the addition of internal ribs (Mathieson and Fam 2015a). Moreover, a reduction of up to 25%
594 in flexural stiffness of the sandwich composites was observed when 45% of the failure load was
595 applied for 2M cycles of fatigue loading (Mathieson and Fam 2014b). Consequently, the
596 authors recommended that the maximum applied load should not exceed 35% and 21% of the
597 ultimate static strength for FRP sandwich systems with ribs (Mathieson and Fam 2015a), and
598 30% and 45% for those without ribs (Mathieson and Fam 2014a, Mathieson and Fam 2014b)

599 under full unloading and under fully reversed loading, respectively, in order to achieve a fatigue
600 life of 2M cycles. The fatigue tests on full-scale GFRP-balsa beams, including an adhesive lap
601 joint at mid-span, for the Avançon bridge under a quasi-static load varied between 21 kN and
602 118 kN, which are 10% and 60% of the failure load, respectively (Keller et al., 2014) returned
603 reassuring results by showing no signs of damage during 5 million cycles. This corresponds to
604 the bridge's 100-year service life. These results highlight the importance of understanding the
605 property retention and failure behavior performance of emerging FRP sandwich systems under
606 fatigue loading for simulating and designing structures subjected to moving loads, wind
607 pressure and suction, and hydraulic forces.

608 *Durability*

609 Primary load-bearing FRP sandwich systems are a relatively new technology in civil
610 engineering applications. As a result, their performance history is relatively short compared to
611 more conventional construction systems utilising hardwood, concrete or steel. Hollaway (2010)
612 identified elevated temperature, fire resistance, ultraviolet-light, and ingress of alkalis or other
613 liquid as some of the most important in-service properties that should be considered when using
614 FRP composite materials in civil engineering applications. A number of researchers showed
615 that glass, carbon and basalt fibers perform reasonably well in harsh environmental conditions
616 (Benmokrane et al. 2015). Most studies conducted to investigate the durability performance of
617 FRP sandwich systems have been, however, related to aerospace and naval applications. Li et
618 al. (2014) confirmed that the incidence of face yield in sandwich composites increased as did
619 the temperature. Similarly, moisture uptake in moist environments affects the mechanical
620 behavior of sandwich composites. Foam shrinks at high temperature, while face sheets
621 delaminate from the inner core as a result of moisture uptake (John et al. 2011). Yin et al. (2015)
622 also observed a considerable decrease in modulus with an up to 45% decrease in flexural
623 strength for polymethacrylimide (PMI) foam-core sandwich composites immersed in seawater

624 at 70°C. In probably one of the first attempts at using bio-fibers in sandwich composites for
625 structural applications, Mak et al. (2015b) found that the tensile strength retention of flax-FRP
626 skins after 300 days of exposure in saltwater was 81%. Based on the Arrhenius model, it was
627 estimated that these skins would retain 60% of their tensile strength after 100 years at an annual
628 mean temperature of 10°C. In a case involving actual field conditions, Reising et al. (2004)
629 found delamination of the top skin of a FRP sandwich bridge deck exposed to direct sunlight
630 and harsh winter conditions as a result of the small, relatively fragile connection between the
631 face and honeycomb core.

632 As in most civil engineering applications, FRP sandwich systems are likely to be
633 subjected to fire and elevated temperature. Bai et al. (2014) strongly recommended that the
634 structural adequacy and integrity shall be satisfied in order to successfully implement FRP
635 composites in structures subjected to relatively high in-service temperature. It is well known
636 that under high temperature, FRP composites may undergo a decrease in stiffness and strength
637 as the resin/matrix binding the fibers will soften resulting in functionality loss of the structure
638 (Mouritz and Gibson 2007). Specifically, FRP composites undergo significant changes at a
639 temperature higher than the glass transition temperature, T_g . Van Erp (2008) reported that
640 depending on the cure schedule, the T_g of the matrix is between 60°C and 110°C for epoxy; 60
641 and 120°C for vinylester; and 85 and 125°C for polyester. Significant degradation of most
642 polymeric foam core's properties occurs at much lower temperature than the T_g of fiber
643 composites which can result in loss of stiffness and strength of FRP sandwich systems even at
644 a temperature within the in-service condition. Taher et al. (2013) reported a 50% decrease in
645 the compressive strength and modulus of PVC foam when exposed to 85°C. Moreover, Garrido
646 et al. (2015b) found a considerable reduction in shear modulus in PET and PU foams used in
647 FRP sandwich systems. At 80°C, the PET foam with density of 94 kg/m³ and PU foam with
648 density of 68 kg/m³ only retained 24% and 66%, respectively of their shear moduli from

649 ambient temperature. Adding to this issue, Taher et al. (2013) highlighted that there exist
650 limited and incomplete information about the temperature dependence of the core properties
651 for FRP sandwich composites.

652 The information from the above studies can be used as a preliminary indication of the
653 durability performance of sandwich systems when used in civil infrastructure. Nevertheless, the
654 performance of FRP sandwich systems under the actual and simulated conditions that are
655 expected for such types of structures should be continuously investigated to ensure that these
656 systems perform satisfactory during their service lives. There is also a great need to ascertain
657 the thermal degradation behavior of the existing and emerging FRP sandwich systems when
658 subjected to combined thermal and structural loads if they are to be used in load-bearing
659 applications. A detailed understanding of how these factors affect the long-term durability of
660 FRP sandwich systems is very important, since it will provide the guidance for their effective
661 design and use in developing sustainable infrastructure. These issues are being addressed and
662 the current pace of development means that solutions are rapidly being found and there is no
663 doubt that sandwich composites will be suitable systems for lightweight civil infrastructure.

664 *Design Oriented Analysis*

665 Bakis et al. (2002) pointed out that without an established design method and data, it is unlikely
666 that FRP-based materials will be used as a construction material beyond the scope of research
667 and demonstration projects. While the Queensland Government (2009) has made an effort to
668 promote composite materials in various industry sectors by providing an introductory guide,
669 the application of sandwich composites in construction has been limited. Currently, there are
670 no Australian Standards and provisions in the Building Code Australia on the strength and
671 performance requirements for FRP sandwich systems. In contrast to this, designers and builders
672 in Europe have started to use sandwich composite panels in structural systems, due to the
673 availability of design guidelines. An example is the document 'European Recommendation for

674 Sandwich Panels, Part I: Design' (Davies, 2000) which has been developed for the design of
675 sandwich composites with metal faces and various types of foam core. Thus, Manalo and
676 Aravinthan (2010b; 2012b) proposed simplified analysis methods to describe the approximate
677 flexural and shear behavior of FRP sandwich systems consisting of GFRP skins and a phenolic
678 core. Similarly, Fam and Sharaf (2010) and Mathieson and Fam (2015b) invested significant
679 efforts in developing simplified analytical tools for sandwich composites with GRFP skins and
680 polyurethane cores. In their proposed model for sandwich composites under in-plane bending,
681 a stiffness-based approach incorporating both flexural and shear rigidities of the core was
682 adopted to establish the load-deflection response while skin wrinkling criterion was used to
683 establish ultimate load. The flexural and shear stiffness of the soft core were also considered in
684 calculating the critical buckling load under axial load for sandwich composites with various
685 slenderness ratios and cross-sectional configurations (Mathieson and Fam 2015c). The
686 comparison and validation of the simplified analytical models showed good agreement with
687 experimental results.

688 The simplified methods described above have been suggested as very useful for
689 scientific research and engineering calculations. Moreover, they are straightforward and easy
690 for designers and engineers to use, but with the capability of reasonably predicting the structural
691 performance of sandwich composites. The suitability of simplified design approaches should
692 therefore be validated for sandwich composites with other core material systems and geometries
693 before these methodologies can be adopted/and or modified for the general design of FRP
694 sandwich systems. Moreover, these simplified approaches can be used to develop failure-
695 mechanism maps as proposed by Gibson and Ashby (1988) by calculating the active collapse
696 mode based on a given geometry and material systems combination. Several researchers
697 (Steeves and Fleck 2004; Gdoutos and Daniel 2008; Kazemahvazi et al. 2009) have used this
698 approach and developed collapse mechanism maps for sandwich composites with different

699 material systems. These maps allow appropriate design criteria to be developed as a function
700 of the design variables critical for sandwich composites, such as loading conditions, face and
701 core dimensions, overall sandwich dimensions, and the skin and core properties, which can be
702 used handy by designers and engineers. However, unique design challenges specific to FRP
703 sandwich systems including significant shear deformations and the evolution of new
704 characteristic modes of failure should not be neglected in the development of simplified design
705 and analysis tools. In many cases, the failure of a sandwich structure involves delaminations,
706 debonding, and disintegration of the layered panel. Another group of failure modes is governed
707 by geometrical nonlinearity, evolution of instabilities, and wrinkling or buckling of the
708 compressed face sheet followed by global failure of the panel. A third group of failure modes
709 is involved with cracking of the core. In all cases, those modes of failure are typically brittle,
710 sudden, and abrupt and they are significantly different than the classical ductile modes of failure
711 attributed to steel or well detailed reinforced concrete structures. The formation of such
712 characteristic and, in many cases, undesirable modes of failure have to be reflected by design
713 procedures and partial safety factors. Moreover, the environmental degradation, fatigue
714 loading, temperature effect, and the level of maintenance and inspection of in-service structures
715 should be accounted in developing design methods and analytical procedures.

716 ***Systems Proof and Performance Evaluation***

717 More typical infrastructure prototypes need to be built to demonstrate practical applications,
718 increase acceptance, and build a market volume of new and emerging FRP sandwich systems.
719 Reising et al. (2004) highlighted the importance of demonstration projects in gaining a better
720 understanding of particular installation issues, connection details, and construction techniques
721 as well as of the long-term performance of composite structures. Moreover, Keller et al. (2008)
722 indicated that the safety factors could be adapted or reduced during the different design stages,
723 from the preliminary to the project execution, based on the results of small-scale materials

724 testing. This approach would be very helpful in achieving a more widespread use of FRP
725 sandwich systems, as the tendency in the industry is to design to suit particular applications,
726 often relying on past experience and guidance from the results of the comprehensive design,
727 experimental programs, and field performances. For new and emerging FRP sandwich systems,
728 the confirmation testing of assemblies' representative of the actual structure would also be an
729 important step in ensuring their compliance with existing design requirements and
730 specifications. Keller et al. (2008) and Keller et al. (2014) have satisfactorily shown that
731 experimental validation of the concept and design in compliance with existing codes for FRP
732 will result in the successful implementation of infrastructure projects, even given the lack of
733 design standards for sandwich composites. Furthermore, load testing and long-term monitoring
734 will help ensure the satisfactory in-service performance of sandwich composites, verify all
735 design assumptions, improve detailing and field installation techniques, and help optimize the
736 product in future applications. This would also help to physically demonstrate to asset owner
737 and other interested parties the effectiveness and suitability of FRP sandwich systems in
738 carrying the design load and complying with the deflection requirements. Inspection programs
739 covering such aspects as dimension tolerances, presence and extent of defects/delaminations,
740 curing of the installed system, adhesion, plate thickness, fiber alignment, and material
741 properties should be put into place. The information that will be obtained from performance
742 evaluation testing would be also helpful in developing design specifications. The availability
743 of these documents will give professional engineers and contractors more confidence in
744 designing and field implementing FRP sandwich systems in civil engineering and construction.

745 **Conclusions**

746 FRP sandwich composites provide an effective alternative in lightweight civil infrastructure
747 when the need for corrosion resistance, high strength, reduced weight, thermal insulation, or
748 fast installation is a driver for the system. Their flexibility in design allows innovative structural

749 developments from these composite systems to suit various structural applications. Based on a
750 comprehensive state-of-the-art review, the following conclusions have been drawn with the aim
751 of expanding the application of FRP sandwich systems in civil infrastructure:

- 752 • The structural behavior of FRP sandwich systems results from the combination of their
753 parts. FRP materials have emerged as excellent alternatives to metallic skins for
754 sandwich composites in structural engineering applications. The key to the system's
755 success is the core stiffness and arrangement.
- 756 • The evolution of FRP sandwich systems with enhanced core material provides an
757 opportunity to expand the application of these systems in civil infrastructure. Cellular
758 manipulation, fiber reinforcement, modification of the core geometry, introduction of
759 an intermediate layer between the skin and the core, and hybrid core materials were
760 found to be effective methods in improving the core structure and enhancing the
761 structural performance of FRP sandwich systems.
- 762 • FRP sandwich systems have been successfully used for or have a strong potential to be
763 applied in multi-functional roofs, cladding and roofing systems for buildings,
764 footbridges and bridge decks, railway sleepers, bridge beams, and floating and
765 protective structures. These innovative applications exploit the many advantages of FRP
766 sandwich systems and can provide the construction industry with more durable and cost-
767 effective infrastructure.
- 768 • Reliable and adaptable joining techniques for connecting FRP sandwich component,
769 panel and structure are important to ensure the integrity of the entire structure. The
770 connection method should provide a high-strength and durable joint that is easy to install
771 and maintain.
- 772 • Performance testing of FRP sandwich systems under actual and simulated conditions
773 should be continuously carried out to ensure that they will satisfactory perform during

774 their service lives. Understanding acoustic, creep, fatigue, thermal and degradation
775 performance of existing and emerging FRP systems especially those consisting of bio-
776 based materials is critical if they are to be used in load-bearing applications.

777 • Design procedures which account for environmental degradation, creep, fatigue
778 loading, temperature effect, and the level of maintenance and inspection of in-service
779 structures will help gain wider acceptance of FRP sandwich systems in various
780 engineering applications. Unique design challenges specific to these systems include
781 the significant shear deformations that should not be neglected as in conventional
782 structures, and the brittle mode of failure. Moreover, systems proof and performance
783 evaluation will ensure satisfactory in-service performance of FRP sandwich systems,
784 improve detailing and field installation techniques, connection details, and help
785 optimize these systems in other civil engineering applications.

786 Such an approach could expedite the implementation of FRP sandwich structures in new design
787 and construction of lightweight civil infrastructure through a more functional and economical
788 design.

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List of Tables

Table 1. Properties of balsa wood and foam core material systems

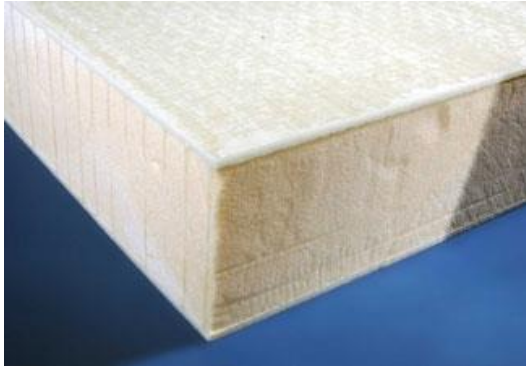
Table 2. Recently developed sandwich composites

Table 1. Properties of balsa wood and foam core material systems

Core Material	Density, kg/m³	Shear Strength, MPa	Shear Modulus, MPa	Compressive Strength, MPa
End-grain balsa wood	96 - 250	1.85 - 4.94	108 - 312	6.5 - 26.6
PU foam	21 - 400	0.15 - 3.1	1.55 - 104	0.2 - 0.35
PVC foam	30 - 400	0.35 - 4.5	8.3 - 108	0.3 - 5.8
PET foam	70 - 200	0.5 - 1.8	13 - 50	0.75 - 3.6
Phenolic foam	855	8.8	530	21.3

Table 2. Recently developed sandwich composites

Category	Existing Core	Modification	Enhanced Properties	Manufacturing method	Researchers
CM	Phenolic foam	Chemically modified	Shear, tension, and compression	Co-curing	Van Erp and Rogers (2008); Manalo et al. (2013a)
CS	PVC foam	Corrugated skin	Shear	Wet lay-up	Kampner and Grenestedt (2007)
FR	Foam	3-D fibre stitch	Shear and compression	VARTM	Reis and Rizkalla (2008)
GA	Balsa wood	Complex shape	Shear	Wet lay-up	Grenestedt and Bekisli (2003); Bekisli and Grenestedt (2004)
	Balsa wood	Alternate grain angles	Shear	Wet lay-up	Kepler (2011)
	Phenolic core	Sandwich orientation	Shear and flexure	Adhesive bonding	Manalo et al. (2010); Manalo et al. (2010b); Manalo et al. (2010c)
	Balsa wood	Multi-layer and mixed densities	Shear	VARTM	Osei-Antwi et al. (2013); Osei-Antwi et al. (2014a)
IL	PVC foam	Intermediate plywood layer	Impact	Adhesive bonding	Mamalis et al. (2008)
	Paulownia wood	Intermediate bamboo layer	Bond strength	VARTM	Fang et al. (2015)
H	PU foam	Mixed densities with orthogonal GFRP webs	Shear	VARTM	Keller et al. (2008)
	PU foam	Internal ribs	Flexure	VARTM	Fam and Sharaf (2010)
	PU foam	Stiffened core	Shear	VARTM	Mohamed et al. (2015)



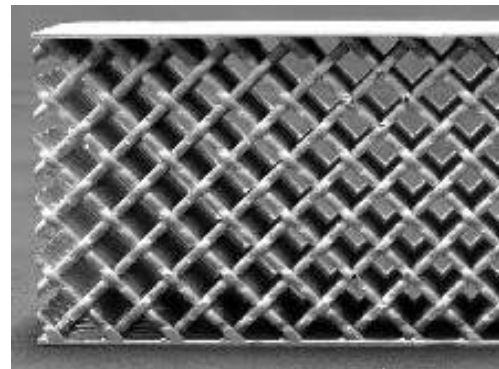
(a) Foam core



(b) Balsa wood



(c) Honeycomb core



(d) Trussed core

Fig. 1. Commonly used core material systems

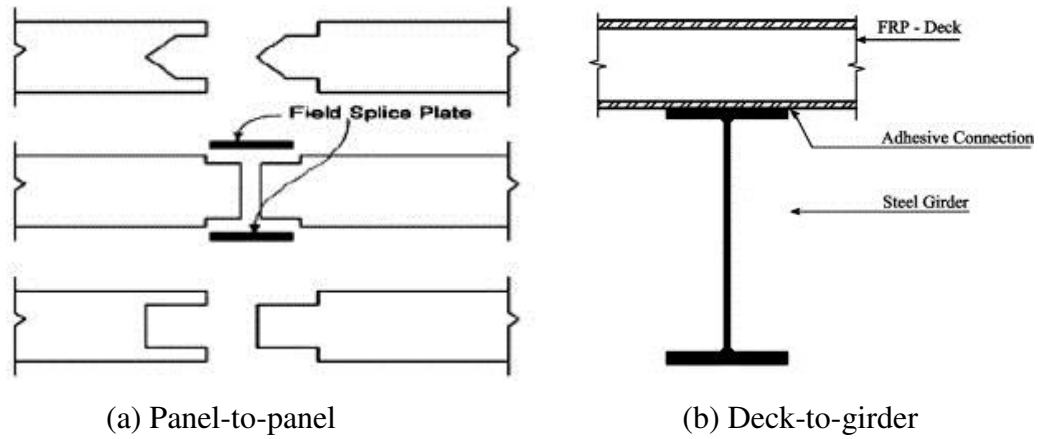
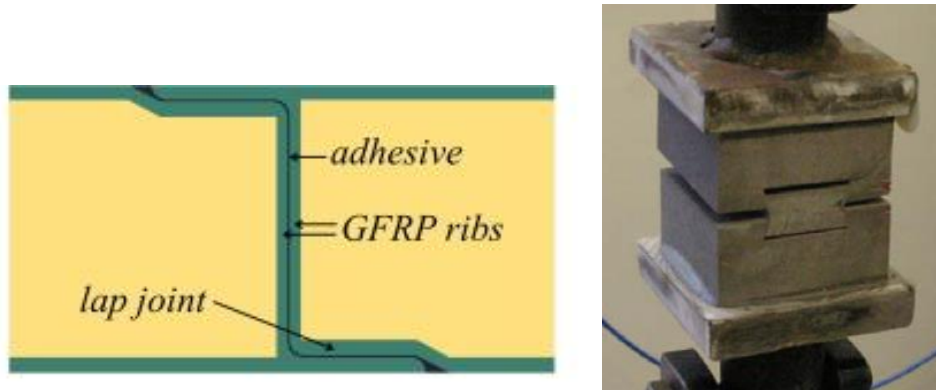


Fig. 2. Joining techniques for FRP decks (Zhou and Keller 2005)



(a) Z-shaped adhesive joint (Garrido et al. 2015) (b) Shear key (Manalo 2013)

Fig. 3. Joining systems for adjacent sandwich panels

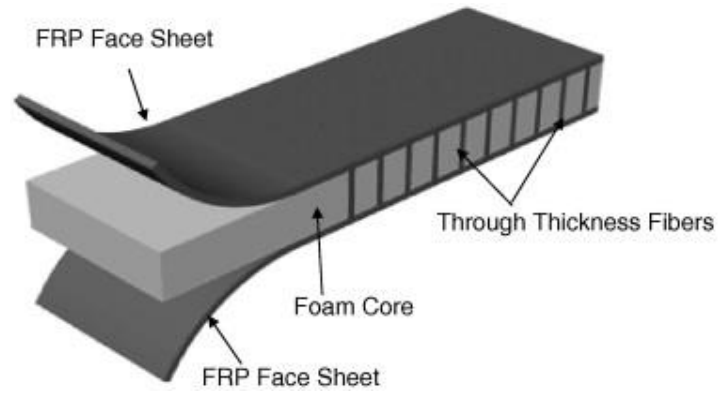
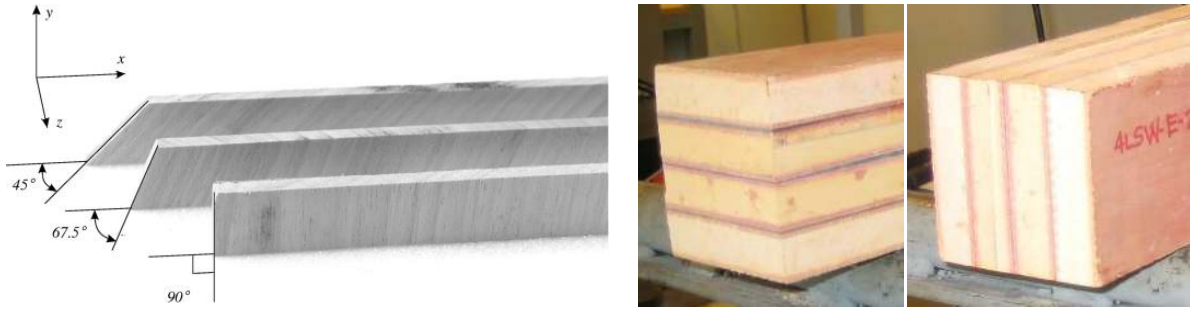


Fig. 4. Fiber reinforced core material systems (Reis and Rizkalla, 2008)



(a) Alternating grain angles (Kepler 2011) (b) Glued sandwiches (Manalo et al. 2010c)

Fig. 5. Geometrically arranged core material

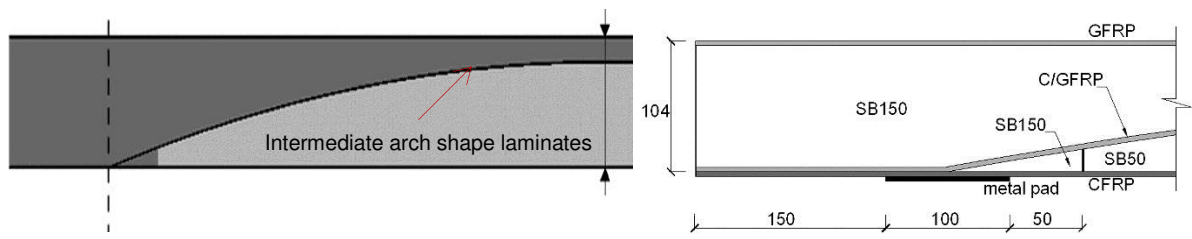
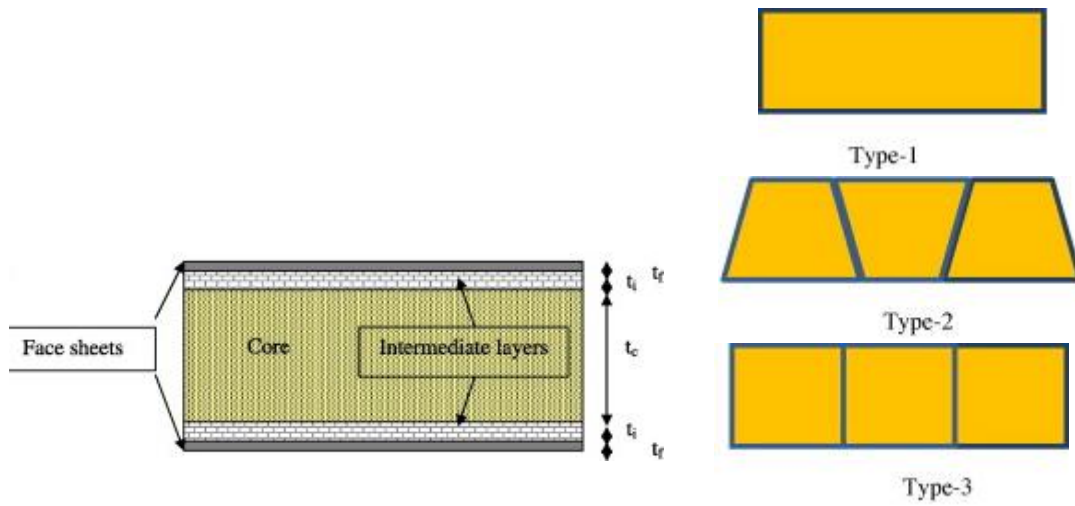


Fig. 6. Sandwich beam with intermediate arch-shaped FRP laminates (Osei-Antwi et al. 2013)



(a) Intermediate layers (Mamalis et al. 2008) (b) Core stiffeners (Mohamed et al. 2015)

Fig. 7. FRP sandwich system with intermediate layers and hybrid core systems



Fig. 8. Lightweight GFRP sandwich roof (Keller et al., 2008)

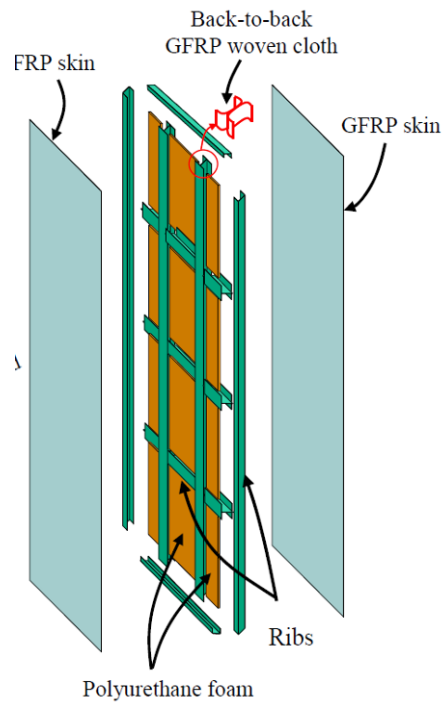


Fig. 9. FRP sandwich walls with ribs (Fam and Sharaf, 2010)



(a) balsa cored sandwich (SAMPE, 2010) (b) GFRP-balsa sandwich (Keller et al., 2014)

Fig. 10. FRP sandwich bridge decks

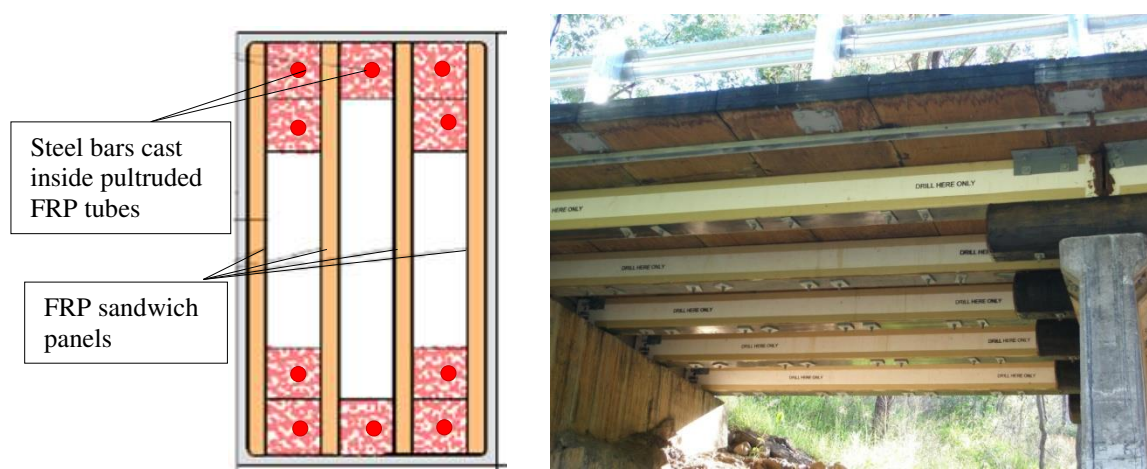
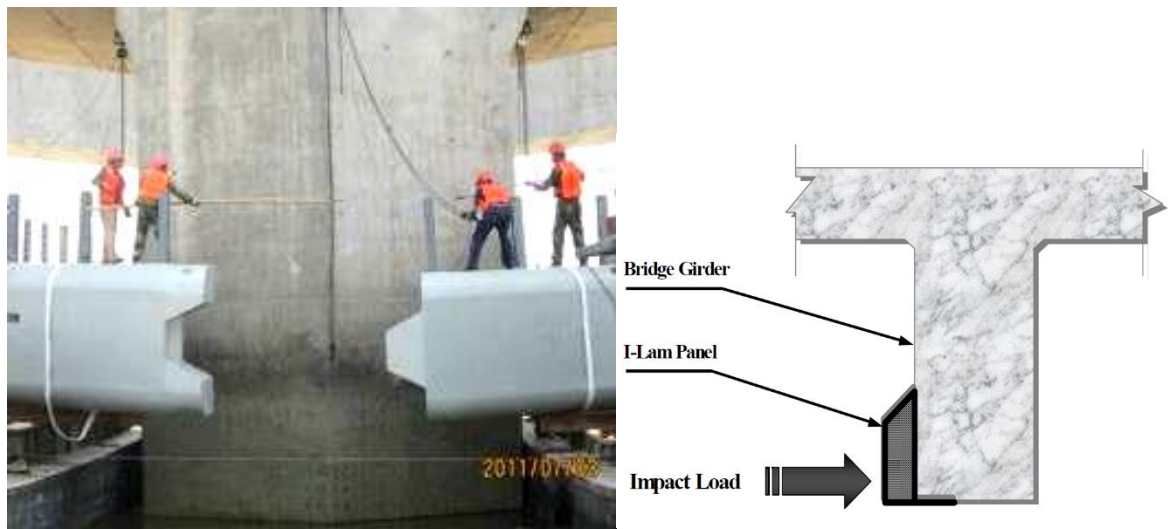


Fig. 11. FRP sandwich bridge beams (Van Erp and McKay, 2012).



(a) Collision-avoidance system (Liu et al., 2013) (b) I-Lam system (Qiao et al. 2008)

Fig. 12. Floating and protective structures using FRP sandwich systems



(a) Timber replacement sleeper

(b) Turnout sleepers

Fig. 13. Railway sleeper technologies using FRP sandwich panels

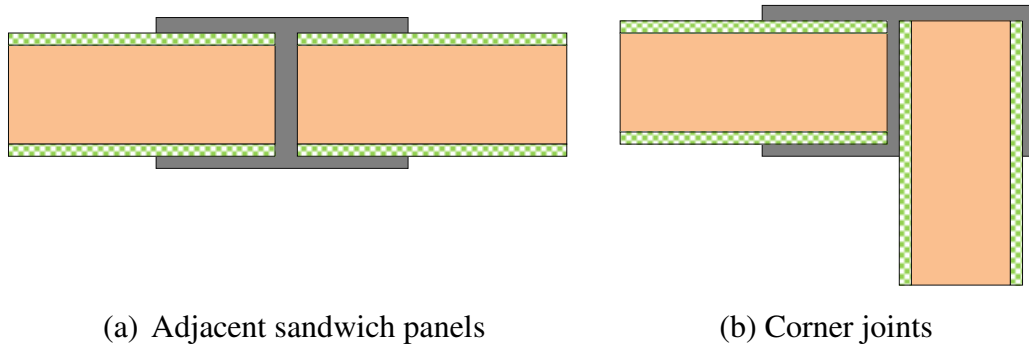


Fig. 14. Design concepts for FRP sandwich node connections

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