

Structural testing and design of wire arc additively manufactured square hollow sections

Pinelopi Kyvelou¹, Cheng Huang², Leroy Gardner³, Craig Buchanan⁴

¹Research Associate, Dept. of Civil and Environmental Engineering, Imperial College London, South Kensington Campus, London SW7 2AZ, UK. Email: pinelopi.kyvelou11@imperial.ac.uk

²Ph.D. Candidate, Dept. of Civil and Environmental Engineering, Imperial College London, South Kensington Campus, London SW7 2AZ, UK (corresponding author). Email: cheng.huang118@imperial.ac.uk

³Professor, Dept. of Civil and Environmental Engineering, Imperial College London, South Kensington Campus, London SW7 2AZ, UK. Email: leroy.gardner@imperial.ac.uk

⁴Lecturer, Dept. of Civil and Environmental Engineering, Imperial College London, South Kensington Campus, London SW7 2AZ, UK. Email: craig.buchanan@imperial.ac.uk

ABSTRACT

Wire arc additive manufacturing (WAAM) is a method of metal 3D printing that has the potential for significant impact on the construction industry due to its ability to produce large parts, with reasonable printing times and costs. There is currently however a lack of fundamental data on the performance of structural elements produced using this method of manufacture. Seeking to bridge this gap, the compressive behavior and resistance of WAAM square hollow sections (SHS) are investigated in this study. Testing reported in a previous study by the authors of sheet material produced in the same manner as the studied SHS is first summarized. The production, measurement and testing of a series of stainless steel SHS stub columns are then described. Regular cross-section profiles were chosen to isolate the influence of 3D printing and enable direct comparisons to be made against equivalent sections produced using traditional methods of manufacture. A range of cross-section sizes and thicknesses were considered to achieve variation in the local cross-sectional slenderness of the tested specimens, allowing the influence of local buckling to be assessed. Repeat tests enabled the variability in response between specimens to be evaluated; a total of 14 SHS stub columns of seven different local slendernesses was tested, covering all cross-section classes of AISC 370 and Eurocode 3. Advanced non-contact measurement techniques were employed to determine the as-built geometric properties, while digital image correlation measurements were used to provide detailed insight into the deformation characteristics of the test specimens. Owing to the higher geometric variability of WAAM relative to

34 conventional forming processes, the tested 3D printed stub columns were found to exhibit
35 more variable capacities between repeat specimens than is generally displayed by stainless
36 steel SHS. Comparisons of the stub column test results with existing structural design rules
37 highlight the need to allow for the weakening effect of the geometric undulations that are
38 inherent to the WAAM process, in order to achieve safe-sided strength predictions.

39 **Keywords:** 3D printing; additive manufacturing; digital image correlation; experiments; laser
40 scanning; structural engineering; stub column testing; wire arc additive manufacturing

41 1. INTRODUCTION

42 Additive manufacturing (AM), also referred to as 3D printing, is a fabrication process where
43 a part is formed through the sequential deposition of layers of material, as dictated by a 3D
44 digital model. The main advantage of this novel method of manufacturing, the use of which
45 has already substantially spread in the aerospace, automotive and biomedical industries
46 (Campbell *et al.*, 2012), is the ability to fabricate parts of complex geometry without the need
47 for specific tooling (Hague *et al.*, 2004). According to ISO/ASTM 52900 (2015), the
48 principal types of metal AM are sheet lamination, powder bed fusion (PBF) and directed
49 energy deposition (DED). Wire arc additive manufacturing (WAAM) is a method of DED
50 using welding technology, where wire feedstock is melted and selectively deposited onto a
51 substrate plate; the deposited material subsequently solidifies and the desired component is
52 formed layer by layer – see Figure 1 (PAS 6012, 2020). WAAM has the potential for
53 significant impact on the construction industry since it can be used for the production of
54 large-scale parts while allowing for high deposition rates, good structural integrity,
55 reasonable costs and reduced waste material compared to conventional manufacturing
56 processes (Buchanan and Gardner, 2019; Williams *et al.*, 2015). Although other metallic AM
57 methods can achieve higher geometrical complexity and accuracy, WAAM allows reduced
58 lead times and manufacturing costs (Lockett *et al.*, 2017), using mature technology and wire
59 feedstock of low cost (Thompson *et al.*, 2016). Finally, design freedom and printing
60 efficiency can be further enhanced by incorporating multi-axis robotic arms and by adopting
61 multi-direction slicing methodologies, which allow material deposition along multiple
62 directions, thus eliminating the need for supporting structures (Ding *et al.*, 2015; Zhang and
63 Liou, 2013).

64 Although WAAM is offering a revolutionary potential for the construction industry, reliable
65 design guidance on the structural behaviour of metal 3D printed structures is required to

66 enable integration of this new technology to the construction sector. Thus far, structural
67 engineering research has been focused on structural elements printed by other AM methods
68 (Yan *et al.*, 2019; Chen *et al.*, 2018; Buchanan *et al.*, 2017; Yasa, 2011) while experimental
69 data on WAAM structural elements are currently scarce (Laghi *et al.*, 2019; Ji *et al.*, 2017;
70 Haden *et al.*, 2017).

71 The world's first large-scale demonstrator of WAAM for structural applications is the
72 stainless steel 3D printed bridge shown in Figure 2, constructed by the Dutch start-up
73 company MX3D. The bridge has an overall mass of approximately 7.8 tonnes (of which
74 approximately 4.6 tonnes was printed at a typical deposition rate of 0.5-2.0 kg/h), a span of
75 about 10.5 m and an average width of about 2.5 m (Gardner *et al.*, 2020). Being the first of its
76 kind, and featuring material properties and structural behavior beyond the scope of existing
77 design specifications, this novel structure has required extensive experimental and numerical
78 research for its safety to be demonstrated.

79 A comprehensive experimental programme, comprising material (Kyvelou *et al.*, 2020) and
80 cross-sectional tests on tubular sections, has been conducted to support the construction and
81 verification of the MX3D bridge, as well as to explore the potential for wider application of
82 this technology. All tested specimens were printed by MX3D utilising the same feedstock
83 material and printing parameters that were used for the bridge. The destructive experiments
84 were undertaken in the Structures Laboratory in the Department of Civil and Environmental
85 Engineering at Imperial College London while extensive non-destructive physical testing of
86 the bridge has also been undertaken (Gardner *et al.*, 2020).

87 In this paper, cross-sectional tests on 14 WAAM stub columns of square hollow section
88 (SHS) members, printed using the same material and production parameters as the MX3D
89 bridge, are presented. The process followed for the production, measurement and testing of
90 the stub columns is described while the test results are analysed and discussed. Finally,
91 comparisons are made against the strength predictions of current structural design
92 specifications (AISC 370, 2020; EN1993-1-4, 2020) and against the performance of AM PBF
93 (Buchanan *et al.*, 2017) and conventionally formed austenitic (Chen *et al.*, 2018; Yuan *et al.*,
94 2014; Gardner and Nethercot, 2004; Rasmussen, 2000; Rasmussen and Hancock, 1993),
95 ferritic (Arrayago *et al.*, 2016; Afshan and Gardner, 2013) and duplex (Chen *et al.*, 2018;
96 Yuan *et al.*, 2014; Theofanous and Gardner, 2009) stainless steel SHS.

97 2. MATERIAL TESTS

98 In order to determine the stress-strain characteristics of the WAAM material, a
99 comprehensive series of tensile coupon tests was undertaken in a previous study by Kyvelou
100 *et al.* (2020); the key aspects of this study are summarized herein. Dog-bone shaped coupons
101 were extracted from WAAM plates at 0° , 45° and 90° to the printing direction, as defined in
102 Figure 3, to investigate the material anisotropy. The influence of the geometric undulations
103 resulting from the WAAM process on the effective material properties was assessed by
104 testing both as-built and machined coupons; for the machined coupons, all surface
105 undulations were removed using an end mill prior to testing. In total, 39 as-built and 12
106 machined coupons were tested.

107 A summary of the average material properties by printing direction (i.e. 0° , 45° and 90°) for
108 the as-built and machined coupons is presented in Tables 1 and 2, respectively, where θ is the
109 direction of testing relative to the print layer orientation as defined in Figure 3, t_{nom} is the
110 nominal thickness of the coupon, E is the Young's modulus, $\sigma_{0.2}$ and $\sigma_{1.0}$ are the 0.2% and
111 1.0% proof stresses respectively, σ_u is the ultimate tensile stress, ε_u is the strain at the ultimate
112 tensile stress and n , $m_{1.0}$ and m_u are the strain hardening exponents of the two-stage Ramberg-
113 Osgood material model (Gardner, 2019; Arrayago *et al.*, 2015; Gardner and Ashraf, 2006;
114 Rasmussen, 2003; Mirambell and Real, 2000; Hill, 1944; Ramberg and Osgood, 1943). Note
115 that the mechanical properties of the as-built material are referred to as effective (signified by
116 'eff' in the subscripts of the symbols) to acknowledge the influence of the undulating
117 geometry. The results highlight the anisotropic behavior of the printed material, while the
118 influence of the irregular geometry on the effective mechanical properties of the WAAM
119 material was shown to be detrimental (and more prominent for loading acting perpendicular
120 to the layer orientation). A more detailed description of the employed test setup and obtained
121 results is provided by Kyvelou *et al.* (2020).

122 3. STUB COLUMN TESTS

123 Compression tests on WAAM SHS stub columns were conducted to investigate their
124 compressive structural response and load carrying capacity. Simple geometries (i.e. square)
125 were chosen deliberately for the stub column test specimens to enable the influence of the
126 production process alone on the exhibited structural response to be isolated and to allow
127 direct comparisons to be made against traditionally manufactured tubular sections and their
128 corresponding design provisions. Variation in the local cross-section slenderness of the tested

129 specimens was considered by adjusting the cross-sectional proportions (i.e. the outer
130 dimensions and wall thickness), allowing the influence of local buckling to be assessed, while
131 repeated tests enabled the variability in response to be evaluated; 14 SHS stub columns were
132 tested in total.

133 The adopted labelling system for the test specimens begins with the nominal cross-sectional
134 dimensions in mm (in the form width×depth×thickness), followed by the nominal length in
135 mm and the letter ‘F’ indicating fixed end conditions; a specimen label ending with an ‘R’ is
136 a repeat test. Note that specimens 120×120×8.0-450-F and 130×130×3.5-500-F (and their
137 repeats) were chosen to be of similar proportions (i.e. similar width-to-thickness ratios) to
138 key elements of the MX3D bridge. A comparison between specimen 120×120×8.0-450-F and
139 its corresponding part of the substructure of the bridge is shown in Figure 4.

140 **3.1 Production and preparation of SHS specimens**

141 The SHS specimens were manufactured by the Dutch start-up company MX3D, using their
142 proprietary multi-axis robotic WAAM technology (MX3D, 2019). The employed printer
143 comprised a 6-axis robotic arm coupled with a metal inert gas (MIG) welding machine. CAD
144 models of the specimens were drawn in Rhino 3D (2017) and sliced into finite layers that the
145 printer could trace along the cross-section slices. Then, wire feedstock, continuously supplied
146 to the printer, was melted and deposited onto a substrate plate, building up the specimen layer
147 by layer. The utilised feedstock material was Grade 308LSi austenitic stainless steel wire.
148 During the deposition process, the current was 100-140 A, the arc voltage was 18-21 V, while
149 the deposition rate was typically between 0.5 and 2.0 kg/h. For the stub columns of 3.5 mm
150 nominal thickness, wire of 1.0 mm diameter was employed with a welding speed of 15- 30
151 mm/s and a wire feed rate of 4-8 m/min while for the stub columns of 8.0 mm nominal
152 thickness, wire of 1.2 mm diameter was used with a welding speed and wire feed rate of 13
153 mm/s and 5.7 m/min respectively. Finally, the employed shield gas was 98% AR and 2%
154 CO₂, at a flow rate of 10-20 L/min. Printing of typical specimens is illustrated in Figure 5.

155 Following their fabrication, the stub columns were detached from their substrate plate using a
156 plasma arc cutter and then cut to specified lengths of approximately four times the outer
157 cross-section dimensions; this was chosen to be long enough to include a representative
158 distribution of residual stresses and geometric imperfections, yet short enough to prevent
159 overall flexural buckling (Ziemian, 2010). Both ends of the stub columns were machined to
160 be flat and parallel and the exterior surfaces were sandblasted with glass beads to remove any

161 welding soot from the WAAM process. The cutting process and surface treatment of a typical
162 specimen are shown in Figures 6(a) and 6(b) respectively.

163 **3.2 Geometrical measurements**

164 Measuring the geometry of the stub column test specimens was more challenging than usual
165 due to the surface undulations and wall thickness variation arising as a result of the WAAM
166 process. Hand measurements, along with a number of more sophisticated techniques – 3D
167 laser scanning, silicone casting and measurements based on Archimedes’ principle – were
168 employed to determine the as-built geometric properties of the SHS specimens. 3D laser
169 scanning and silicone casting were finally adopted, while the hand and Archimedes’
170 measurements served as references for comparison and verification purposes.

171 *3.2.1 Hand measurements*

172 Digital hand calliper measurements were taken to provide baseline geometric data for the
173 examined specimens. Measurements of the SHS face widths (i.e. the outer dimensions H_h)
174 were taken at five locations along the length of each specimen (including the two ends), while
175 the wall thickness t_h was recorded at three equally spaced locations on each face, at both ends
176 (i.e. 12 measurements per end in total). Finally, separate length measurements L_h were taken
177 along each SHS face (utilising a tape measure for the longer specimens). The average
178 geometric properties are listed in Table 3, where A_h is the cross-sectional area calculated
179 using the average values of the measurements and with the inner and outer corner radii taken
180 as equal to $1.0t_h$ and $1.5t_h$ respectively, based on hand measurements.

181 *3.2.2 Archimedes’ principle*

182 The water displacement method, which is based on Archimedes’ physical law of buoyancy
183 and is frequently employed to determine porosity in concrete elements (Ibrahim *et al.* 2014;
184 Park and Tia, 2004) and diffusible hydrogen in welds (Schmid and Rodabaugh, 1980), was
185 utilised for the determination of the average cross-sectional areas of the examined stub
186 columns. Each specimen was hung using chains from weighing scales and its mass was
187 measured both when in air (m_a) and when submerged in a water bath (m_w); the employed
188 setup is illustrated in Figure 7(a). The mean cross-sectional area of the specimens A_{Arch} ,
189 reported in Table 3, was hence determined according to Equation (1), by dividing the
190 resulting volume V_{Arch} by the member length L_h (measured as described in Section 3.2.1). In

191 Equation (1), $m_{c,a}$ and $m_{c,w}$ are the mass of the chain in air and in water (submerged to the
192 same depth as with the specimen hanging) respectively and ρ_w is the density of the water.

$$A_{\text{Arch}} = \frac{V_{\text{Arch}}}{L_h} = \frac{[(m_a - m_{c,a}) - (m_w - m_{c,w})] / \rho_w}{L_h} \quad (1)$$

193 3.2.3 Laser scanning

194 In order to obtain an accurate and detailed representation of the external and internal surfaces
195 of all specimens prior to testing, 3D laser scanning was employed. A FARO ScanARM,
196 capable of capturing up to 600,000 points per second to an accuracy of 0.1 mm, was used to
197 scan and digitally reproduce all the printed specimens. Although full scans of the outer
198 surface of the specimens could be taken, the physical size of the head of the laser scan arm
199 prevented direct scanning of the complete inner surface profile (allowing only direct scanning
200 of the inner surface at the column ends) – see Figure 7(b). Hence, silicone casting was
201 undertaken to form a scannable replica of the internal surface of the specimens.

202 The silicone casts were formed using SUPERSIL 25, a two-component silicone elastomer.
203 The components were thoroughly mixed mechanically and then degassed in a vacuum
204 chamber to remove any entrained air – see Figure 8. A central insert was placed within the
205 specimens, as shown in Figure 9(a), prior to casting to reduce the volume of silicone required
206 and to facilitate easier removal. Silicone release spray was also applied to the inner and outer
207 surfaces of the specimens and inserts respectively. The prepared silicone mixture was then
208 slowly poured in between the insert and the specimen, to avoid the introduction of air voids,
209 which could affect the scanned silicone surface, and allowed to set for at least 24 hours. Once
210 set, the insert was removed, allowing the silicone cast to be collapsed into the void and
211 removed from the printed specimen – see Figure 9(b). A silicone cast adjacent to its parent
212 SHS stub column is shown in Figure 10. Following its extraction from within the specimen,
213 the cured silicone internal replica was laser scanned.

214 The outer steel and inner silicone scans of the as-built geometries were merged and converted
215 into 3D CAD models with polygon meshes using Geomagic Wrap (2017). The CAD models
216 were subsequently imported into Rhino 3D (2017), where contouring of the specimens was
217 undertaken, allowing accurate determination of the cross-sectional dimensions. Processing of
218 a typical specimen in Rhino is presented in Figure 11, where only a limited number of cross-
219 sectional contours is shown for illustration purposes.

220 Special attention was given to the determination of the most suitable contour spacing dx
221 along the length of the specimens in order to achieve computational efficiency and accurate
222 determination of the geometric properties. A sensitivity study was therefore undertaken on a
223 typical specimen and its repeat ($80 \times 80 \times 3.5$ -320-F and $80 \times 80 \times 3.5$ -320-FR), obtaining their
224 geometric measurements at contour spacings of 0.1 mm, 0.2 mm, 0.5 mm, 1.0 mm and 2.0
225 mm. The obtained results are presented in Figure 12, where the mean, minimum and
226 maximum measurements of the cross-sectional area (A , A_{\min} and A_{\max} , respectively)
227 determined for the different contour spacings are normalized against the equivalent values
228 corresponding to $dx = 0.1$ mm. As expected, the extreme values of the measurements (i.e.
229 A_{\min} and A_{\max}) were more sensitive to the contour spacing compared to the respective mean
230 values (i.e. A). Overall, since the measurements obtained using a spacing of 0.2 mm were
231 almost identical to these obtained with a spacing of 0.1 mm, a contour spacing of 0.2 mm was
232 adopted. Note that the considered contour spacings were all below the typical WAAM
233 deposition width w , shown in Figure 13, which was found to vary between about 3 mm and 5
234 mm for the studied specimens; a similar value of 4 mm was reported by Ding *et al.* (2014).

235 A summary of the geometric properties of all specimens, as obtained from the laser scans, is
236 presented in Table 4, where t and t_{sd} are the mean and standard deviation values of the
237 thickness respectively, A , A_{\max} and A_{\min} are the mean, maximum and minimum cross-
238 sectional areas respectively, H is the average face width and r and R are the inner and outer
239 corner radii, obtained by means of fitting a cylinder to the scanned data of each corner region,
240 as illustrated in Figure 14.

241 3.2.4 Comparison between methods

242 Comparisons between the geometric properties determined using the different measuring
243 techniques are presented in Table 5. The average cross-sectional areas determined from the
244 hand measurements A_h differ somewhat, ranging between 10% below to 15% above, from
245 those calculated based on Archimedes' principle A_{Arch} . This confirms that the use of hand
246 measurements alone can lead to substantial errors in the determination of the geometric
247 properties of WAAM specimens; this is because the discrete hand measurements cannot, in
248 general, be extrapolated to the full sample. Conversely, there is very good agreement between
249 the cross-sectional areas A_{Arch} and A , with differences consistently below 3%, providing
250 confidence in the employed 3D laser scanning technique – see Table 5.

251 3.3 Results of geometric measurements

252 Comparisons between the values of the mean and minimum and mean and maximum cross-
253 sectional areas, as obtained by laser scanning (A_{\min}/A and A_{\max}/A respectively), are presented
254 in Table 5, revealing the maximum geometric variation within a given specimen. Histograms
255 showing the cross-sectional area variation within specimens are presented in Figure 15,
256 where each cross-sectional area measurement A_i is normalized by the corresponding average
257 cross-sectional area A of each specimen. The values of the coefficient of variation (COV) of
258 the area V_A , defined as the ratio of the standard deviation of the area divided by the average
259 area A_{sd}/A of each specimen, are reported in Table 5, and were found to range between 0.04
260 and 0.10; a similar statistical geometric measure (i.e. t_{sd}/t) was used by Kyvelou *et al.* (2020)
261 to predict the influence of the geometric undulations on the effective mechanical properties of
262 WAAM sheet material.

263 3.4 Local geometric imperfections

264 Determination of the amplitudes of the local geometric imperfections, as distinct from the
265 surface undulations associated with the individual weld layers – see Figure 16, along the
266 length of the examined specimens was undertaken using the points located along the
267 centreline of the outer flat faces (i.e. along one line per face) – see Figure 17. The
268 imperfection amplitude for each face was then defined as the maximum deviation of the
269 selected data points from a straight line fitted to the data using least squares regression. This
270 definition of local imperfection amplitude is considered to be appropriate for evaluating the
271 structural performance of the examined profiles and for use in subsequent numerical
272 analyses, since it is the deviation from flatness along the longitudinal axis of structural
273 elements that triggers and amplifies local instability phenomena (i.e. local plate buckling) and
274 hence governs the ultimate cross-section strength. However, simply using the maximum
275 deviation of the raw data from the reference line was deemed to be inappropriate due to the
276 presence of some particularly prominent surface undulations and pronounced weld beads,
277 which could result in unrealistically large imperfection amplitudes. Therefore, to eliminate
278 the effect of these unwanted features from the data, the obtained imperfection distributions
279 were smoothed to a 10 mm moving average curve, a typical example of which is shown in
280 Figure 18. The 10 mm averaging interval spanned 3 to 4 weld layers and was found, by trial
281 and error, to be suitable for removing the unwanted features, without affecting the underlying
282 imperfection profile; this was the case for all but specimen 100×100×3.5-400-F, where two

283 pronounced weld beads on one of the faces were not fully removed using the moving average
284 approach and hence resulted in unrepresentative imperfection amplitudes; these were
285 therefore removed manually – see Figure 19.

286 The maximum deviation of the smoothed curves from the reference line among the four faces
287 was then taken as the local imperfection amplitude of each specimen e_{\max} , as reported in
288 Table 4. The maximum measured imperfection values typically lie between about 0.5 mm
289 and 2.0 mm; these are higher than the imperfection values typically observed in
290 conventionally formed sections (Meng and Gardner, 2020; Schafer and Pekoz, 1998) and
291 similar to the dimensional accuracy of around ± 1.0 mm to ± 2.0 mm generally quoted for
292 WAAM elements (Kumar *et al.*, 2020; Li *et al.*, 2020; Laghi *et al.*, 2019). There was no clear
293 link in the limited dataset between the imperfection amplitude and either the thickness or
294 width of the examined specimens. Note that the largest imperfection amplitude was recorded
295 for specimen 180×180×3.5-720-FR; visual inspection of this specimen confirmed the lower
296 print quality.

297 A histogram of all 56 local imperfection amplitude measurements (one measurement per
298 section face) is presented in Figure 20, where the cumulative distribution function (CDF) is
299 also plotted. CDF values, along with the key statistics, are given in Table 6. A CDF value,
300 expressed as $P(e_{\max,r} < e_{\max,d})$, reflects the probability that the maximum geometric
301 imperfection in a randomly selected WAAM specimen $e_{\max,r}$ is less than a defined value
302 $e_{\max,d}$. The $P(e_{\max,r} < e_{\max,d}) = 0.95$ (i.e. the characteristic value of the imperfection)
303 corresponds to $e_{\max,d} \approx 3.0$ mm, indicating that a WAAM member is expected to have
304 maximum imperfections greater than this value only 5% of the time.

305 **3.5 Test setup**

306 The experimental layout adopted for the compressive stub column tests is presented in Figure
307 21. The load was applied through an Instron 3500 kN testing machine at a displacement rate
308 of 0.5 mm/min while a self-locking spherical head was used to ensure full contact between
309 the stub column ends and end platens. Four equally spaced linear variable displacement
310 transducers (LVDTs) and four strain gauges attached to the specimens at mid-height on
311 opposite faces were used to measure the vertical deformation of the test specimens, while a
312 load cell within the testing machine measured the applied load – this setup has also been used
313 for previous SHS stub column tests (Buchanan *et al.*, 2017; Wang *et al.*, 2017).

314 Owing to the undulating surface of the examined specimens, a two-component PS polyester
315 adhesive was employed as a surface precoating agent to provide a smooth surface for the
316 attachment of the strain gauges. However, this technique was deemed to provide accurate
317 measurements only up to 0.2% strain, which corresponds to strains substantially lower than
318 those reached during testing. Furthermore, the localised nature of strain gauge readings
319 renders them less representative of the overall structural response in specimens with
320 undulating surfaces; hence, the use of strain gauges was omitted for most of the conducted
321 tests. It should be also mentioned that the testing machine size and the length of specimens
322 180×180×3.5-720-F and 180×180×3.5-720-FR rendered the use of the self-locking spherical
323 head infeasible; cement grout was used at the top of these specimens instead to ensure full
324 contact between the stub column ends and the loading platen.

325 Axial load, strain gauge (when used) and LVDT measurements were recorded at a frequency
326 of 2 Hz using an in-house developed data logger. A two camera LaVision DIC system was
327 also used, acquiring images at a frequency of 0.2 Hz, allowing surface deformations and
328 strain fields to be accurately recorded for one of the flat faces of the specimen; the applied
329 force was also recorded through an analogue to digital converter. The acquired images were
330 processed in the software DaVis (LaVision, 2017). Vertical displacements adjacent to the top
331 and bottom end platens were calculated, exported and subtracted to determine the true end
332 shortening response of the stub columns. The surface deformation field of a typical WAAM
333 specimen at the peak load is presented in Figure 22(a) while the equivalent field for a PBF
334 SHS specimen of similar slenderness (Buchanan *et al.*, 2017) is shown in Figure 22(b). It can
335 be observed that the deformation field of the WAAM specimen is less regular than for the
336 PBF specimen, especially in terms of out-of-plane deformations. This is attributed to the
337 more variable geometry of the as-built WAAM specimens, particularly the variations in
338 thickness and the surface undulations.

339 **3.6 Test results**

340 Two alternative methods were initially adopted for the determination of the load-end
341 shortening curves: (i) using the LVDT and strain gauge data, accounting for the deformation
342 of the end platens (Meng and Gardner, 2020; Zhao *et al.*, 2015; Gardner *et al.*, 2016; Centre
343 for Advanced Structural Engineering, 1990), and (ii) using the DIC data, by subtracting the
344 vertical deformations recorded at the stub column ends. Typical comparisons between load-
345 end shortening curves derived according to these two different approaches are shown in

346 Figure 23, where it is apparent that the curves yielded by the two different methods are
347 almost identical. Hence, only the DIC derived results are reported herein since they are
348 deemed to be generally more accurate since the measurements are made directly on the
349 specimens.

350 The load-end shortening curves of all specimens are presented in Figure 24, while a summary
351 of the obtained results is given in Table 7, where N_u is the ultimate axial load and δ_u is the
352 column end shortening at N_u as calculated from the DIC data. The deformed shapes of the
353 stub columns, shown in Figure 25, although akin to the classical ‘in-out’ local buckling, were
354 clearly influenced by the initial imperfections and surface undulations inherent to the WAAM
355 process.

356 Some variation in structural behavior between repeat specimens was observed, with
357 differences in ultimate capacity up to 18%, reflecting the greater geometric variability
358 associated with WAAM cross-sections relative to conventional sections. In Figure 26(a), the
359 normalized axial resistance $N_u/A\sigma_{0.2,eff}$ of the tested specimens is plotted against the local
360 slenderness $c/(t\varepsilon)$, in order to capture the general trend of decreasing capacity with increasing
361 local slenderness. In Figure 26(a), c is the mean flat width of the faces of the SHS, t is the
362 mean thickness and $\varepsilon = \sqrt{(235 / \sigma_{0.2}) / (E / 210000)}$ (EN 1993-1-4, 2006). In Figure 26(b), the
363 normalized axial resistance $N_u/A\sigma_{0.2,eff}$ of each specimen is itself normalized by the general
364 $N_u/A\sigma_{0.2,eff}$ versus $c/t\varepsilon$ linear regression trend for all tested specimens, the expression for
365 which is denoted ρ_{linear} and given in Figure 26(a), and plotted against the local imperfection
366 amplitude e_{max} (as defined in Section 3.3) normalized by the average thickness t . It is clear
367 from the results that the relative structural performance of the WAAM specimens degrades
368 with increasing e_{max}/t values, and that geometric imperfections are the key cause of variation
369 in structural behavior between the repeat tests.

370 Analysis of the geometric data from the laser scans revealed a general correlation between the
371 location of failure (i.e. local buckling) in the specimens and the regions containing the most
372 prominent thickness reductions and geometric imperfections. In Figure 27, the average wall
373 thickness t and imperfection amplitude e (as defined in Section 3.3) of each cross-section are
374 plotted against the specimen length for two typical specimens (one for each nominal
375 thickness). It can be observed that local buckling is triggered in areas where high values of
376 imperfections and low values of thickness coincide.

377 4. STRUCTURAL DESIGN OF WAAM ELEMENTS

378 Before broader application of metal 3D printing in the construction sector is possible, further
379 research and greater standardisation is needed. In this section, the performance of the
380 examined WAAM SHS specimens is initially compared against the response of
381 conventionally manufactured SHS, and, subsequently, against strength predictions yielded by
382 design standards of current practice, in order to assess their suitability for the structural
383 design of WAAM SHS.

384 4.1 Comparisons with existing tests data on conventionally manufactured stainless steel 385 SHS

386 The structural performance of the tested WAAM specimens is compared with that of AM
387 PBF (Buchanan *et al.*, 2017) and conventionally formed austenitic (Chen *et al.*, 2018; Yuan
388 *et al.*, 2014; Gardner and Nethercot, 2004; Rasmussen, 2000; Rasmussen and Hancock,
389 1993), ferritic (Arrayago *et al.*, 2016; Afshan and Gardner, 2013) and duplex (Chen *et al.*,
390 2018; Yuan *et al.*, 2014; Theofanous and Gardner, 2009) stainless steel SHS in this sub-
391 section. A graphical illustration is presented in Figure 28, where the normalized axial
392 resistance $N_w/A\sigma_{0.2}$ of the tested specimens is plotted against the local slenderness $c/(t\varepsilon)$,
393 where c is the mean flat width of the faces of the SHS, t is the mean thickness and $\varepsilon =$
394 $\sqrt{(235 / \sigma_{0.2}) / (E / 210000)}$ (EN 1993-1-4, 2006). Note that, although in the new version of
395 EN 1993-1-4 (2020) the calculation of ε has been simplified by omitting the $E/210000$ ratio,
396 it has been retained herein because of the significant deviation of the Young's modulus E of
397 the WAAM material from that of traditionally produced material.

398 Comparisons are shown based on both the machined and effective material properties. It is
399 clear that, when the underlying material properties of the machined coupons are used, the
400 cross-sections under-perform relative to current design provisions. However, when the
401 effective material properties are used, the weakening effect of the geometric undulations,
402 caused by local thickness variations and eccentricities associated with the individual weld
403 layers, is normalized out, and the obtained test results fall within the range of conventionally
404 produced SHS stainless steel stub columns.

405 4.2 Comparisons with AISC 370

406 In this sub-section, the ultimate test capacities of the WAAM stub columns are compared to
407 the strength predictions determined according to AISC 370 (2020). Two different sets of

408 material properties were utilised in the design equations: (1) the material properties (E and
409 $\sigma_{0.2}$) from the machined coupons in the 90° direction, as reported in Table 2 and (2) the
410 effective material properties (E_{eff} and $\sigma_{0.2,\text{eff}}$) from the as-built coupons in the 90° direction for
411 both nominal material thicknesses (i.e. 3.5 mm and 8.0 mm), as reported in Table 1. Note that
412 the influence of the geometric undulations associated with WAAM inherently features in the
413 effective material properties determined from the tensile coupon tests performed on the as-
414 built material. The capacity predictions derived using the two different sets of material
415 properties are denoted $N_{u,\text{AISC},m}$ and $N_{u,\text{AISC},\text{eff}}$ respectively. The mean cross-sectional
416 dimensions, as determined from the laser scans, were used in all design calculations and all
417 safety factors were set to unity.

418 Comparisons between the test results and AISC 370 strength predictions are presented in
419 Figure 29 and listed in Table 8. Note that, although the reduction factor accounting for local
420 buckling is used in the design equations to reduce only the flat widths of the SHS faces (and
421 not the gross cross-sectional area), in Figure 29, for illustration purposes, the AISC 370
422 reduction factor function has been directly plotted. It can be observed that use of the material
423 properties obtained from the machined coupons leads to consistent overpredictions of the
424 load-carrying capacities of the examined cross-sections (with $N_u/N_{u,\text{AISC},m} = 0.85$ on average),
425 while use of the effective material properties leads to more reasonable and safe-sided
426 resistance predictions (with $N_u/N_{u,\text{AISC},\text{eff}} = 1.12$ on average). Hence, it is clear that account of
427 the weakening effect of geometric undulations should be taken (for example through the use
428 of effective material properties as adopted herein or through an alternative reduced thickness
429 approach) to achieve suitable strength predictions using the AISC 370 resistance function, but
430 further data and reliability analyses are required before a suitable safety factor could be
431 derived.

432 **4.3 Comparisons with EN 1993-1-4**

433 In this sub-section, the ultimate test capacities of the WAAM stub columns are compared to
434 the resistance predictions determined according to EN 1993-1-4 (2020). The comparisons are
435 illustrated in Figure 30 and are presented in Table 9, together with the compressive cross-
436 section classes. As in Section 4.2, use of the material properties from both the machined and
437 as-built coupons for the 90° direction has been assessed in the EN 1993-1-4 resistance
438 function; the resulting resistance predictions are denoted $N_{u,\text{EN},m}$ and $N_{u,\text{EN},\text{eff}}$ respectively.
439 Note that effective cross-sectional properties have been calculated for Class 4 sections in line

440 with EN 1993-1-4 (2020) and EN 1993-1-5 (2006) to account for the loss of effectiveness
441 due to local buckling while ε was calculated as explained in Section 4.1 (i.e. $\varepsilon =$
442 $\sqrt{(235 / \sigma_{0.2}) / (E / 210000)}$). Also, as for the AISC 370 comparisons, although the reduction
443 factor accounting for local buckling ρ is only used to reduce the flat widths of the SHS faces
444 (and not the gross cross-sectional area), in Figure 30, for illustration purposes, the EN 1993-
445 1-4 reduction factor function has been directly plotted.

446 It is clear that, when the underlying material properties of the machined coupons are used, the
447 load-carrying capacities of the examined cross-sections are generally overpredicted (with
448 $N_u/N_{u,EN,m} = 0.85$ on average). Conversely, when the effective material properties of the
449 undulating coupons are employed, more reasonable resistance predictions are achieved (with
450 $N_u/N_{u,EN,eff} = 1.13$ on average). Hence, as for AISC 370, provided the weakening effect of the
451 undulations inherent in the as-built geometry is considered (through the use of effective
452 mechanical properties in the present study), adequate resistance predictions for WAAM SHS
453 in compression are achieved using the existing EN 1993-1-4 (2020) design equations. Again,
454 further data and reliability analyses are required before a suitable safety factor could be
455 derived.

456 **4.4 Comparisons with the continuous strength method**

457 The continuous strength method (CSM) (Arrayago *et al.*, 2020; Afshan and Gardner, 2013;
458 Gardner *et al.*, 2011) has been also used to predict the cross-sectional resistances of the tested
459 WAAM specimens. The CSM, which has been recently included in both AISC 370 (2020)
460 and EN 1993-1-4 (2020), is a deformation-based design approach that accounts for the
461 beneficial influence of strain hardening. The CSM capacities predicted using the material
462 properties of the machined and as-built coupons are denoted $N_{u,CSM,m}$ and $N_{u,CSM,eff}$
463 respectively. Normalized CSM resistance predictions are provided in Table 10 and illustrated
464 in Figure 31 (where the CSM prediction curve has been plotted using the effective material
465 properties of the 3.5 mm specimens), with $N_u/N_{u,CSM,m} = 0.76$ and $N_u/N_{u,CSM,eff} = 1.00$ on
466 average. The CSM resistance predictions are accurate when the effective material properties
467 of the as-built coupons are employed, but may, nonetheless, require recalibration for
468 application to WAAM structural elements in order to ensure that the required level of
469 reliability is achieved.

470 **5. CONCLUSIONS**

471 An experimental study into the material and cross-sectional properties of WAAM stainless
472 steel structural elements has been presented. The research was carried out to gain insight into
473 the structural behavior of WAAM stainless steel members and, also, to complement the
474 safety verification of the world's first metal 3D printed bridge (Gardner *et al.*, 2020).

475 Compression tests on a total of 14 SHS stub columns, covering a wide range of local
476 slendernesses, were performed. Sophisticated non-contact measurement techniques were
477 employed to determine the as-built geometric properties of the specimens, featuring 3D laser
478 scanning, silicone casting and measurements based on Archimedes' principle, while digital
479 image correlation measurements were used to provide detailed insight into the deformation
480 characteristics of the test specimens. It was found that the WAAM stub columns exhibited
481 more variable capacities between repeat specimens than generally displayed by
482 conventionally formed stainless steel SHS and this was demonstrated to relate principally to
483 the variation in local geometric imperfections.

484 The test results were compared with capacity predictions obtained using the AISC 370, EN
485 1993-1-4 and CSM resistance functions with mechanical properties determined from tensile
486 tests on both machined and as-built WAAM coupons, with the latter including the weakening
487 effect of the undulating geometry inherent to the WAAM process. Use of the machined
488 material properties generally resulted in unconservative capacity predictions, while this was
489 remedied through the use of the effective mechanical properties of the as-built coupons.
490 Further test data and reliability analyses are required for the determination of suitable safety
491 factors for the design of cross-sections produced by wire arc additive manufacturing.

492 **DATA AVAILABILITY STATEMENT**

493 Some or all data, models, or code that support the findings of this study are available from the
494 corresponding author upon reasonable request.

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653

654 **TABLES**

655 Table 1: Average effective material properties of as-built coupons by direction of testing relative
656 to the print layer orientation (Kyvelou *et al.*, 2020)
657

t_{nom} (mm)	θ (°)	E_{eff} (MPa)	$\sigma_{0.2,\text{eff}}$ (MPa)	$\sigma_{1.0,\text{eff}}$ (MPa)	$\sigma_{\text{u,eff}}$ (MPa)	$\varepsilon_{\text{u,eff}}$	n_{eff}	$m_{1.0,\text{eff}}$	$m_{\text{u,eff}}$
3.5	0	135900	333	362	553	0.273	15.5	1.8	2.2
	45	192600	344	391	570	0.255	9.4	2.4	2.3
	90	90200	261	319	448	0.119	6.5	2.5	2.6
8.0	0	137100	325	349	535	0.325	22.9	1.8	2.4
	45	201200	351	391	559	0.255	11.5	2.3	2.3
	90	109100	271	326	423	0.103	5.5	2.6	2.5

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661 Table 2: Average material properties of machined coupons by direction of testing relative to the
662 print layer orientation (Kyvelou *et al.*, 2020)
663

θ (°)	E (MPa)	$\sigma_{0.2}$ (MPa)	$\sigma_{1.0}$ (MPa)	σ_{u} (MPa)	ε_{u}	n	$m_{1.0}$	m_{u}
0	143300	356	382	575	0.307	15.8	1.7	2.4
45	219500	407	437	626	0.364	13.6	2.0	2.4
90	139600	338	381	554	0.297	6.8	2.3	2.7

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Table 3: Summary of the average hand measured geometric properties of the SHS specimens

Specimen ID	L_h (mm)	H_h (mm)	t_h (mm)	A_h (mm ²)	A_{Arch} (mm ²)
60×60×3.5-240-F	240.1	59.7	3.91	857.1	888.3
60×60×3.5-240-FR	240.3	60.0	3.82	841.8	838.8
80×80×3.5-320-F	320.4	80.0	3.86	1160.5	1191.9
80×80×3.5-320-FR	320.0	79.9	3.80	1140.9	1153.4
100×100×3.5-400-F	400.4	100.0	4.31	1627.8	1516.8
100×100×3.5-400-FR	409.3	100.0	4.39	1657.4	1499.2
120×120×8.0-450-F	475.4	119.2	7.23	3181.7	2876.5
120×120×8.0-450-FR	450.9	119.1	7.04	3101.8	2700.9
130×130×3.5-500-F	501.6	131.0	3.96	1994.1	1840.9
130×130×3.5-500-FR	487.5	130.6	4.35	2176.6	1829.1
150×150×3.5-600-F	600.0	149.6	3.88	2247.8	2285.4
150×150×3.5-600-FR	599.5	149.8	3.98	2304.7	2271.2
180×180×3.5-720-F	720.0	179.4	3.96	2761.0	2765.3
180×180×3.5-720-FR	720.3	179.1	3.59	2503.8	2790.1

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Table 4: Summary of the average geometric properties of the SHS specimens as determined by the laser scans and by measurements based on Archimedes' principle

673

Specimen ID	H (mm)	r (mm)	R (mm)	t (mm)	A (mm ²)	A_{max} (mm ²)	A_{min} (mm ²)	e_{max} (mm)
60×60×3.5-240-F	60.0	4.48	7.15	4.11	914.8	1116.0	795.1	1.41
60×60×3.5-240-FR	60.0	4.76	7.19	3.85	843.6	969.2	750.6	1.39
80×80×3.5-320-F	79.9	4.46	7.11	4.05	1227.4	1544.6	1069.0	1.65
80×80×3.5-320-FR	80.0	4.49	7.15	3.91	1182.2	1332.6	1052.5	2.05
100×100×3.5-400-F	100.0	4.32	6.54	3.99	1496.7	1703.2	1372.2	0.62
100×100×3.5-400-FR	99.9	4.29	6.79	3.99	1520.2	1699.0	1355.3	0.72
120×120×8.0-450-F	118.0	5.60	8.11	6.53	2894.2	3217.2	2495.1	1.78
120×120×8.0-450-FR	117.1	5.28	8.09	6.27	2709.9	3417.9	2426.8	2.69
130×130×3.5-500-F	129.0	4.92	7.35	3.62	1848.8	2149.3	1652.0	1.53
130×130×3.5-500-FR	128.8	4.96	7.73	3.64	1824.2	2340.3	1636.4	1.41
150×150×3.5-600-F	149.8	4.37	6.67	4.05	2324.1	2633.8	2009.5	1.14
150×150×3.5-600-FR	149.7	4.22	7.11	4.00	2327.1	2737.4	1910.5	0.71
180×180×3.5-720-F	179.5	4.45	6.73	4.05	2832.3	3248.9	2419.5	1.13
180×180×3.5-720-FR	179.0	4.65	6.87	4.06	2874.1	4849.2	2380.8	3.80

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676 Table 5: Comparison of geometric properties determined using different measurement methods

Specimen ID	A_h / A_{Arch}	A / A_{Arch}	A / A_{min}	A / A_{max}	A_{sd} / A	e_{max} / t
60×60×3.5-240-F	0.96	1.03	1.15	0.82	0.07	0.34
60×60×3.5-240-FR	1.00	1.01	1.12	0.87	0.06	0.36
80×80×3.5-320-F	0.97	1.03	1.15	0.79	0.07	0.41
80×80×3.5-320-FR	0.99	1.02	1.12	0.89	0.05	0.52
100×100×3.5-400-F	1.07	0.99	1.09	0.88	0.04	0.16
100×100×3.5-400-FR	1.11	1.01	1.12	0.89	0.05	0.18
120×120×8.0-450-F	1.11	1.01	1.16	0.90	0.04	0.27
120×120×8.0-450-FR	1.15	1.00	1.12	0.79	0.05	0.43
130×130×3.5-500-F	1.08	1.00	1.12	0.86	0.05	0.42
130×130×3.5-500-FR	1.19	1.00	1.11	0.78	0.08	0.39
150×150×3.5-600-F	0.98	1.02	1.16	0.88	0.05	0.28
150×150×3.5-600-FR	1.01	1.02	1.22	0.85	0.04	0.18
180×180×3.5-720-F	1.00	1.02	1.17	0.87	0.04	0.28
180×180×3.5-720-FR	0.90	1.03	1.21	0.59	0.10	0.94
Mean	1.04	1.01	1.14	0.83	0.06	0.37
COV	0.08	0.01	0.03	0.09	0.29	0.51

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681 Table 6: Cumulative distribution function (CDF) values for imperfection amplitudes

$P (e_{max,r} < e_{max,d})$	$e_{max} (mm)$
0.25	0.73
0.50	1.28
0.75	1.50
0.90	2.05
0.95	2.94
0.99	3.80
Mean	1.29
St. Dev.	0.68

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Table 7: Summary of stub column test results

Specimen ID	N_u (kN)	δ_u (mm)
60×60×3.5-240-F	277.5	5.84
60×60×3.5-240-FR	250.6	5.74
80×80×3.5-320-F	353.0	3.69
80×80×3.5-320-FR	314.1	3.25
100×100×3.5-400-F	440.8	2.86
100×100×3.5-400-FR	437.6	2.81
120×120×8.0-450-F	993.2	8.86
120×120×8.0-450-FR	841.1	6.64
130×130×3.5-500-F	437.4	2.23
130×130×3.5-500-FR	414.8	1.87
150×150×3.5-600-F	520.3	2.27
150×150×3.5-600-FR	556.1	2.34
180×180×3.5-720-F	528.1	1.89
180×180×3.5-720-FR	468.1	2.26

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Table 8: Comparisons of test results with AISC 370 design predictions

Specimen ID	AISC 370 predictions (machined properties)				AISC 370 predictions (effective properties)			Comparisons	
	N_u (kN)	Class	$\lambda\sqrt{\sigma_{0.2}/E}$	$N_{u,AISC,m}$ (kN)	Class	$\lambda\sqrt{\sigma_{0.2}/E}$	$N_{u,AISC,eff}$ (kN)	$N_u / N_{u,AISC,m}$	$N_u / N_{u,AISC,eff}$
60×60×3.5-240-F	277.5	Non slender	0.53	309.2	Non slender	0.58	238.8	0.90	1.16
60×60×3.5-240-FR	250.6	Non slender	0.56	285.1	Non slender	0.62	220.2	0.88	1.14
80×80×3.5-320-F	353.0	Non slender	0.78	414.9	Non slender	0.85	320.4	0.85	1.10
80×80×3.5-320-FR	314.1	Non slender	0.81	399.6	Non slender	0.89	308.6	0.79	1.02
100×100×3.5-400-F	440.8	Non slender	1.05	505.9	Non slender	1.15	390.6	0.87	1.13
100×100×3.5-400-FR	437.6	Non slender	1.05	513.8	Non slender	1.14	396.8	0.85	1.10
120×120×8.0-450-F	993.2	Non slender	0.74	978.2	Non slender	0.75	784.3	1.02	1.27
120×120×8.0-450-FR	841.1	Non slender	0.76	915.9	Non slender	0.77	734.4	0.92	1.15
130×130×3.5-500-F	437.4	Slender	1.54	534.7	Slender	1.68	386.3	0.82	1.13
130×130×3.5-500-FR	414.8	Slender	1.52	530.8	Slender	1.66	383.2	0.78	1.08
150×150×3.5-600-F	520.3	Slender	1.64	632.7	Slender	1.79	454.7	0.82	1.14
150×150×3.5-600-FR	556.1	Slender	1.65	631.3	Slender	1.81	454.3	0.88	1.22
180×180×3.5-720-F	528.1	Slender	2.00	656.5	Slender	2.18	471.4	0.80	1.12
180×180×3.5-720-FR	468.1	Slender	1.98	675.7	Slender	2.16	486.0	0.69	0.96
							Mean	0.85	1.12
							COV	0.09	0.06

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Table 9: Comparisons of test results with EN 1993-1-4 design predictions

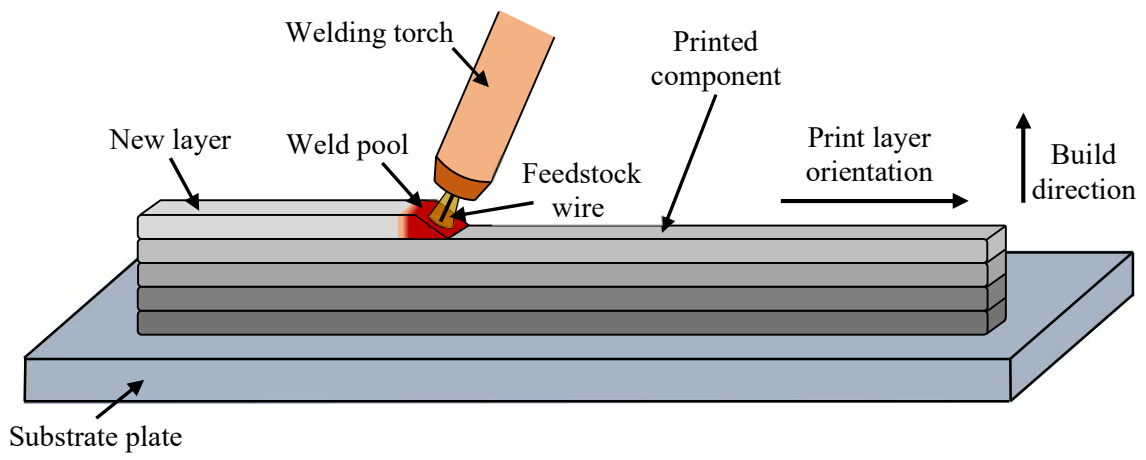
Specimen ID	Test			EN1993-1-4 predictions (machined properties)			EN1993-1-4 predictions (effective properties)			Comparisons	
	N_u (kN)	Class	$c/(t\epsilon)$	$N_{u,EN,m}$ (kN)	Class	$c/(t\epsilon)$	$N_{u,EN,eff}$ (kN)	$N_u / N_{u,EN,m}$	$N_u / N_{u,EN,eff}$		
60×60×3.5-240-F	277.5	1	15.8	309.2	1	17.3	238.8	0.90	1.16		
60×60×3.5-240-FR	250.6	1	16.9	285.1	1	18.5	220.2	0.88	1.14		
80×80×3.5-320-F	353.0	1	23.3	414.9	1	25.5	320.4	0.85	1.10		
80×80×3.5-320-FR	314.1	1	24.3	399.6	1	26.5	308.6	0.79	1.02		
100×100×3.5-400-F	440.8	1	31.4	505.9	2	34.3	390.6	0.87	1.13		
100×100×3.5-400-FR	437.6	1	31.3	513.8	2	34.2	396.8	0.85	1.10		
120×120×8.0-450-F	993.2	1	22.0	978.2	1	22.3	784.3	1.02	1.27		
120×120×8.0-450-FR	841.1	1	22.9	915.9	1	23.2	734.4	0.92	1.15		
130×130×3.5-500-F	437.4	4	45.9	532.0	4	50.2	384.6	0.82	1.14		
130×130×3.5-500-FR	414.8	4	45.4	528.2	4	49.7	381.4	0.79	1.09		
150×150×3.5-600-F	520.3	4	48.9	629.6	4	53.5	452.7	0.83	1.15		
150×150×3.5-600-FR	556.1	4	49.4	628.3	4	54.0	452.4	0.89	1.23		
180×180×3.5-720-F	528.1	4	59.6	653.9	4	65.2	469.6	0.81	1.12		
180×180×3.5-720-FR	468.1	4	59.2	673.0	4	64.7	484.3	0.70	0.97		
							Mean	0.85	1.13		
							COV	0.08	0.06		

Table 10: Comparisons of test results with CSM design predictions

Specimen ID	Test			CSM predictions (machined properties)			CSM predictions (effective properties)			Comparisons	
	N_u (kN)	$\bar{\lambda}_{p,CS}$	$\epsilon_{csm}/\epsilon_y$	$N_{u,csm,m}$ (kN)	$\bar{\lambda}_{p,CS}$	$\epsilon_{csm}/\epsilon_y$	$N_{u,csm,eff}$ (kN)	$N_u / N_{u,csm,m}$	$N_u / N_{u,csm,eff}$		
60×60×3.5-240-F	277.5	0.28	15.00	420.9	0.30	14.43	342.8	0.66	0.81		
60×60×3.5-240-FR	250.6	0.30	15.00	388.1	0.32	14.36	315.6	0.65	0.79		
80×80×3.5-320-F	353.0	0.41	6.16	470.1	0.45	4.47	356.4	0.75	0.99		
80×80×3.5-320-FR	314.1	0.43	5.36	444.5	0.47	3.89	337.4	0.71	0.93		
100×100×3.5-400-F	440.8	0.55	2.12	520.5	0.60	1.54	397.5	0.85	1.11		
100×100×3.5-400-FR	437.6	0.55	2.15	529.0	0.60	1.56	404.0	0.83	1.08		
120×120×8.0-450-F	993.2	0.39	7.61	1145.0	0.39	7.27	908.8	0.87	1.09		
120×120×8.0-450-FR	841.1	0.40	6.64	1049.2	0.41	6.34	833.7	0.80	1.01		
130×130×3.5-500-F	437.4	0.81	0.90	564.5	0.88	0.85	410.6	0.77	1.07		
130×130×3.5-500-FR	414.8	0.80	0.91	561.0	0.87	0.86	408.3	0.74	1.02		
150×150×3.5-600-F	520.3	0.86	0.87	680.6	0.94	0.81	493.6	0.76	1.05		
150×150×3.5-600-FR	556.1	0.87	0.86	677.2	0.95	0.81	490.9	0.82	1.13		
180×180×3.5-720-F	528.1	1.05	0.75	718.0	1.15	0.70	517.0	0.74	1.02		
180×180×3.5-720-FR	468.1	1.04	0.75	732.9	1.14	0.70	527.8	0.64	0.89		
							Mean	0.76	1.00		
							COV	0.09	0.10		

696 FIGURES

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699 Figure 1: Illustration of the WAAM process

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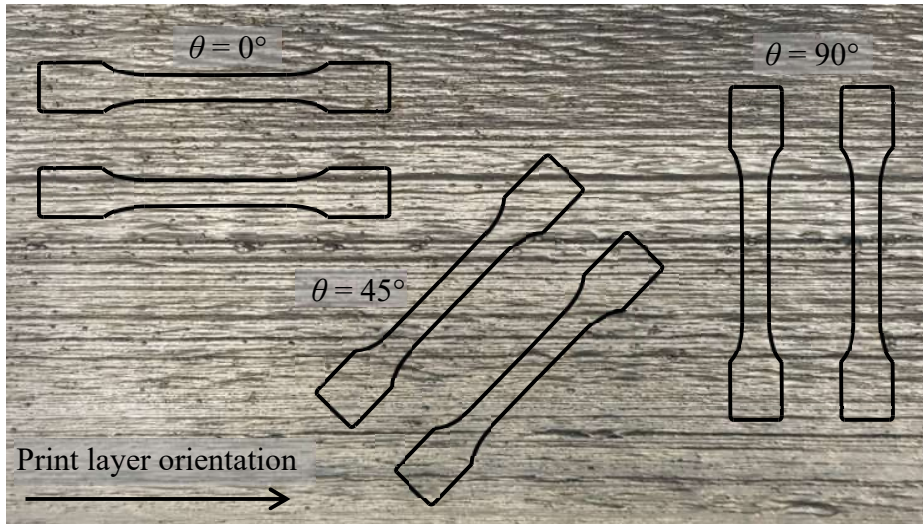


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704 Figure 2: Overall view of MX3D Bridge (Gardner *et al.*, 2020)

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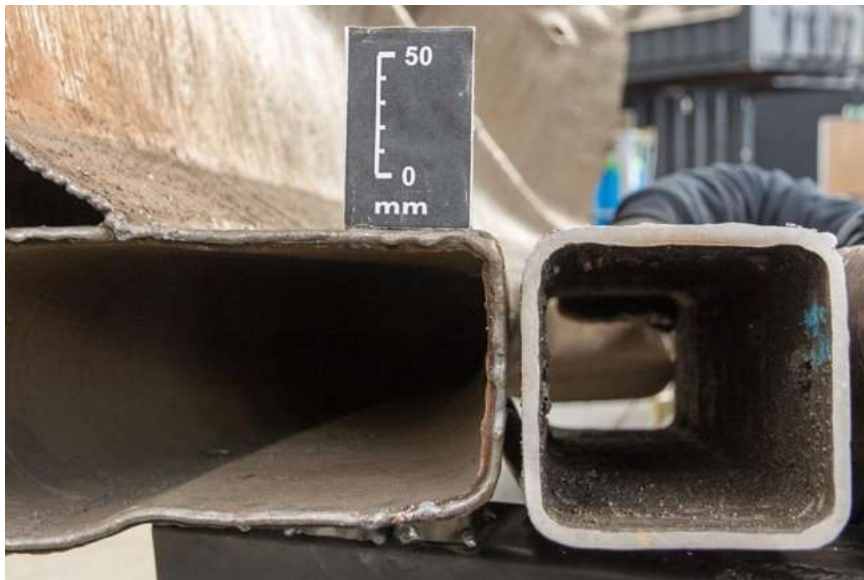
Figure 3: Orientation of tensile coupons extracted from WAAM plate relative to print layer orientation (Kyvelou *et al.*, 2020)

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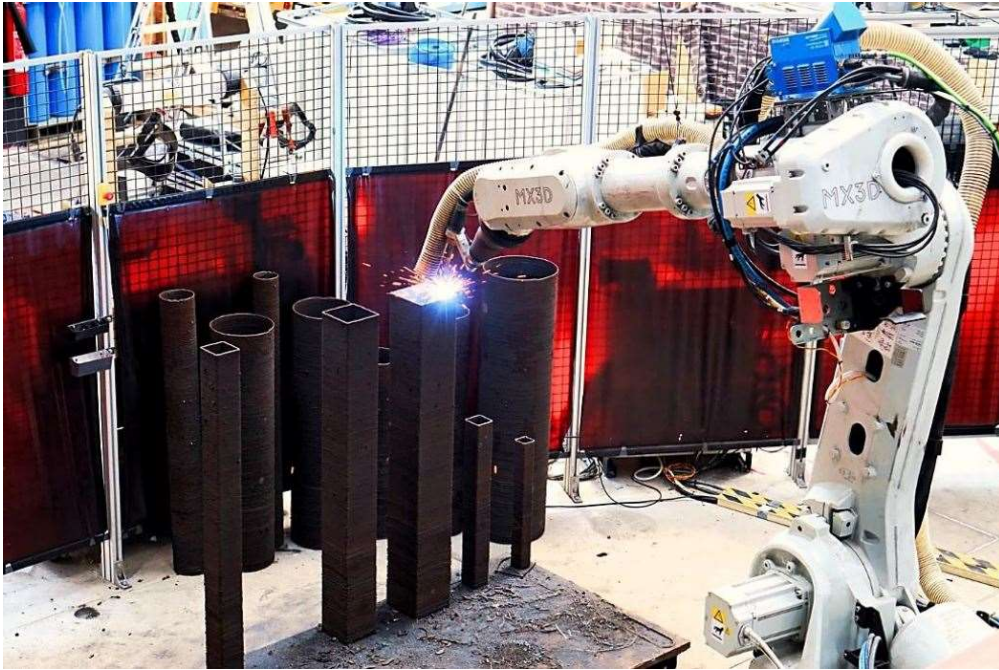
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Figure 4: Comparison between SHS specimen 120×120×8.0-450-F and its corresponding part of the MX3D Bridge (Gardner *et al.*, 2020)

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Figure 5: Printing of a subset of the SHS (and CHS) WAAM specimens

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(a) Cutting using a band saw



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(b) Sand-blasting with glass beads

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Figure 6: Preparation of typical WAAM test specimen



(a) Archimedes' measurements
(shown for a CHS)



(b) Laser scanning

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Figure 7: Employed methods for geometrical measurements of WAAM stub columns

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(a) Mixing silicone parts

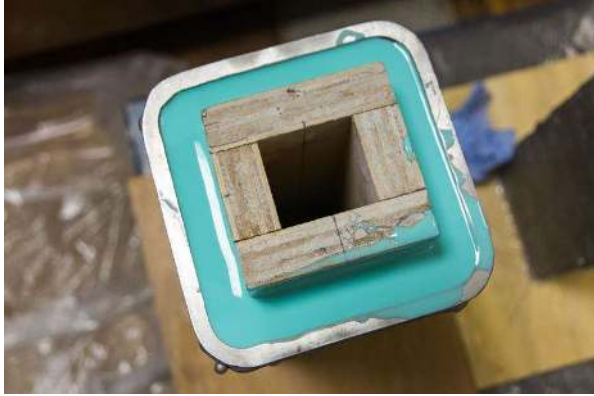
(b) Degassing

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Figure 8: Preparation of silicone mixture

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(a) Wooden insert within SHS

(b) Removal of silicone cast from within the SHS

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Figure 9: Process followed to produce silicone replicas of the inner surface of the SHS specimens

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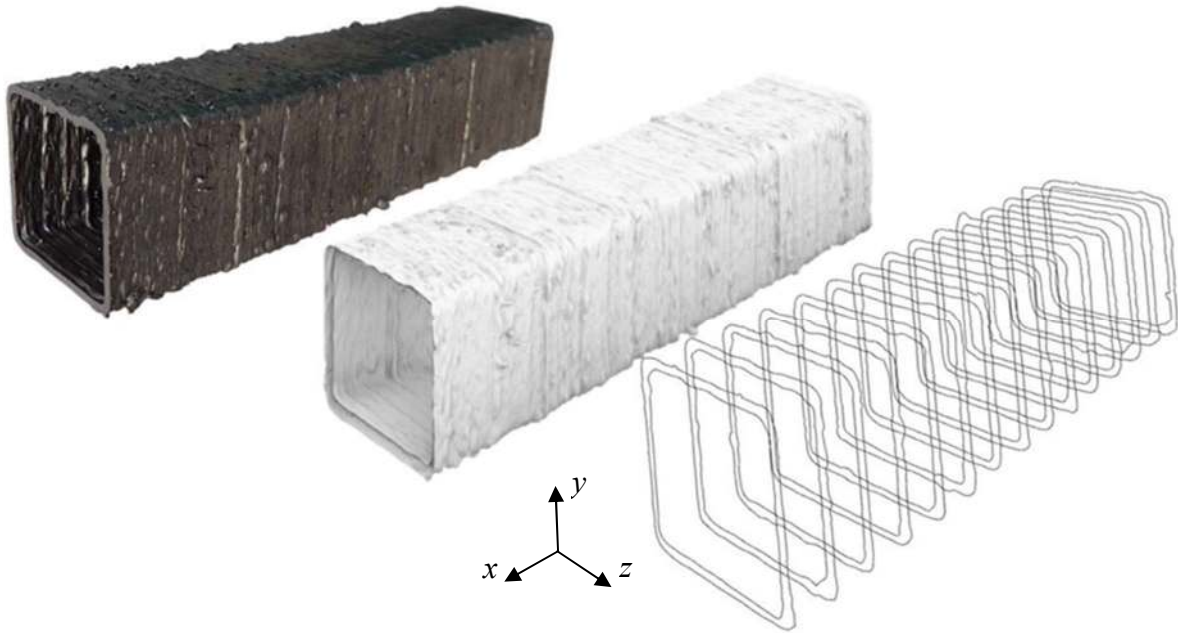
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Figure 10: Silicone replica of the inner surface profile of typical SHS specimens



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752 Figure 11: Specimen, 3D CAD model and cross-sectional contours of a typical SHS specimen

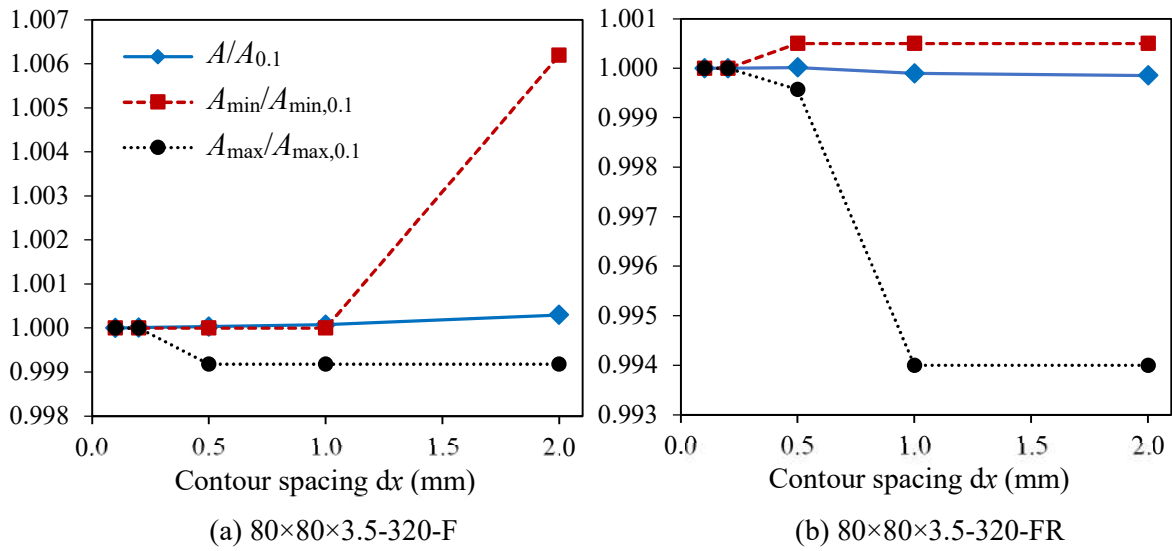
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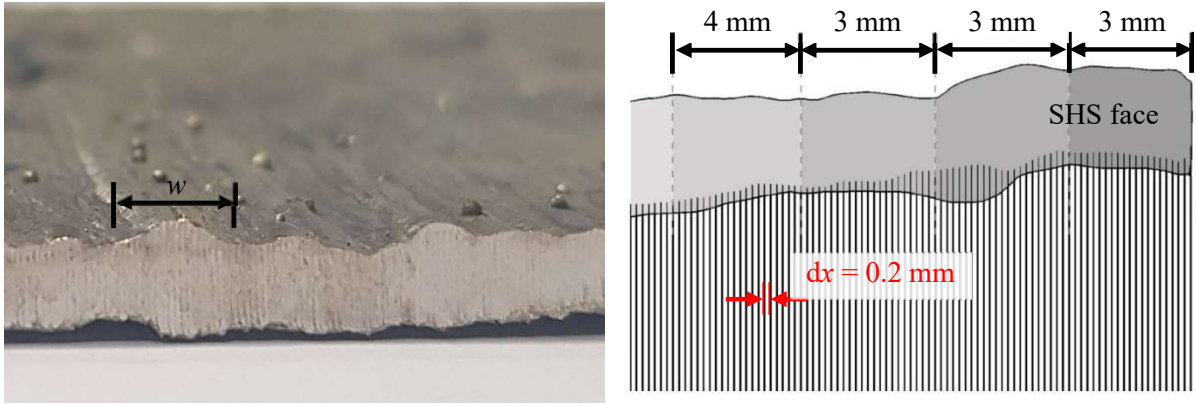


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Figure 12: Results of sensitivity study on contour spacing

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Figure 13: Typical deposition path widths compared to 0.2 mm contours

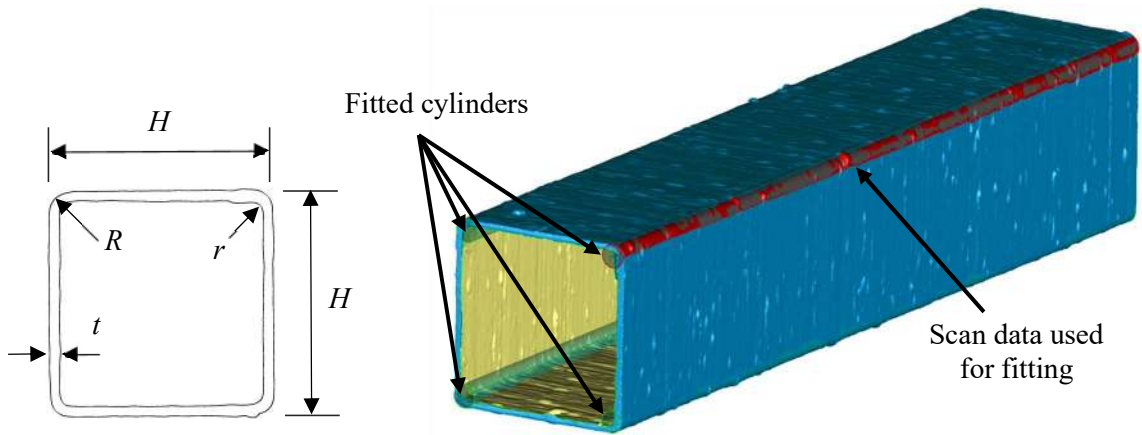
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Figure 14: Cross-section geometry and derivation of the outer corner radius R from scanned data

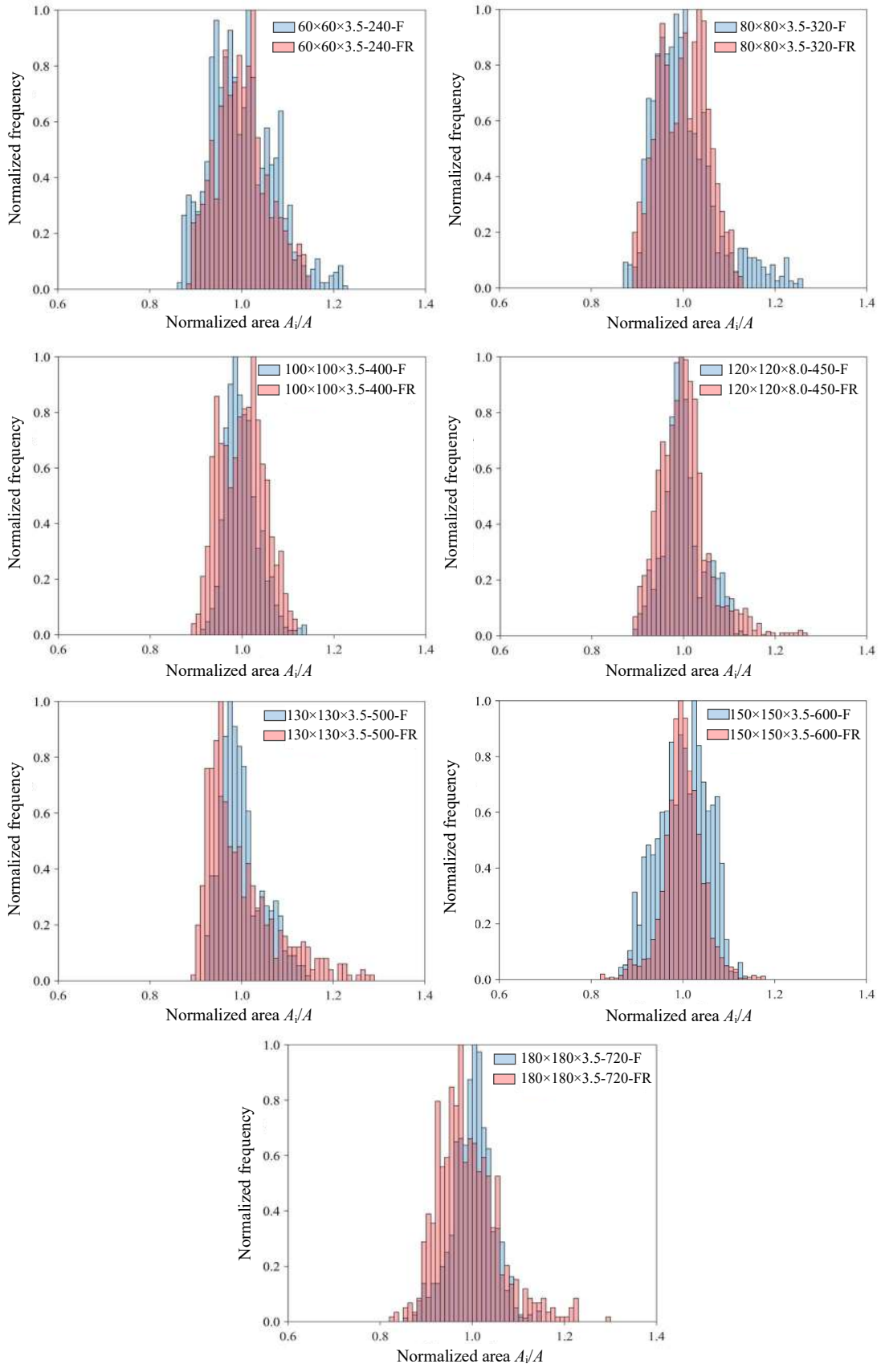
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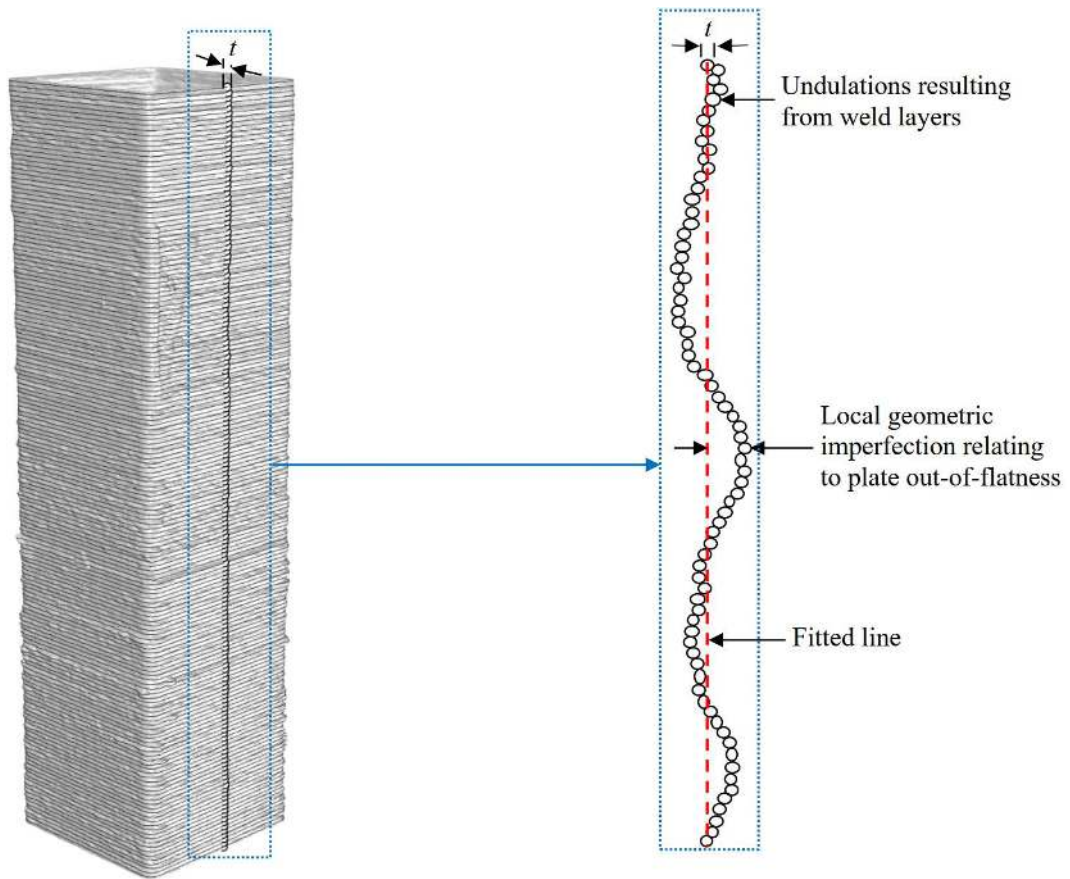
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Figure 15: Distribution of normalized areas A_i/A of examined specimens



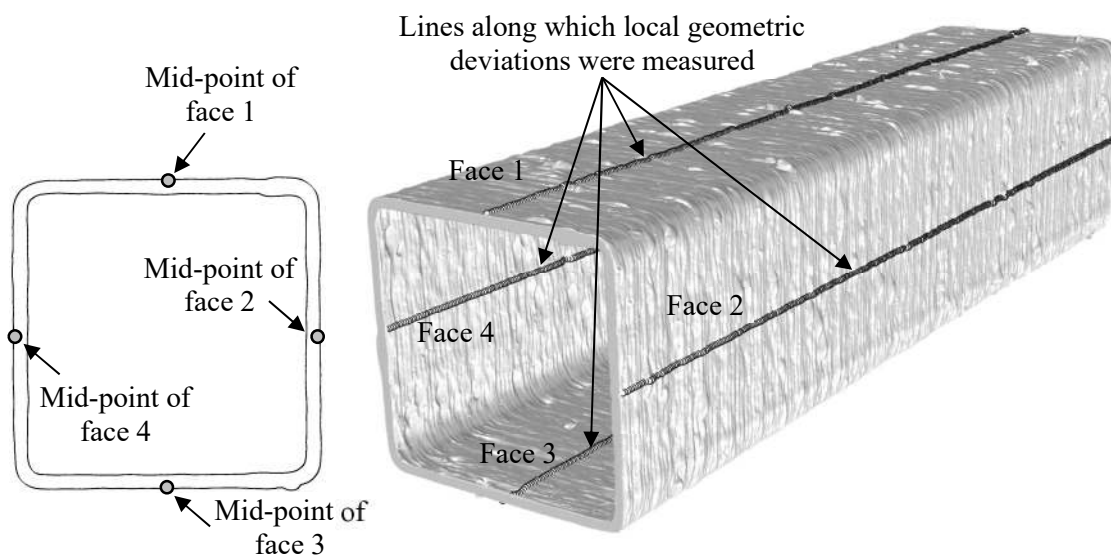
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Figure 16: Distinction between surface undulations and local geometric imperfections

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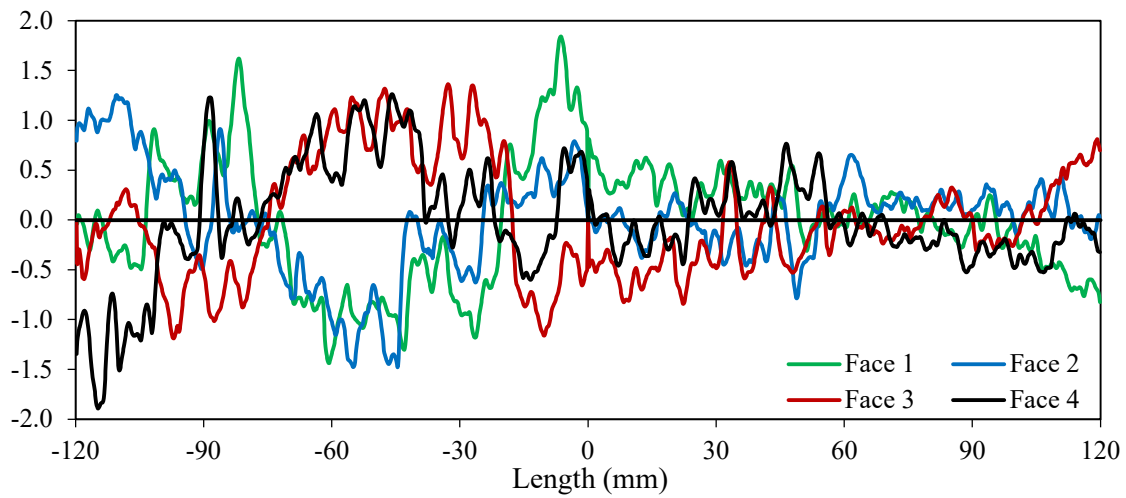


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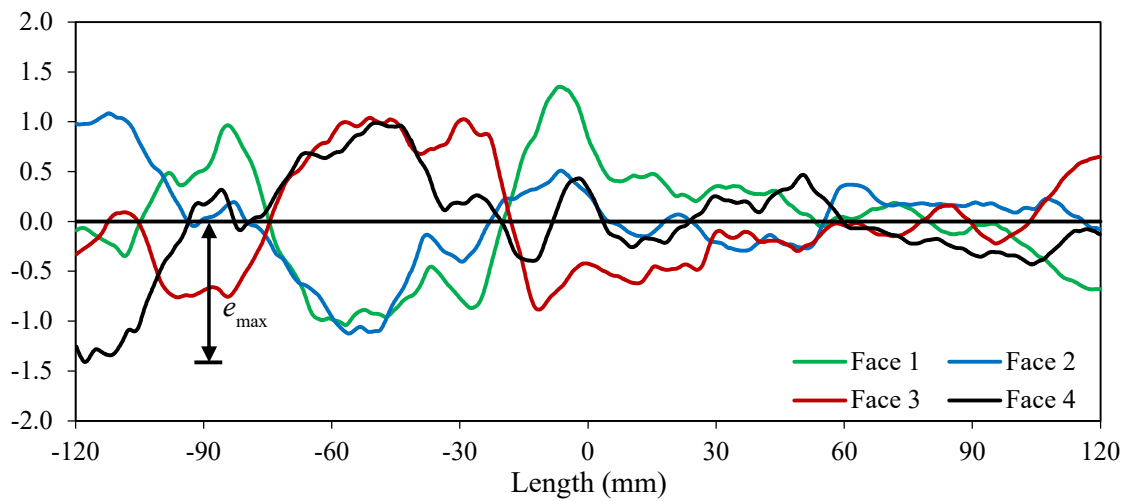
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Figure 17: Determination of local geometric imperfections

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(a) Imperfection distributions before smoothing



(b) Imperfection distributions after smoothing

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785 Figure 18: Smoothing of local imperfection distributions to remove influence of unwanted features
786 (e.g. weld beads) for a typical specimen (60×60×3.5-240-F)

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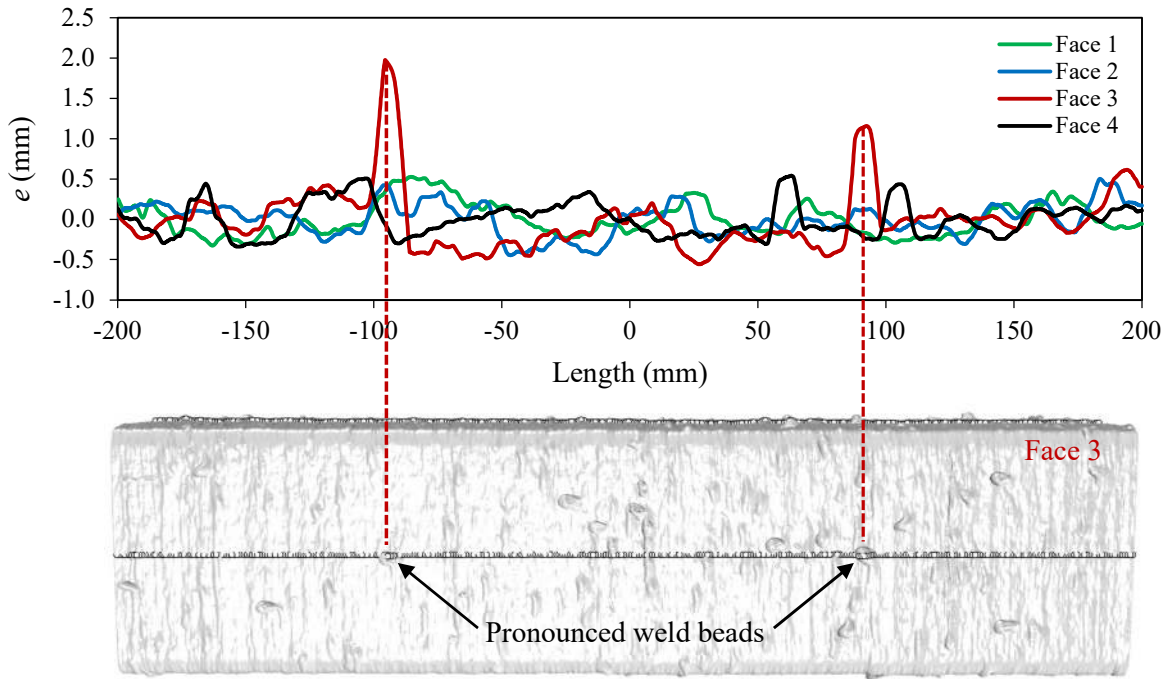
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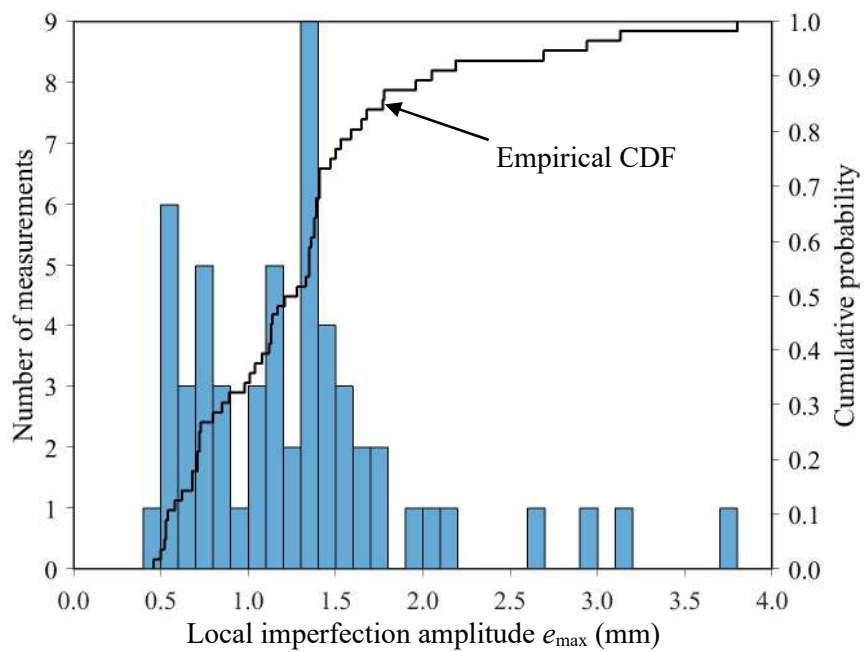


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Figure 19: Measured local imperfection distributions for specimen 100×100×3.5-400-F

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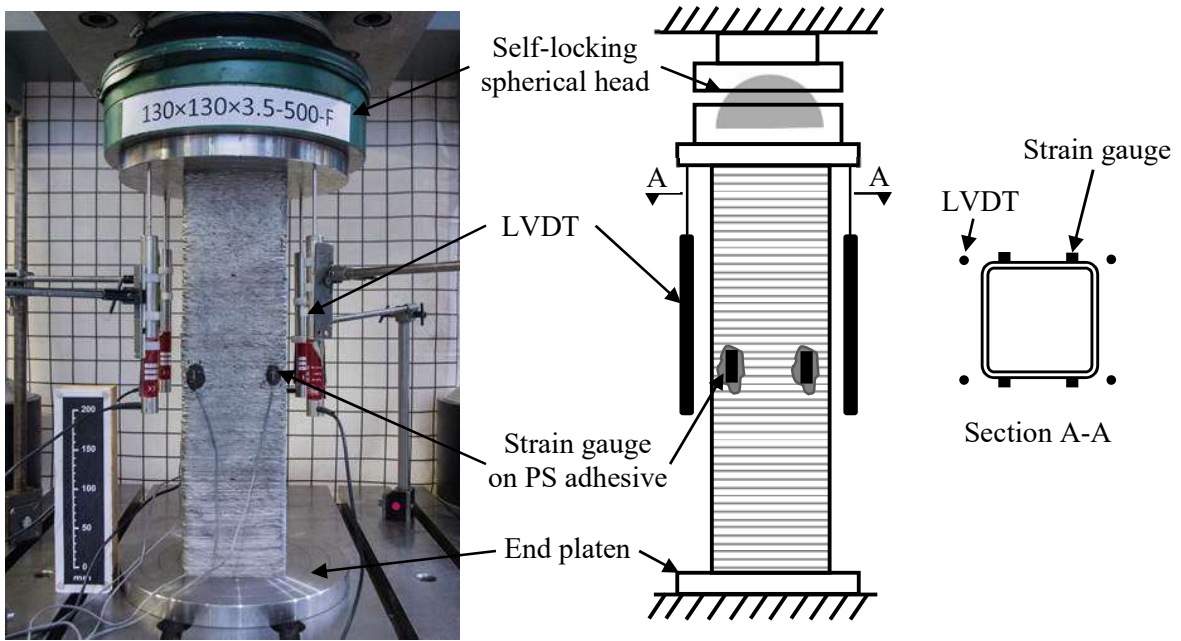
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Figure 20: Histogram and CDF of local geometric imperfection amplitudes

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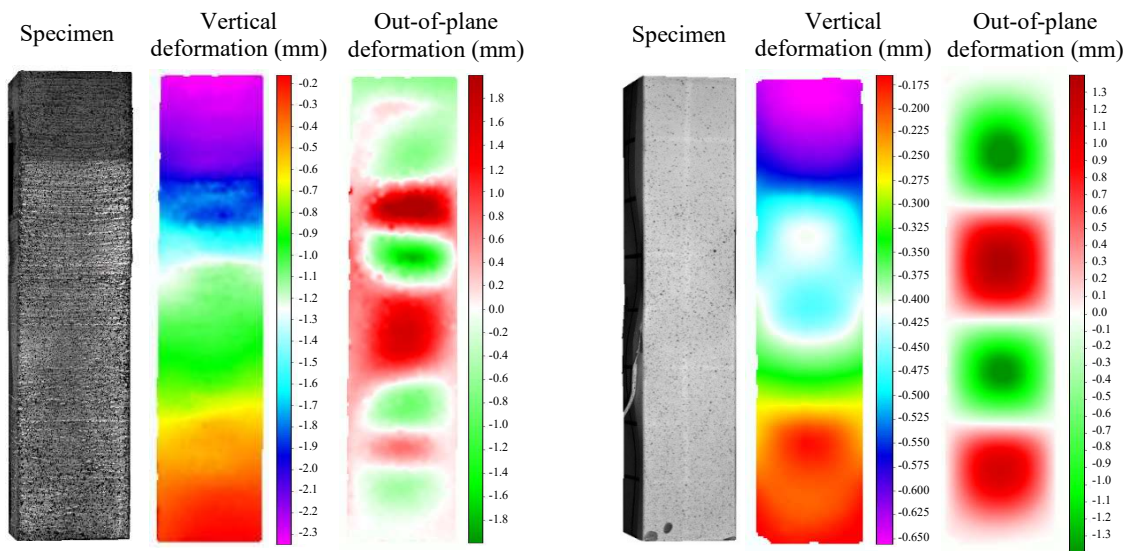
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Figure 21: Experimental setup for stub column tests

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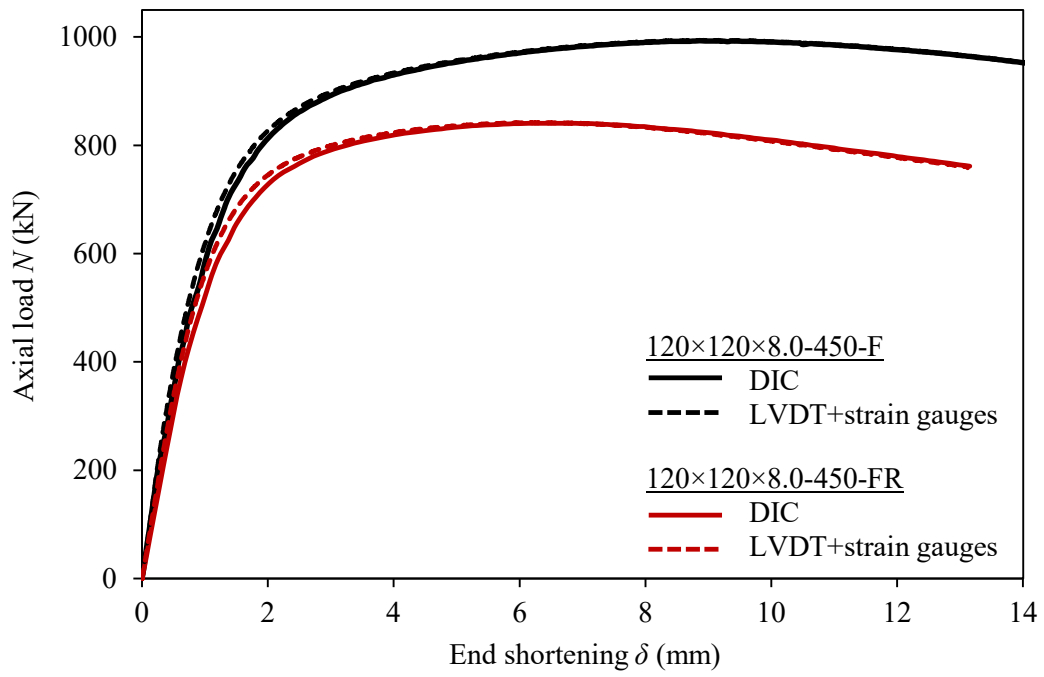
(a) WAAM SHS, $\bar{\lambda}_p = 1.13$
(180×180×3.5-720-FR)

(b) PBF SHS, $\bar{\lambda}_p = 1.12$

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Figure 22: DIC analysis of typical WAAM and PBF specimens at ultimate load

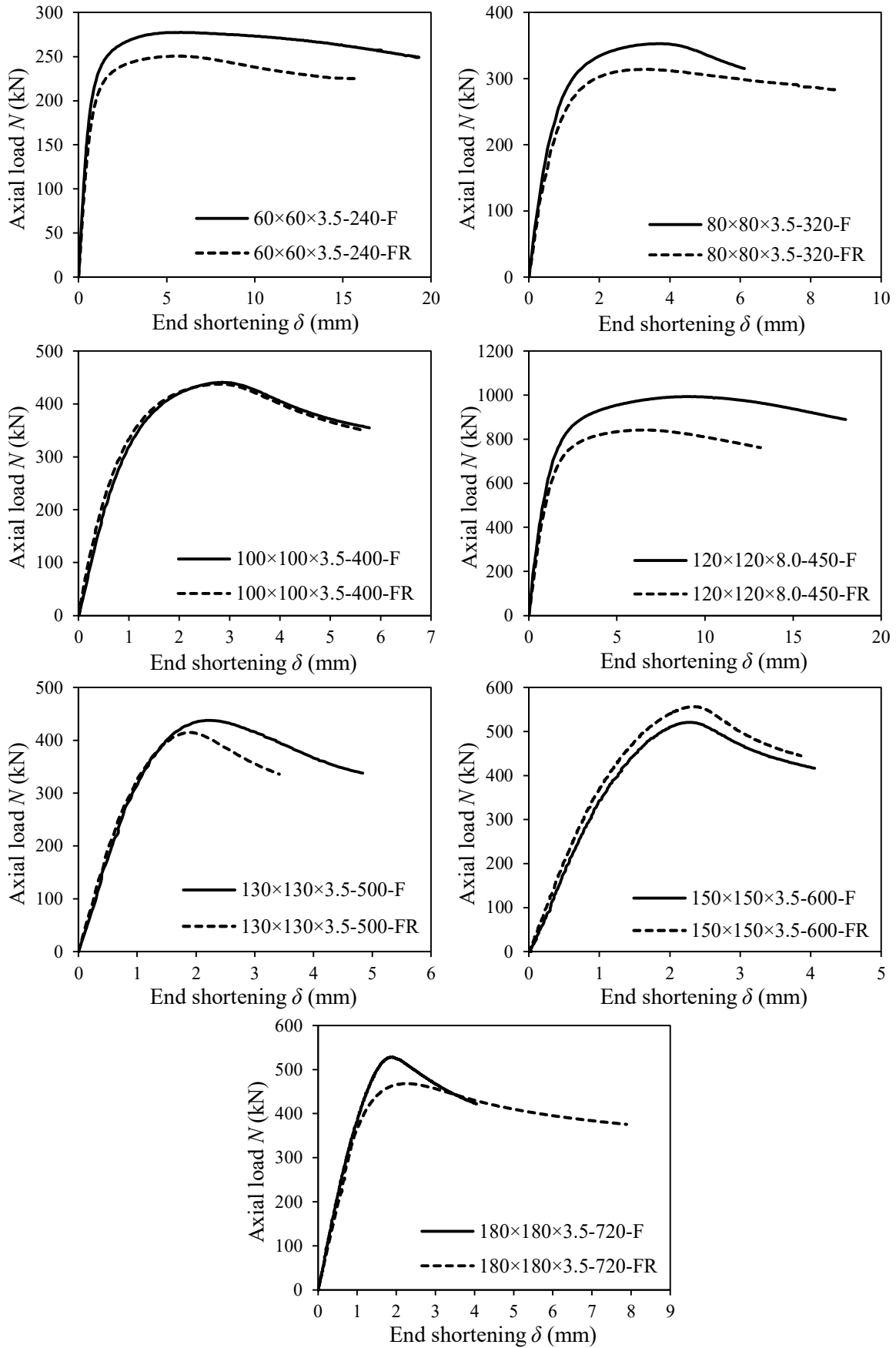


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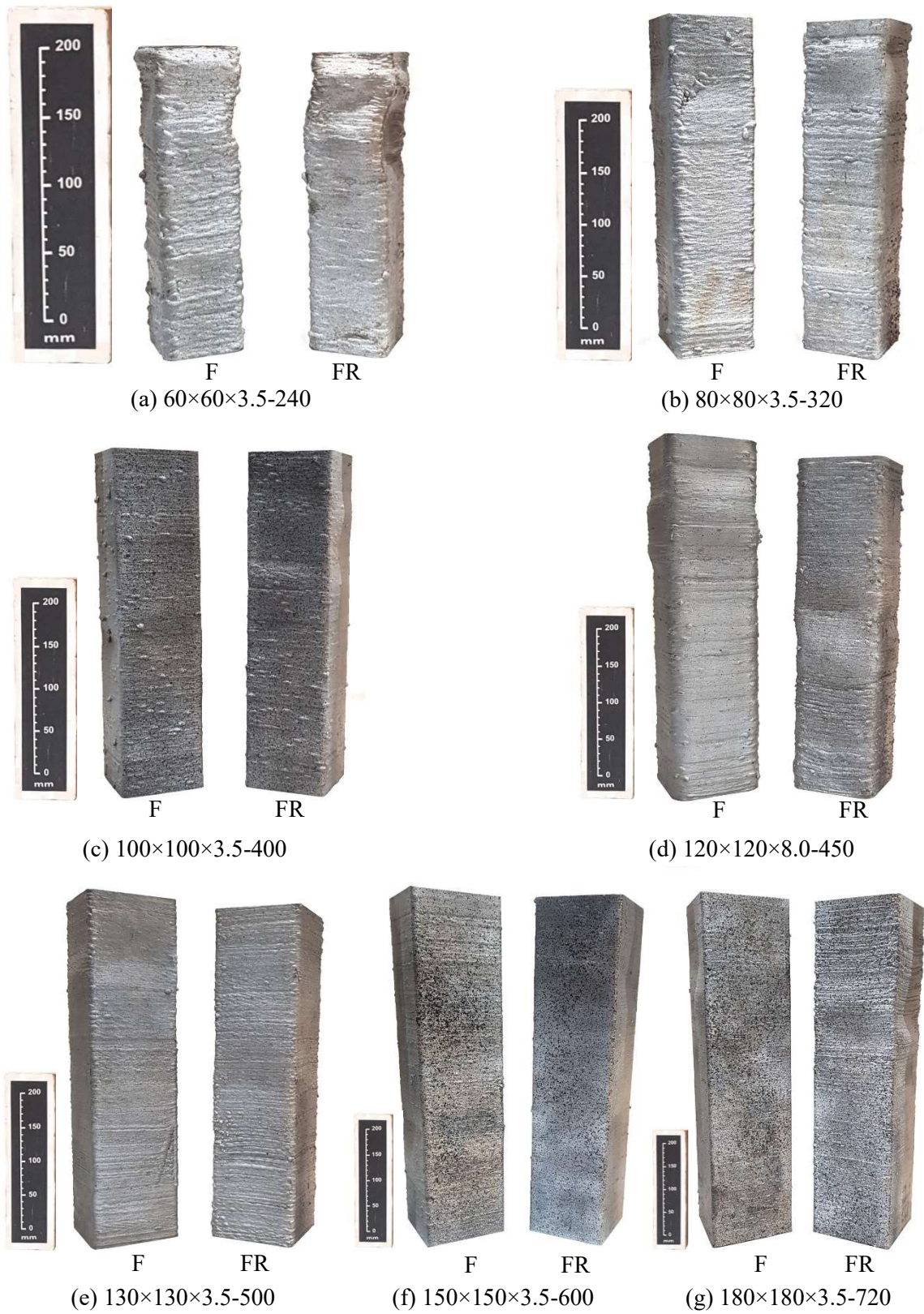
Figure 23: Typical comparisons of load-end shortening curves derived using DIC and LVDT+strain gauge data



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Figure 24: Load-end shortening curves of tested stub columns

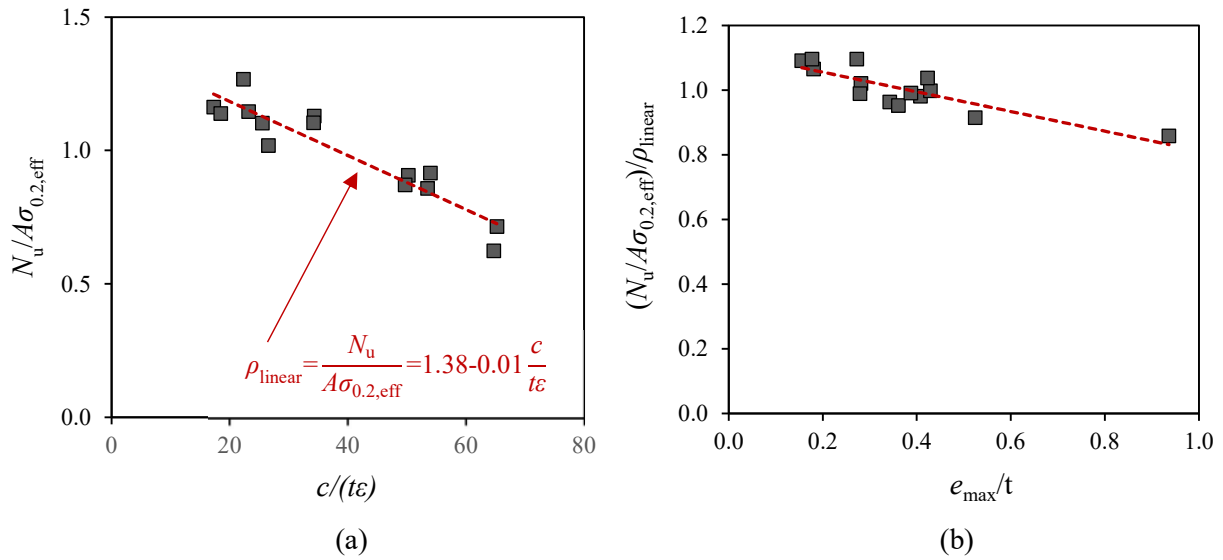


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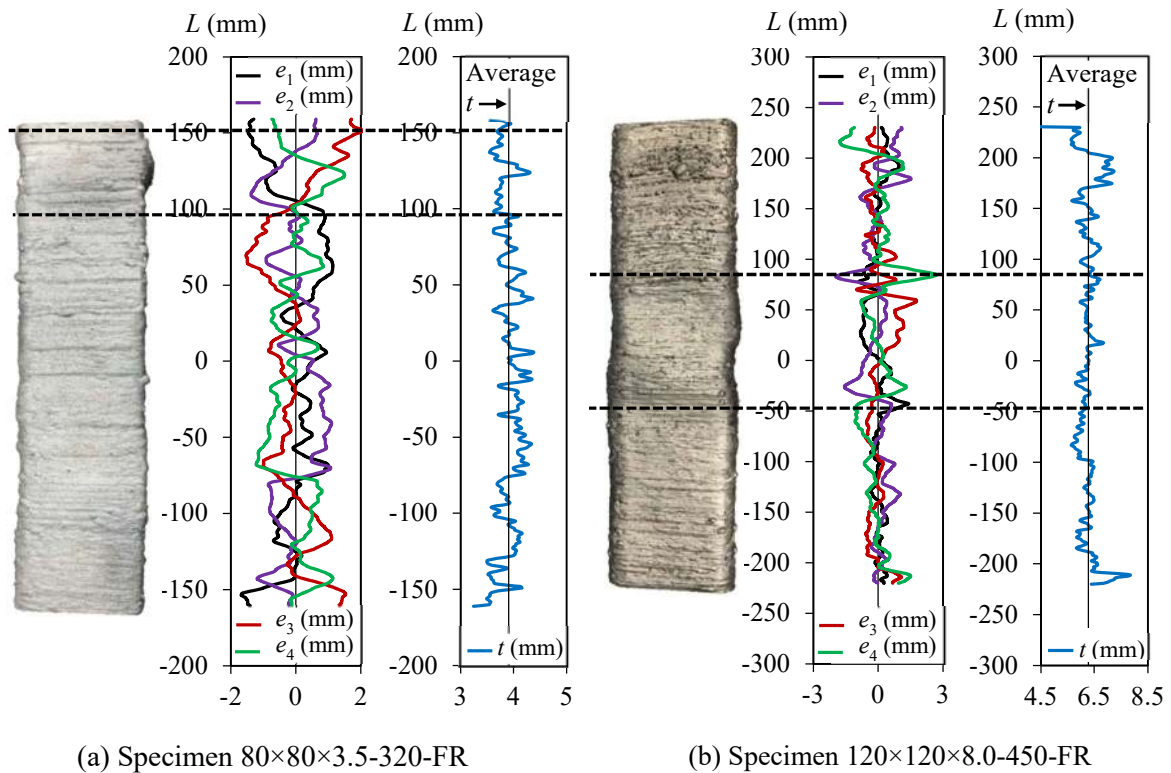
Figure 25: Deformed shapes of tested stub columns



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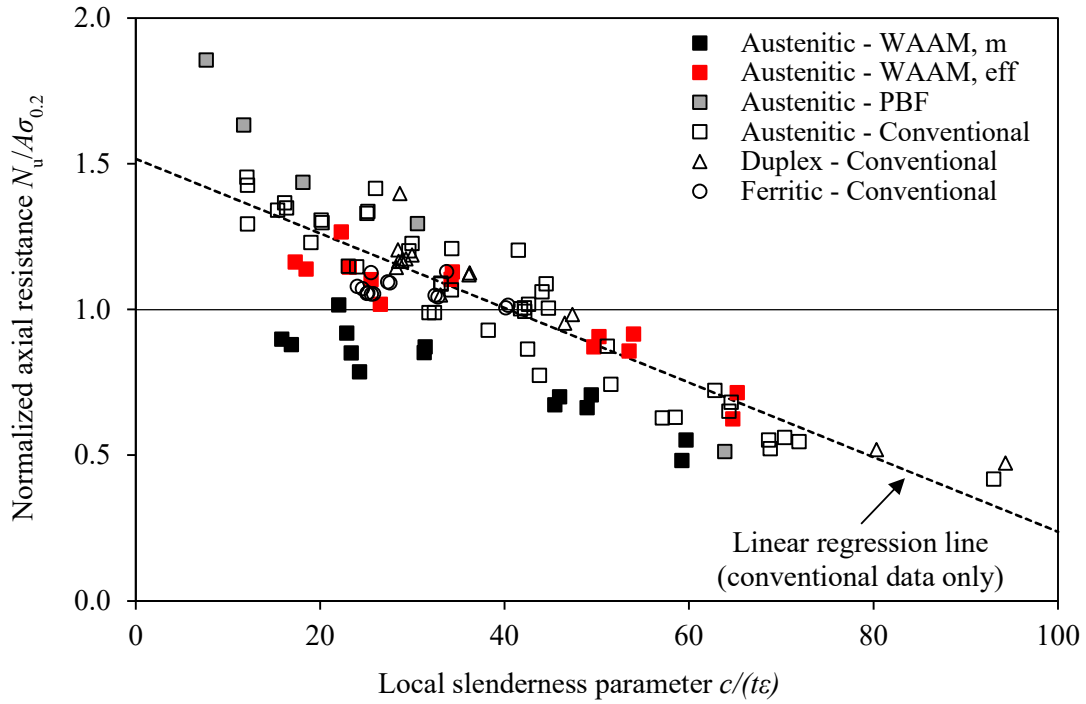
818 Figure 26: (a) Normalized compressive capacities of specimens and (b) variation of relative response
819 of specimens with normalized geometric imperfection amplitude

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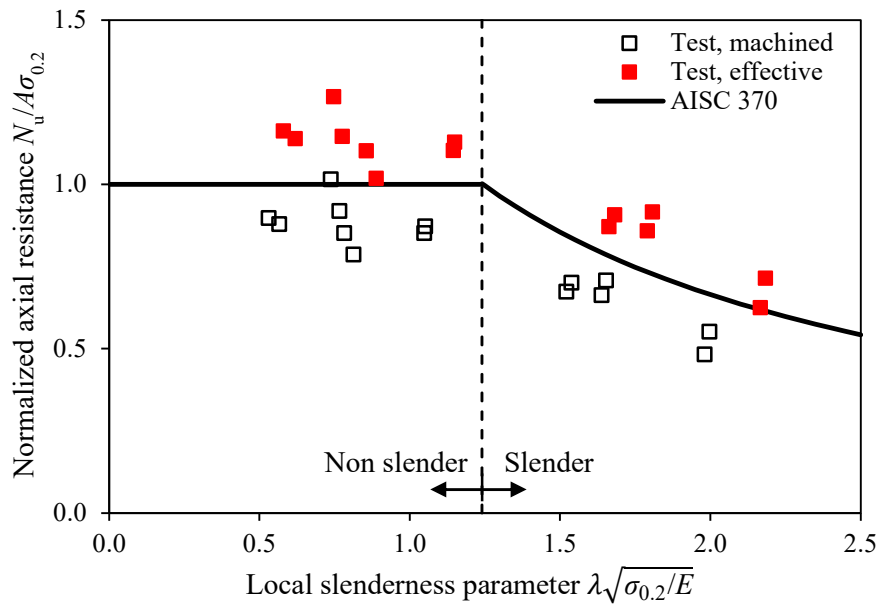
822 Figure 27: Correlation between geometric variability and failure locations for typical specimens of (a)
823 3.5 mm and (b) 8.0 mm nominal thickness



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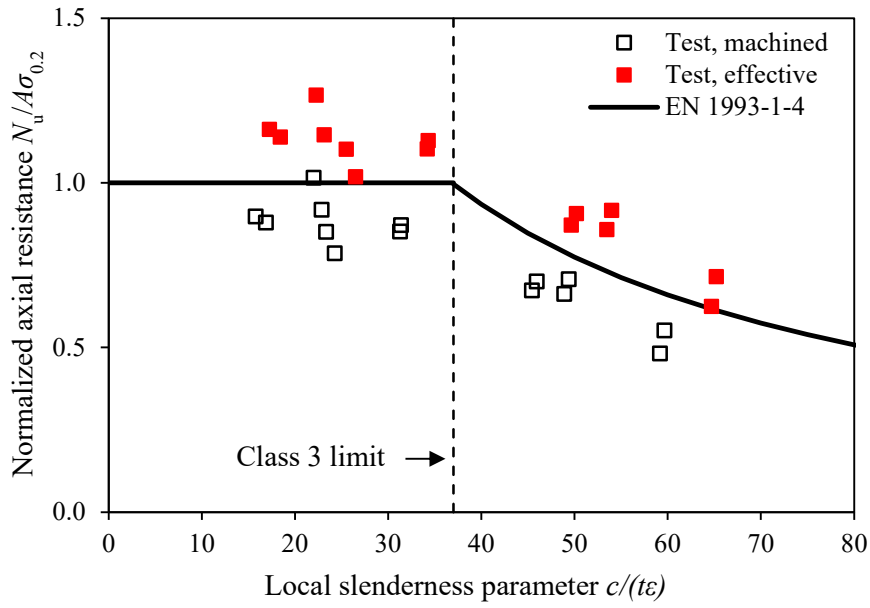
825 Figure 28: Comparison of normalized compressive capacities of WAAM SHS with those of PBF SHS
 826 and conventionally manufactured cold-formed SHS

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829 Figure 29: Comparison of compressive capacities of WAAM SHS with AISC 370 capacity
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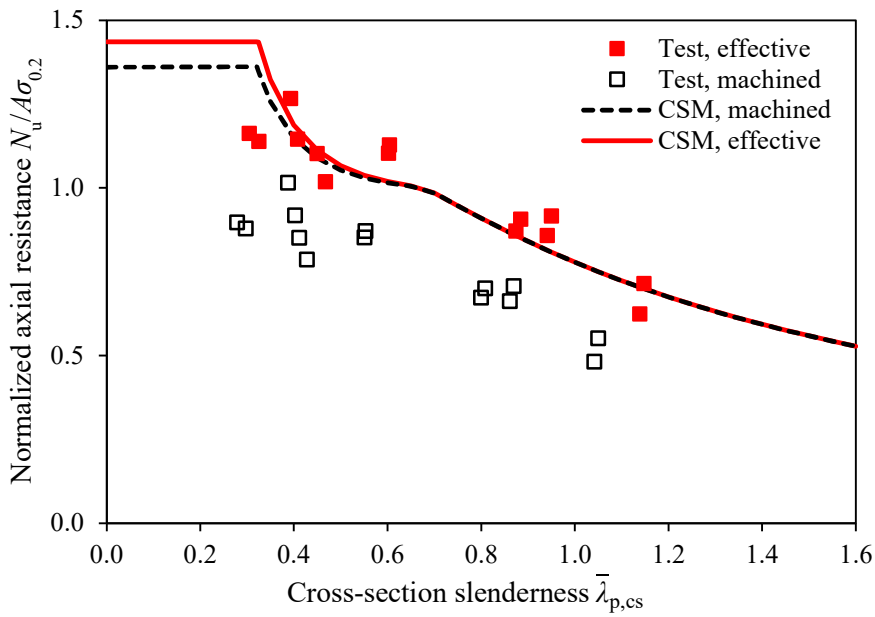
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Figure 30: Comparison of compressive capacities of WAAM SHS with EN 1993-1-4 capacity predictions

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Figure 31: Comparison of compressive capacities of WAAM SHS with CSM capacity predictions

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