

Research Article

The Influence of Aggregate Size and Binder Material on the Properties of Pervious Concrete

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Specimens were prepared by altering parameters such as aggregate sizes, binder materials, and the amounts of binder used and were subsequently tested by using permeability, porosity, mechanical strength, and soundness tests. The results indicated that the water permeability coefficient and connected porosity decreased as the amount of binder used increased and increased with increasing aggregate size. In the mechanical strength test, the compressive, splitting tensile, and flexural strengths increased as the amount of binder used increased and decreased with the increase of aggregate size. Highly viscous binder enhanced compressive strength, water permeability, and the resistance to sulfate attacks. In the mechanics and sulfate soundness tests, the mix proportion of alkali-activated slag paste used in this study exhibited a superior performance than the Portland cement pervious concrete (the control) did, but the difference in water permeability between the two types of concrete was insignificant. The mix proportions of cement paste containing 20% and 30% silica fume exhibited less mechanical strength than the control did. Moreover, compared with the control, the cement paste containing silica fume demonstrated poor resistance to sulfate attacks, and the difference in the water permeability between such specimen and the control was not noticeable.

1. Introduction

Pervious concrete, also known as no-fines concrete, is a type of porous composite material that can be regarded as concrete composed of minimal to fine uniformly graded aggregates and a limited amount of cement paste. Because of its pervious properties, the pervious concrete pavement provides a better temperature and humidity exchange between atmosphere and earth than the impervious pavement such that the so-called “heat-island” effect in the urban region can be reduced. In addition, pervious concrete is regarded as a remedy for the flood control due to its excellent water permeability. Also, it is known that the pervious concrete pavement reduces traffic noise and has an excellent antiskid performance.

However, a lower mechanical strength of pervious concrete constrains the application of pervious concrete. Due to the porosity inside the pervious concrete, its compressive strength does not satisfy the minimal requirement for the structural concrete (21 MPa for 28-day compressive strength). Therefore, most applications for pervious concrete are parking lot pavement, pedestrian walkway, bike route, and places

where concrete compressive strength is not important. In the following, literature survey for pervious concrete is given.

Generally, because of the high amount of connected voids in pervious concrete, compressive strength is relatively low; thus, the strength of pervious concrete can be improved using the following strategies [1]:

- (1) enhancing the characteristics of binder by decreasing the water-cement (w/c) ratio and adding pozzolanic materials such as silica fume;
- (2) adopting different binder materials such as using epoxy to replace cement pastes;
- (3) applying slight pressure and increasing the temperature in the curing stage.

Marolf et al. studied the effect of aggregate size and gradation on the acoustic absorption for pervious concrete. They reported that pervious concrete mixtures with single-sized aggregates provide substantial improvement to sound absorption as compared with conventional concrete [2]. Park et al. studied the sound absorption properties of pervious

concrete using recycled aggregate and various target void ratios. They reported that the sound absorption characteristics of the porous concrete using recycled waste concrete aggregate showed that the Noise Reduction Coefficient (NRC) was optimum at the void ratio of 25% but the percent content of the recycled aggregate had very little influence on the NRC. Therefore, they concluded that the optimum void ratio is 25% and the recycled aggregate is 50% [3].

Park and Tia studied water purification effect of pervious concrete. They found that a porous concrete with a smaller size of aggregate and a higher void content was found to have superior ability of the removal of the total phosphorus and total nitrogen in the test water. They concluded that this effect is due to the large specific surface area of the porous concrete [4]. Huang et al. reported the properties of polymer-modified pervious concrete and they concluded that it was possible to produce pervious concrete mixture with acceptable permeability and strength through the combination of latex and sand [5]. Crouch et al. studied the aggregate effect on the static elastic moduli of pervious concrete and they reported that an increased aggregate amount resulted in a statistically significant decrease in both compressive strength and static elastic moduli due to the subsequent decrease in paste amount [6]. Chindapasirt et al. studied the effects of aggregate size and binder strength on the compressive strength and void ratio of pervious concrete [7]. Chindapasirt et al. also reported that good porous concretes with void ratio of 15–25% and strength of 22–39 MPa are produced using paste with flow of 150–230 mm and top surface vibration of 10 s with vibrating energy of 90 kN m/m² [8].

Neithalath et al. used the values of porosity and the morphologically determined pore sizes, along with the pore phase connectivity represented using an electrical conductivity ratio, in a Katz-Thompson type relationship to predict the permeability of pervious concretes [9]. Lian et al. developed a new model, which was based on the Griffith theory, to predict the compressive strength of pervious concrete using its porosity [10]. Bentz used computer to simulate various virtual pervious concrete microstructural models and compares their percolation characteristics and computed transport properties to those of real world pervious concretes [11].

Putman and Neptune evaluated different pervious concrete test specimen preparation techniques in an effort to produce specimens having properties similar to in-place pervious concrete pavement [12]. Lian and Zhuge studied the optimal mix design for pervious concrete [13]. Kim and Lee studied the influence of cement flow and aggregate type on mechanical and acoustic properties of pervious concrete [14]. Fujiwara et al. reported that a high strength pervious concrete could be made by coating the coarse aggregates with a high-strength mortar, then applying vibration to fuse them [15]. Safiuddin and Hearn compared the permeable porosity obtained from three different ASTM saturation techniques, namely, cold-water saturation (CWS), boiling-water saturation (BWS), and vacuum saturation (VAS). They concluded that vacuum saturation technique is more efficient

than cold-water or boiling-water saturation and therefore this technique should be recommended for measuring the permeable porosity of concrete [16]. Haselbach et al. developed a theoretical model between the effective permeability of a sand-clogged pervious concrete block, the permeability of sand, and the porosity of the unclogged block [17]. Tho-In et al. have tried to use alkali-activated high-calcium fly ash to make pervious concrete. They found that the high-calcium fly ash geopolymer binder could be used to produce pervious concrete with satisfactory mechanical properties [18].

In this study, the performance of pervious concrete was investigated by conducting various tests (i.e., mechanical, permeability, soundness, porosity, and unit weight tests) by changing parameters, which include aggregate size and the type, amount, and w/c ratio of binders (for filling the voids among aggregates). However, in addition to featuring excellent water permeability, pervious concrete is expected to exhibit a mechanical performance to a certain level. Generally, once pervious concrete has excellent water permeability then its strength becomes poor. Various strategies can be used to improve the strength of pervious concrete while achieving the required permeability. By altering the parameters of pervious concrete, this study obtained the optimal relationship between strength and water permeability. Subsequently, the required mix proportion was attained through relevant analyses, and different binders were applied to multiple types of pervious concrete to determine the mechanical characteristics and water permeability of pervious concrete. In the experiment, the control group comprised pure cement paste as binding material, and the experiment group involved two types of cement pastes, one containing silica fume and one with alkali-activated slag. The feasibility of enhancing mechanical strength or changes in water permeability can be investigated using different binders.

2. Experiment Design

The experiment involved two stages. The first part involved considering three crucial parameters, which were aggregate size, the volume percent of aggregate voids filled with binders, and the w/c ratio of binders, for determining the relationship between water permeability and mechanical strength. Next, the feasibility of enhancing mechanical strength was determined by varying the types of binders used based on the mix proportions obtained from the first part of the experiment. The second stage was aimed at investigating cement pastes containing silica fume and alkali-activated slag. Varying amounts of silica fume were used to replace cement; in other words, cement combined with pozzolanic material was used instead of pure cement paste. Alkaline-activated slag paste with different liquid/slag (L/Sg) ratios was used to replace the pure cement pastes. Finally, the experiment and control (pure cement paste only) groups were compared to investigate the influences of different binding materials on the properties of pervious cement.

Pervious concrete is primarily composed of aggregates and binding materials, which bind aggregates and fill voids among aggregates to form porous, permeable concrete.

TABLE 1: Experimental variables of the two-stage experiment.

(a)				
Experimental variables of Stage 1				
Aggregate code and size	A	B	C	D
	0.32 cm	0.48 cm	0.64 cm	0.95 cm
Binder	Cement paste			
w/c ratio	0.25 0.35 0.452			
Total volume percent of filled voids	50%	60%	70%	80%
Aggregate code and size	B (0.48 cm)			
(b)				
Experimental variables of Stage 2				
Binder	Alkali-activated slag paste			Silica-fume cement paste
Total volume percent of filled voids	80%			
L/Sg or w/c ratio	0.35	0.4	0.45	0.35
Substitution ratio	—			10% 20% 30%

Regarding applications in engineering, pervious concrete should have a certain level of water permeability before its mechanical strength is enhanced. Therefore, designing the void content inside pervious concrete is the key that affects the overall property of the material. High porosity in pervious concrete indicates excellent water permeability but poor mechanical strength because of insufficient compactness. By contrast, low porosity can enhance mechanical strength but might decrease water permeability. Although such judgment can be preliminarily made based on physics concepts, when variables such as binder characteristics and aggregate size are considered, whether or not the effect of porosity on the property of pervious concrete conforms to common concepts must be determined by experimental studies. The parameter design concepts are explained as follows.

- (i) Aggregate size: in this experiment, uniformly graded aggregates (narrow gradation) were used, which are aggregates of a single size. In addition, aggregate size affects the water permeability and mechanical strength of pervious concrete.
- (ii) W/c ratio: in pervious concrete, when designing the w/c ratios of cement pastes, workability, water permeability, and mechanical strength should be considered. The workability of cement pastes influences the overall performance of pervious concrete.
- (iii) The volume percent of aggregate voids filled with binders (hereafter referred to as volume percent of filled voids): this volume percent refers to the amount of binders used in pervious concrete. This amount also influences the water permeability and mechanical strength of pervious concrete.
- (iv) Binders: mechanical strength is enhanced by altering binders, which influence the property of pervious concrete.

The binders used in the first stage were cement pastes made by mixing cement with water. Superplasticizers were added to the binder with a low w/c ratio of 0.25 to increase

the binder fluidity. In the second stage, cement pastes containing silica fume and alkali-activated slag were used. Based on the first stage of the experiment, the control group involved the following parameters: B aggregate size, w/c ratio of 0.35, and 80% volume percent of filled voids. Subsequently, comparison test was conducted in which the mix proportion of binders was altered. The experimental variables are shown in Table 1. In this table, the aggregate size means the minimum size of the sieve grid, for which aggregates pass through the sieve with greater sieve grid and stay in the current one. The total volume percent of filled voids can be understood in the following. First, we packed aggregates inside a unit volume and we could calculate the volume of aggregate by the weight of aggregate and the specific weight of aggregate. Then, we subtracted the total volume by the aggregate volume and obtained the void volume. In such a manner, the total volume percent of filled voids of 50% means that we use paste to fill 50% void volume.

2.1. Materials. This study used the Portland Type I cement produced and coarse aggregates of four different sizes as those in Table 1. The aggregates were pebbles and their properties are presented in Table 2.

Superplasticizers are high-performance carboxylic plasticizers. The chemical compositions of sodium silicate, sodium hydroxide, and phosphoric acid used for making alkali-activated slag paste are shown in Tables 3, 4, and 5. The phosphoric acid is used as the retarder for avoid quick-setting problem of the alkali-activated slag [19].

2.2. Mix Proportion Design

2.2.1. The Mix Proportion of Cement Paste as a Binder. We first determined the void volume per unit volume for Types A, B, C, and D aggregates as shown in Table 2. Subsequently, 50%, 60%, 70%, and 80% void volumes were filled using cement pastes. In addition, 5% superplasticizers (by the cement weight) were added to cement pastes with a w/c ratio

TABLE 2: Physical properties of coarse aggregates.

Code	A	B	C	D
Nominal size	1/8 in.	3/16 in.	1/4 in.	3/8 in.
Maximum size	0.48 cm	0.64 cm	0.95 cm	1.27 cm
Range of particle sizes	0.24–0.48 cm	0.48–0.64 cm	0.64–0.95 cm	0.95–1.27 cm
Specific gravity	2.65	2.69	2.66	2.72
Void volume per unit volume	37.3%	37.5%	36.8%	38.3%

TABLE 3: Chemical composition of sodium silicate.

Item	Sodium silicate
Test item	Test result
Insoluble residue	Max. 0.01%
Silicon dioxide (%)	37.0%
Sodium oxide (%)	17.7%
Mole ratio	2.16
Iron (Fe)	Max. 0.02%

TABLE 4: Chemical composition of sodium hydroxide.

Test item	Test results
Chloride (Cl)	Max. 0.005%
Sulfate (SO ₄)	Max. 0.003%
Silicate (SiO ₂)	Max. 0.01%
Phosphate (PO ₄)	Max. 0.001%
Heavy metals (arsenic and lead)	Max. 0.001%
Iron (Fe)	Max. 0.0007%
Aluminum (Al)	Max. 0.003%
Calcium (Ca)	Max. 0.001%
Magnesium (Mg)	Max. 0.0005%
Potassium (K)	Max. 0.1%
Total nitrogen (N)	Max. 0.001%
Arsenic (As)	Max. 0.0002%
Sodium carbonate (Na ₂ CO ₃)	Max. 2.0%
Assay (NaOH)	Max. 95.0%

of 0.25 to enhance fluidity. The mix proportion design is shown in Table 6.

2.2.2. The Mix Proportion of Alkali-Activated Slag Paste as a Binder. The alkali activator was made by first adding sodium hydroxide to sodium silicate and then mixing the mixture uniformly before phosphoric acid (retarder) was added. The mix proportion design is shown in Table 7. In this table, the L/Sg ratio means the weight ratio between liquid phase activator and slag. The activator was made by mixing the hydroxide and sodium silicate and to reach the concentration SiO₂ = 106 g/L and Na₂O = 105 g/L. And the retarder was phosphoric acid of 0.74 M.

2.2.3. The Mix Proportion of Silica-Fume Cement Paste as a Binder. Table 8 shows the mix proportion design for silica-fume cement paste; the substitution percentages for silica fume were 10%, 20%, and 30% of the cement weight.

TABLE 5: Chemical composition of phosphoric acid.

Item	Phosphoric acid
Test item	Test results
Chloride (Cl)	Max. 0.001%
Nitrate (NO ₃)	To pass test
Sulfate (SO ₄)	Max. 0.006%
Alkali and other phosphates (sulfate)	To pass test
Substances reducing KMnO ₄	To pass test
Heavy metals (As and Pb)	Max. 0.001%
Iron (Fe)	Max. 0.005%
Arsenic (As)	Max. 0.0003%
Specific gravity	1.700–1.710
Assay	Max. 85.0%

2.3. Tests

- (1) Porosity test: the total porosity in pervious concrete includes disconnected porosity and connected porosity, which is the primary influencing factor of water permeability. A caliper was used to measure and calculate specimen volume V_1 ; the specimen was immersed in water until it is filled with water before its weight in water W_1 is measured. Subsequently, the specimen was taken out of water and dried, and then its weight in air W_2 when its weight is stable was measured. The equation for connected porosity P_1 is as follows:

$$P_1 = \left[1 - \frac{(W_2 - W_1)}{V_1} \right] * 100\%. \quad (1)$$

- (2) Unit weight: after the pervious concrete specimens solidified and were demolded, they were dried in an oven at $105 \pm 5^\circ\text{C}$ until their weights were stable. The specimens were weighed and analyzed using a caliper to measure, calculate, and obtain their volumes. Dividing the weight by volume yields the weight of pervious concrete per unit volume.
- (3) Compressive strength: compressive strength was determined based on the ASTM C39 for cylindrical concrete specimens.
- (4) Flexural strength: flexural strength was determined based on the three-point bending test.
- (5) Splitting tensile strength: this test was performed for determining the splitting tensile strength of cylindrical concrete specimens.

TABLE 6: Testing proportions of cement pastes as a binder.

Binder	Aggregate	W/c ratio	Percent of voids filled	Amount of aggregate (kg/m ³)	Amount of cement (kg/m ³)	Amount of mixing water (kg/m ³)	Amount of plasticizer (kg/m ³)
Cement paste	A	0.25	50%	1665	328	66	17
			60%	1665	394	79	20
			70%	1665	460	92	23
			80%	1665	525	105	26
		0.35	50%	1665	279	96	—
			60%	1665	335	115	—
			70%	1665	391	134	—
			80%	1665	447	154	—
		0.45	50%	1665	243	109	—
			60%	1665	291	131	—
			70%	1665	340	153	—
			80%	1665	389	175	—
	B	0.25	50%	1684	330	66	17
			60%	1684	396	79	20
			70%	1684	462	92	23
			80%	1684	568	106	26
		0.35	50%	1684	281	98	—
			60%	1684	337	118	—
			70%	1684	393	138	—
			80%	1684	449	157	—
		0.45	50%	1684	244	110	—
			60%	1684	293	132	—
			70%	1684	342	154	—
			80%	1684	391	176	—
	C	0.25	50%	1683	324	65	16
			60%	1683	389	78	19
			70%	1683	454	91	23
			80%	1683	518	104	26
		0.35	50%	1683	276	96	—
			60%	1683	331	116	—
			70%	1683	386	135	—
			80%	1683	441	154	—
0.45		50%	1683	240	108	—	
		60%	1683	288	129	—	
		70%	1683	336	151	—	
		80%	1683	384	173	—	
D	0.25	50%	1682	337	67	17	
		60%	1682	404	81	20	
		70%	1682	471	94	24	
		80%	1682	530	108	27	
	0.35	50%	1682	286	100	—	
		60%	1682	343	120	—	
		70%	1682	401	140	—	
		80%	1682	458	160	—	

TABLE 6: Continued.

Binder	Aggregate	W/c ratio	Percent of voids filled	Amount of aggregate (kg/m ³)	Amount of cement (kg/m ³)	Amount of mixing water (kg/m ³)	Amount of plasticizer (kg/m ³)
			50%	1682	249	112	—
		0.45	60%	1682	299	134	—
			70%	1682	349	157	—
			80%	1682	398	179	—

TABLE 7: The mix proportion of alkali-activated slag paste.

L/Sg	0.35	0.40	0.45
Aggregate size		B	
Volume percent of filled voids		80%	
Concentration of alkali activator	SiO ₂ = 106 g/L		Na ₂ O = 105 g/L
Amount of phosphoric acid		0.74 M	
Amount of slag (kg/m ³)	432	403	377
Amount of alkali-activated solution (kg/m ³)	151	161	170
Amount of aggregate (kg/m ³)	1684	1684	1684

TABLE 8: The mix proportion of silica-fume cement paste.

Proportion of cement replaced by silica fume	10%	20%	30%
Aggregate size		B	
Volume percent of filled voids		80%	
Amount of cement (kg/m ³)	404	359	314
Amount of silica fume (kg/m ³)	39	78	118
Amount of mixing water (kg/m ³)	155	153	151
Amount of aggregate (kg/m ³)		1684	

(6) Water permeability coefficient: the water permeability coefficient was calculated using the constant-head permeability test, which is based on the Pavement Test Manual established by the Japan Road Association. The permeability instrument measured the permeability coefficient of $\Phi 10 \times 20$ cm cylindrical specimens. The equation for water permeability coefficient K is expressed as follows:

$$K = \frac{QL}{AH\Delta t}, \quad (2)$$

where K is water permeability coefficient (cm/s), Q is flow volume (mL), L is specimen thickness (cm), A is the pervious surface area of specimens (cm²), H is water head height (cm), and $\Delta t = t_1 - t_0$ represents time duration of measurement (s).

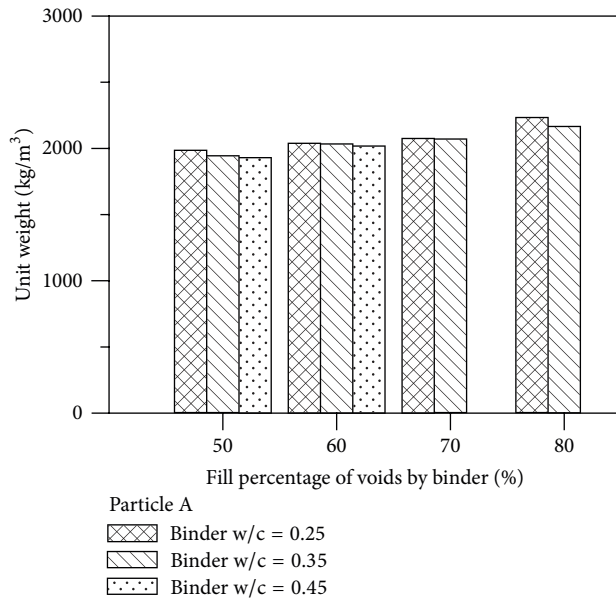
(7) Soundness test: aggregate soundness tests using magnesium sulfate were employed. Specimens were placed in an oven at $110 \pm 5^\circ\text{C}$ to dry until the constant weight W_1 was obtained. After cooled to room temperature, the specimen was fully immersed

(liquid rising 13 mm above the top of the specimen) for at least 16 h but less than 18 h in an oversaturated magnesium sulfate solution. After immersion, the specimen was taken out of the solution, washed using clean water, and then dried until the constant weight W_2 was obtained. The equation for calculating specimen weight-loss percentage is expressed as follows:

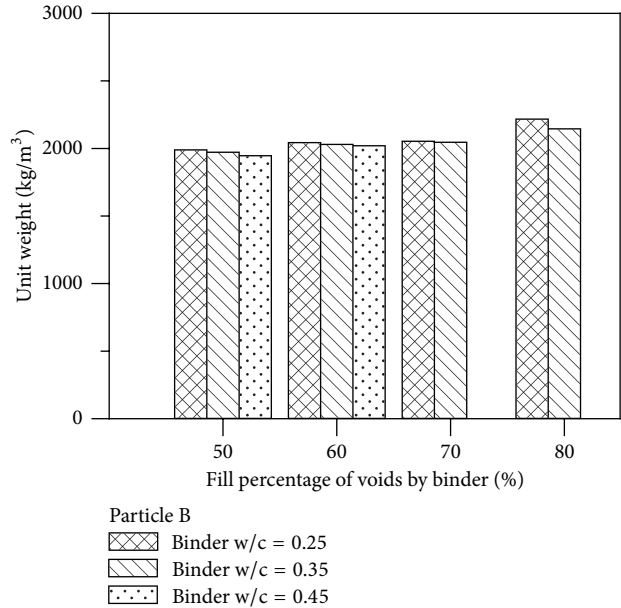
$$\text{Weight loss\%} = \frac{w_1 - w_2}{w_1} \times 100\%. \quad (3)$$

3. Test Results and Analysis

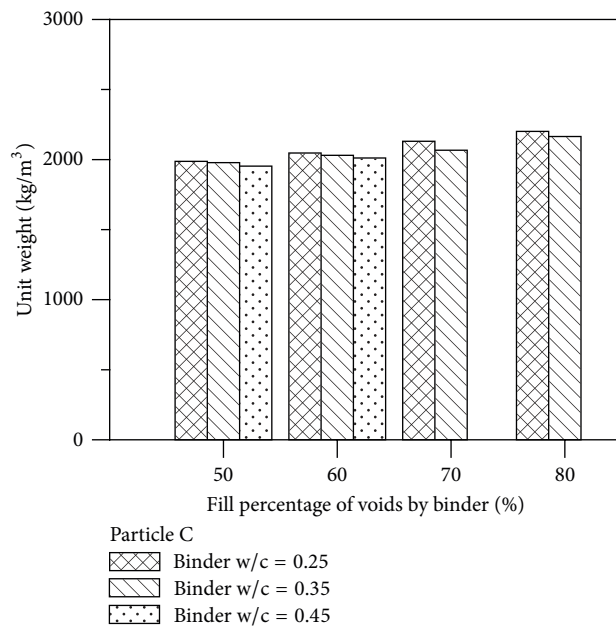
3.1. Unit Weight Test. Because pervious concrete has numerous voids, its unit weight is slightly lighter than those of common concrete. Regarding the mix proportion of cement pastes, changes in unit weight were observed based on a fixed aggregate size. The unit weight increased with the amount of binders but decreased with the increase of w/c ratios as shown in Figures 1(a)–1(d). Comparing the influence of different binders on unit weight, the unit weight of alkali-activated slag



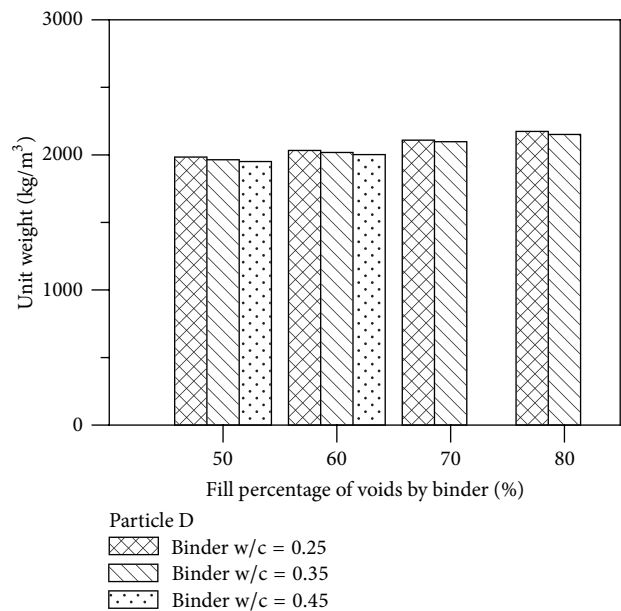
(a) Unit weights for particle A



(b) Unit weights for particle B



(c) Unit weights for particle C



(d) Unit weights for particle D

FIGURE 1: Unit weight of specimens comprising various particle sizes.

paste was greater than that of the control specimen ($w/c = 0.35$; aggregate code = B; volume percent of filled voids = 80%; binder = Portland cement paste), whereas the unit weight of the silica-fume cement paste was smaller than that of control specimen. In addition, the unit weights of specimens decreased as the amount of silica fume increased.

3.2. *Permeability Test.* Table 9 shows the water permeability coefficients of the specimens with different mix proportions. The specimen label is explained in the following. The first two

digits represent the w/c ratio (Portland cement paste) or L/Sg ratio (alkali-activated slag paste) or w/b ratio (silica-fume modified Portland cement paste). The alphabet followed the two digits means the aggregate code (A, B, C, and D). The next two digits mean the volume percent of filled voids. After these, the remaining characters mean the binder type (C stands for cement paste; A stands for the alkali-activated slag paste; S1 stands for silica-fume modified cement paste with the replacement percentage of 10%; S2 stands for silica-fume modified cement paste with the replacement percentage

TABLE 9: The water permeability coefficients for various mix proportions.

Label	K (cm/sec)	Label	K (cm/sec)	Label	K (cm/sec)	Label	K (cm/sec)
25A50C	0.1140	35A50C	0.0997	45A50C	0.0881	35B80A	0.0430
25A60C	0.1052	35A60C	0.0555	45A60C	0.0464	40B80A	0.0446
25A70C	0.0852	35A70C	0.0323	45A70C	NA	45B80A	0.0421
25A80C	0.0436	35A80C	0.0304	45A80C	NA	35B80S1	0.0454
25B50C	0.1259	35B50C	0.1192	45B50C	0.1176	35B80S2	0.0441
25B60C	0.1163	35B60C	0.1044	45B60C	0.0925	35B80S3	0.0444
25B70C	0.0864	35B70C	0.0585	45B70C	NA		
25B80C	0.0857	35B80C	0.0439	45B80C	NA		
25C50C	0.1423	35C50C	0.1273	45C50C	0.1210		
25C60C	0.1283	35C60C	0.1199	45C60C	0.1097		
25C70C	0.1185	35C70C	0.1118	45C70C	NA		
25C80C	0.1126	35C80C	0.1046	45C80C	NA		
25D50C	0.1440	35D50C	0.1355	45D50C	0.1331		
25D60C	0.1350	35D60C	0.1292	45D60C	0.1283		
25D70C	0.1284	35D70C	0.1131	45D70C	NA		
25D80C	0.1149	35D80C	0.1105	45D80C	NA		

of 20%; and S3 stands for silica-fume modified cement paste with the replacement percentage of 30%). For example, the label 25A50C means $w/c = 0.25$; aggregate code = A; volume percentage of filled voids = 50%; and binder = cement paste. In Table 9, there exist some specimens without permeability coefficients. The sagging phenomenon, which means that the cement paste does not stick to the aggregate and sags to the bottom of specimen due to its gravity, was observed. Once the sagging phenomenon happens, the paste will not be uniformly distributed around aggregates and will be accumulated in the bottom of specimen due to the gravity. Consequently, the water permeability will dramatically decrease since on the cross-section near the bottom the permeability is low. Based on this, we discard the records of these specimens. This result indicated that for $w/c = 0.45$, the viscosity enhancer should be introduced to avoid the possible sagging phenomenon.

The mix proportion of particle D exhibited an optimal water permeability coefficient K . Specifically, the specimen 25D50C with aggregate D, w/c ratio of 0.25, and 50% binder-filled voids had the greatest coefficient K of 0.1440 cm/sec. However, the specimen 35A80C with particle A, w/c ratio of 0.35, and 80% binder-filled voids had the smallest coefficient K of 0.0304. Figures 2(a)–2(d) show the graphs comparing the particle size variables based on data from Table 5.

For the different aggregate sizes, the water permeability coefficients decreased as binder w/c ratios increased. This is because at low w/c ratio, binders are highly viscous, enabling the binder to fully cover the aggregates. Intact and fully covered aggregate forms after specimens solidified, and point-to-point contact among the aggregates remains, yielding a void structure that forms a path for water permeation. By contrast, with high w/c ratios, because binders have great fluidity,

excess binders that cannot cover aggregates may block the path, thereby decreasing water permeability.

In addition, as the aggregate size increased, the water permeability of pervious concrete increased. For the conventional concrete, three phases (mortar, interface transition zone, and coarse aggregates) exist. The interface transition zone (ITZ) has the worst property among these three phases. It means that the mechanical strength of ITZ is weakest and the water permeability of ITZ is highest since the microstructure of ITZ contains more microcracks, voids, and so on. Therefore, it is well known that the ITZ dominates the behaviors of conventional concrete. If the volume fraction of ITZ increases, a lower compressive strength and a higher permeability coefficient then are expected. Consequently, as the aggregate size was smaller, the volume fraction of ITZ was expected to be larger. It then becomes very puzzling why the water permeability coefficient increased as the aggregate size increased. The reason is explained as the following. For the pervious concrete, four phases (paste, ITZ, coarse aggregates, and designed porosity) exist. Unlike the conventional concrete which adopts mortar to fill the space between aggregates, we only partially fill the space between aggregates by binder for pervious concrete. This concept intentionally makes some porosity inside concrete in order to allow water penetration. Among these four phases, the designed porosity has the worst properties. It has no mechanical strength at all and highest water permeation coefficient. Therefore, the volume fraction of the porosity dominates the behaviors of pervious concrete. When aggregate size is smaller, the initial porosity after packing is smaller. Consequently, when the volume percent of filled voids is the same, the remaining porosity for pervious concrete made with smaller size aggregates is smaller. Therefore, the water permeability coefficient becomes lower.

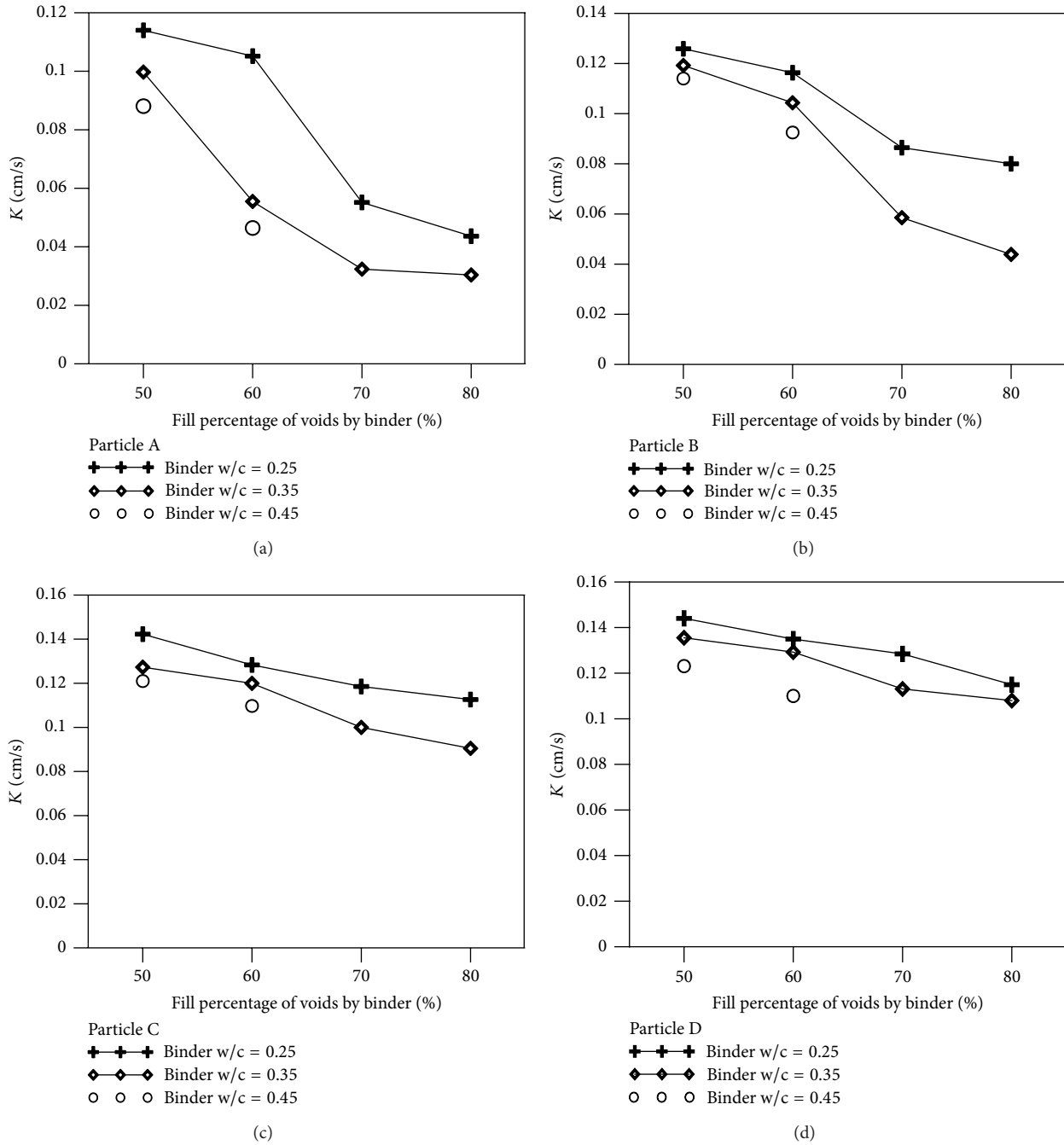


FIGURE 2: Water permeability coefficients of specimens comprising different aggregate sizes: (a) aggregate A; (b) aggregate B; (c) aggregate C; (d) aggregate D.

3.3. *Porosity Test.* The internal structure of pervious concrete is relatively less compact because using an insufficient amount of binders produces voids. Water permeability is primarily affected by connected porosity, which can be measured through experimental tests. As shown in Figures 3(a)–3(d), a fixed trend was observed when other parameters were compared based on the various mix proportions. The porosity in pervious concrete is typically inversely proportional to the amount of binders used. Basically, the trends for

the values of connected porosity were similar to those for water permeability coefficient. It is worth mentioning here that the connected porosity increased as the aggregate size increased. The reason has been mentioned in the previous subsection. Once again, the dominating factor is the total voids inside pervious concrete but not the ITZ.

3.4. *Compressive Strength Test.* Table 10 shows the results of 28-day compressive strength tests for various mix

TABLE 10: The results of the compressive strength test.

Label	Compressive strength (MPa)	Label	Compressive strength (MPa)	Label	Compressive strength (MPa)	Label	Compressive strength (MPa)
25A50C	17.49	35A50C	12.30	45A50C	10.62	35B80A	28.73
25A60C	19.01	35A60C	16.01	45A60C	14.83	40B80A	24.98
25A70C	22.61	35A70C	20.95	45A70C	—	45B80A	17.49
25A80C	25.67	35A80C	24.94	45A80C	—	35B80S1	21.98
25B50C	17.07	35B50C	11.87	45B50C	8.74	35B80S2	15.43
25B60C	17.45	35B60C	14.49	45B60C	11.62	35B80S3	7.68
25B70C	20.36	35B70C	18.81	45B70C	—		
25B80C	21.95	35B80C	20.86	45B80C	—		
25C50C	16.26	35C50C	11.24	45C50C	7.49		
25C60C	16.86	35C60C	13.18	45C60C	9.49		
25C70C	20.01	35C70C	15.51	45C70C	—		
25C80C	21.38	35C80C	16.49	45C80C	—		
25D50C	13.45	35D50C	10.34	45D50C	5.62		
25D60C	13.68	35D60C	11.87	45D60C	7.93		
25D70C	17.32	35D70C	14.86	45D70C	—		
25D80C	19.20	35D80C	15.61	45D80C	—		

proportions. The alkali-activated slag paste (35B80A) yielded the highest compressive strength of 28.73 MPa, and the cement paste (45D50C) yielded the smallest strength of 5.62 MPa. After compiling the data obtained from Table 6, Figures 4(a)–4(b) were plotted, displaying a comparison of the parameters based on fixed particle sizes.

It can be found that the compressive strength decreased as the aggregate size increased. Similar results were also found for other mechanical properties such as splitting tensile strength test and flexural strength test. As explained above, for the conventional concrete, the property of ITZ dominates the property of concrete since ITZ plays as the weakest part in concrete. However, for pervious concrete the weakest part now is designed void. As an increase in aggregate size, a higher volume of void then is expected under the same condition. In such a manner, the compressive strength decreased. Similar result has been observed in [4].

3.5. Splitting Tensile Strength Test. Figures 5(a)–5(d) show the results of the splitting tensile strength tests. The alkali-activated slag paste (35B80A) yielded the highest splitting tensile strength of 3.19 MPa, and the cement paste (45D50C) yielded the smallest strength of 1.21 MPa. According to Figures 5(a)–5(d), the binder with a w/c ratio of 0.25 had the greatest splitting tensile strength, whereas the binder with a w/c ratio of 0.45 had the weakest splitting tensile strength. Therefore, the w/c ratios of binders influence splitting tensile strength. In addition, using a substantial amount of binder strengthened splitting tensile strength; moreover, the specimens with 80% binder-filled voids yielded the highest splitting tensile strength.

Furthermore, the splitting tensile strength decreased as the aggregate size increased. The reason has been given in Section 3.4.

3.6. Flexural Strength Test. Figures 6(a)–6(d) present the results of flexural strength tests. Specimen 35B80C exhibited the highest flexural strength at 4.76 MPa, and specimen 45D50C produced the weakest flexural strength of 2.18 MPa. The figures show that flexural strength is directly proportional to the amount of binder used. In addition, the flexural strength of binders with low w/c ratio is superior to that of high-w/c ratio binders. Thus, the mix proportions with highest flexural strengths had a w/c ratio of 0.25 and 80% binder-filled voids, and those with weakest flexural strengths had a w/c ratio of 0.45 and 50% binder-filled voids. In addition, the flexural strength decreased as the aggregate size increased. The reason has been given in Section 3.4.

3.7. Soundness Test. Soundness tests can simulate the resistance of pervious concrete specimens to sulfate attacks. In this study, 12-day old specimens were immersed in an oversaturated magnesium sulfate solution and then washed before subjecting to a round robin test. Subsequently, the weight loss percentages of the specimens were measured to infer the resistance to sulfate attacks; the results are shown in Figures 7(a)–7(d). Specimen 35B80S3, which comprised cement paste and 30% silica fume, exhibited a great weight loss percentage of 3%, and the alkali-activated slag specimen 35B80A yielded a minimal weight loss percentage of 0.57%.

For all the aggregate sizes, maximum weight loss occurred with 50% binder-filled void, and weight loss increased as the amount of binder used increased. Regarding binder characteristics, weight loss was minimal at a w/c ratio of 0.25 and increased with increasing w/c ratio. This was caused by the overall compactness of the specimen. When a small amount of binder was used, increased number of voids in the specimen became available for sulfate attacks and reactions between cements and aggregates. As with w/c ratios, after

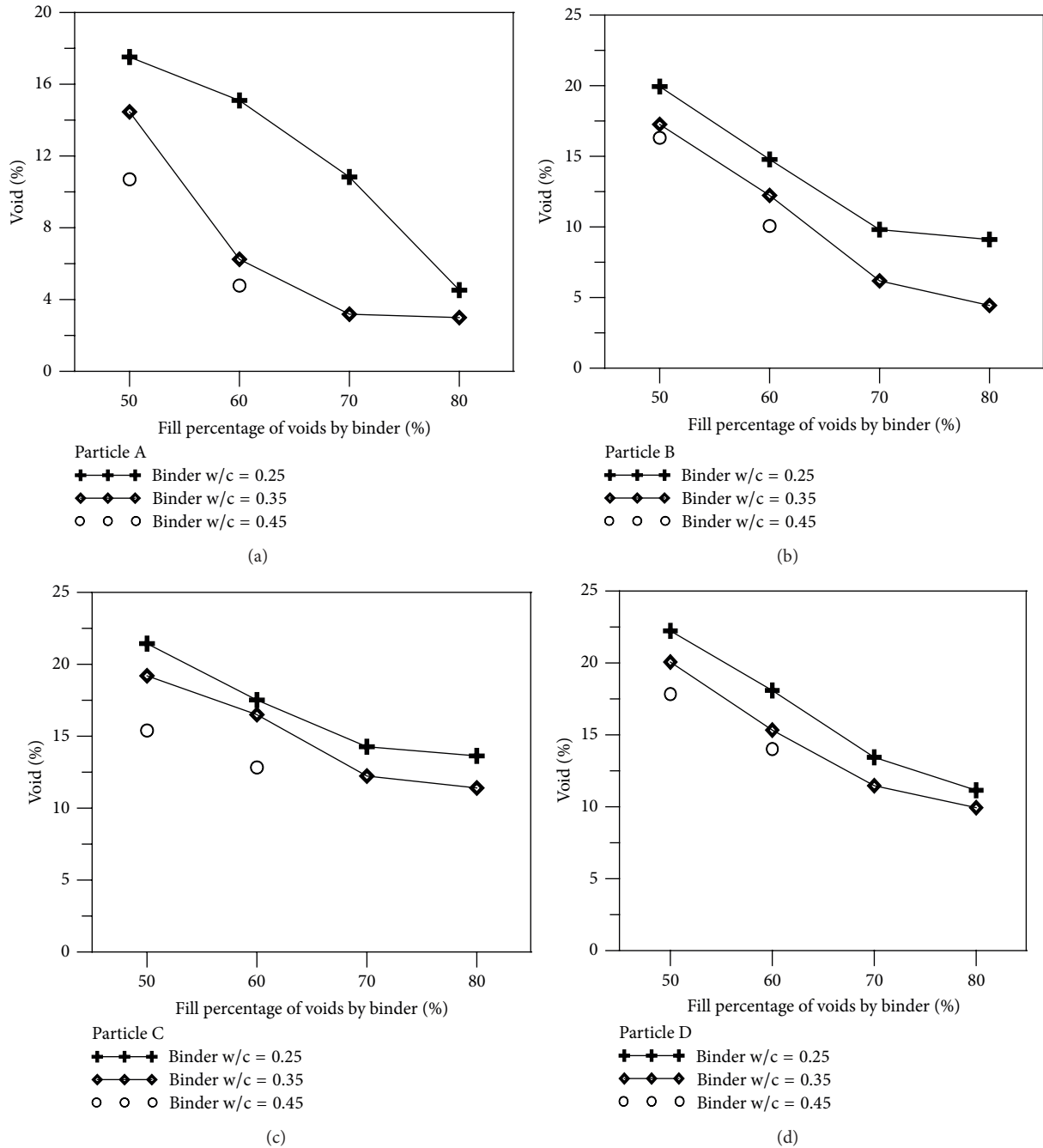


FIGURE 3: Connected porosity of specimens comprising different aggregate sizes: (a) aggregate A; (b) aggregate B; (c) aggregate C; (d) aggregate D.

high-w/c binders solidified, the amount of small voids that remained after water loss or hydration was greater than that of low-w/c binders. This increase in void amount created paths for sulfate attacks, thus decreasing the sulfate-attack resistance of high-w/c pervious concrete containing low amount of binders.

3.8. *The Cross-Comparison of Water Permeability Coefficients.* The specimens comprising four various aggregate sizes were subjected to permeability and connected porosity tests. The

test results were then used to plot an X-Y distribution graph to which trend lines were added as shown in Figure 8. For the four types of particle sizes, water permeability coefficient K was directly proportional to connected porosity. Next, an analysis on water permeability coefficients based on a fixed porosity was conducted, which revealed a directly proportional relationship between permeability and aggregate size. From a physics point of view, a fixed porosity should only correspond with a specific permeability coefficient, which contradicts the results of this study, in which

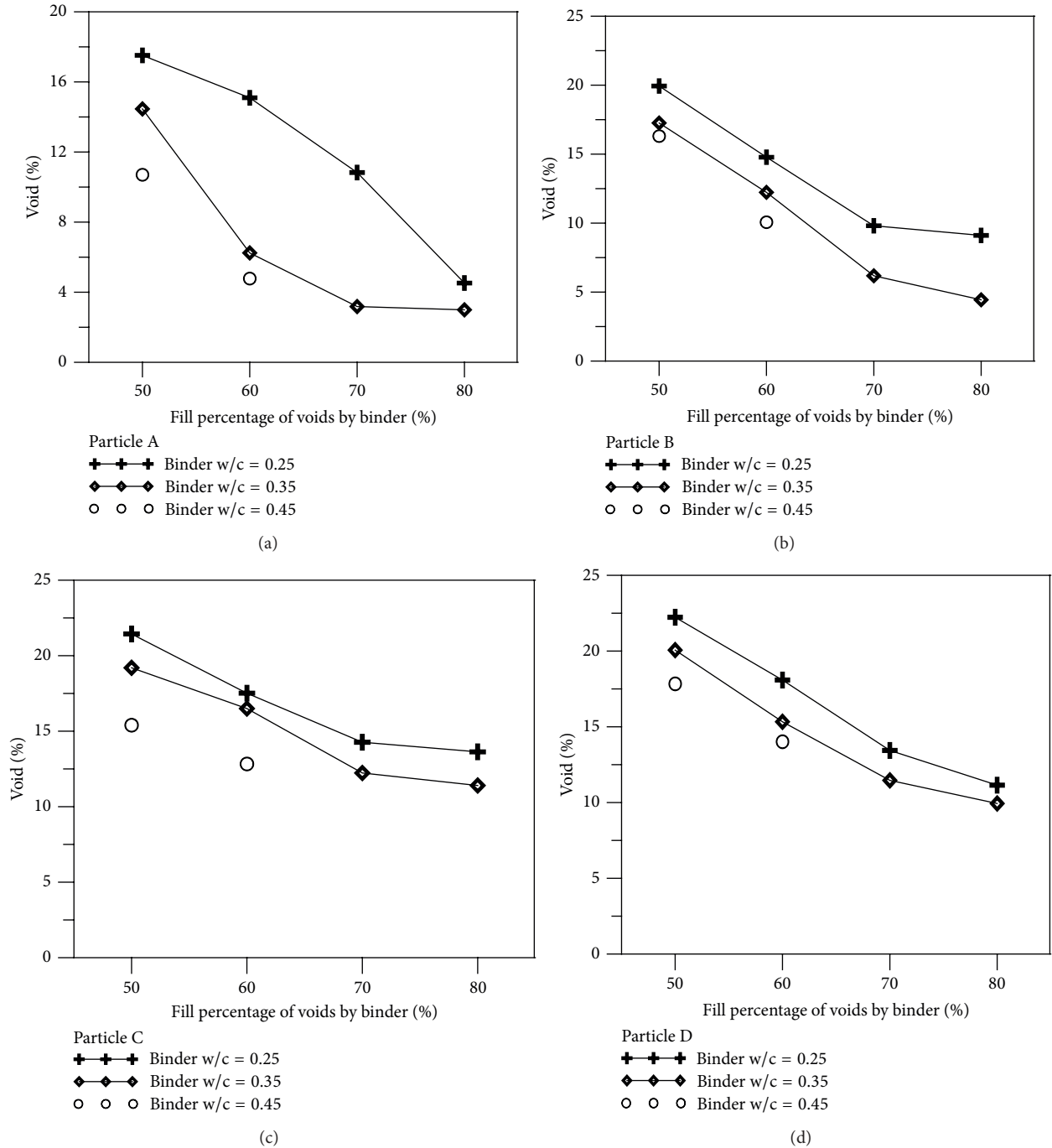


FIGURE 4: Compressive strengths of specimens comprising different aggregate sizes: (a) aggregate A; (b) aggregate B; (c) aggregate C; (d) aggregate D.

multiple coefficients were observed. The reason might be that although the specimens had the same porosity under certain conditions, the void sizes differ depending on the aggregate size. Under identical connected porosity, the specimens with small aggregates had small sectional areas in the connected voids, generating meandering paths for water permeation. Conversely, specimens consisting of large particles had large sectional areas and, thus, had straight paths. Therefore, the

velocity (V) of the water infiltrated in the specimen differed, yielding variation in the flow volume (Q) of the water flowing out of the specimen.

To determine the correlation between compressive strength and water permeability, these two variables were measured for each particle size and then an X-Y distribution graph was plotted. Trend lines were added to the graph to illustrate the correlation. Figure 9 shows that mechanical

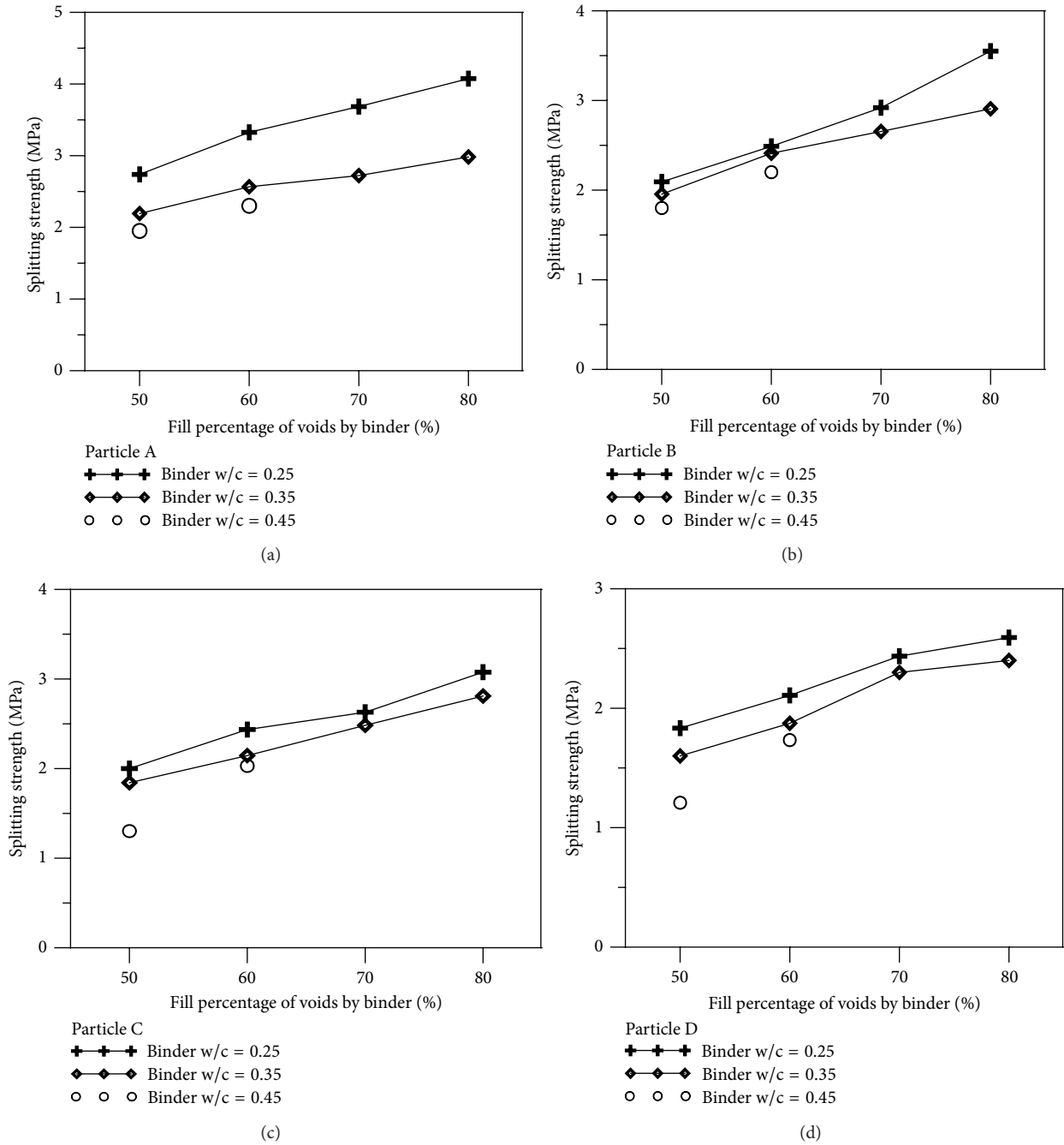


FIGURE 5: Splitting tensile strengths of specimens comprising different aggregate sizes: (a) aggregate A; (b) aggregate B; (c) aggregate C; (d) aggregate D.

strength decreases as water permeability coefficient increases, and such relationship is applicable for all aggregate sizes.

4. Conclusions

- (1) Water permeability coefficient and connected porosity decreased with the increase of binder amounts but increased with increasing aggregate size.
- (2) Pervious concrete with binders of low w/c ratio is highly viscous, which facilitated covering the aggregates. This enabled sufficient binding between particles and effectively reduced excess binders from blocking water permeation paths, thereby influencing water permeability.
- (3) Under identical connected porosity, the specimens with small particles had small sectional areas in the connected voids, producing meandering paths

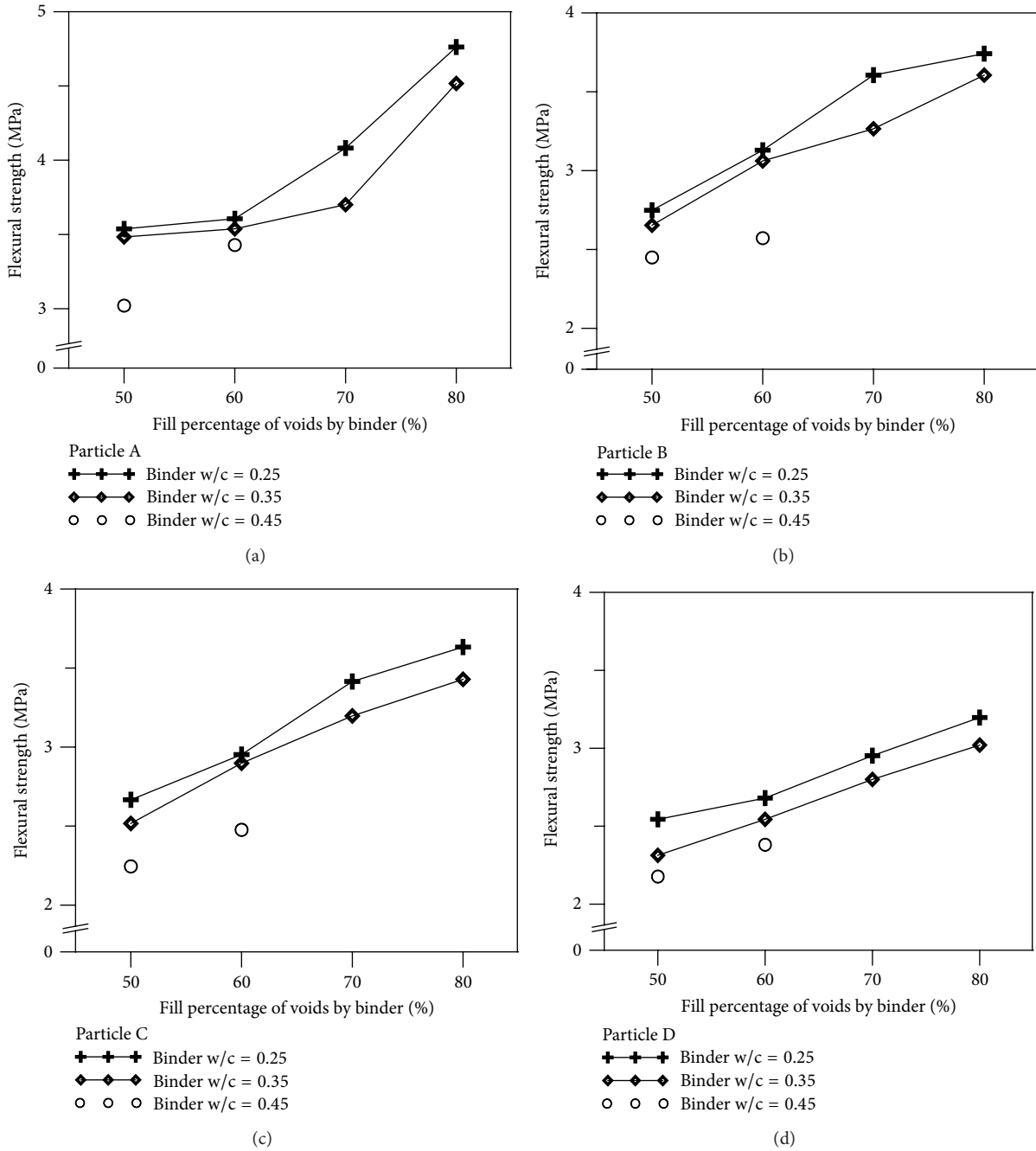


FIGURE 6: Flexural strengths of specimens comprising different particle sizes: (a) aggregate A; (b) aggregate B; (c) aggregate C; (d) aggregate D.

for water permeation. Specimens consisting of large particles had large sectional areas and straight paths. Thus, the velocity of the water infiltrated in the specimen differed, yielding variation in the flow volume of the water flowing out of the specimen.

- (4) Mechanical strength decreased with increasing water permeability. Although using substantial amount of binders can enhance mechanical strength, permeability might decrease because the volume percent of binder-filled voids increases. The amount of

binder used was directly proportional to mechanical strength, and increased aggregate size decreased mechanical strength.

- (5) Alkali-activated slag paste with L/Sg = 0.35 and 0.4 had greater mechanical strength than the cement-paste control did. Consequently, using appropriate amount of alkali-activated slag as a binder can effectively enhance the mechanical strength of pervious concrete.

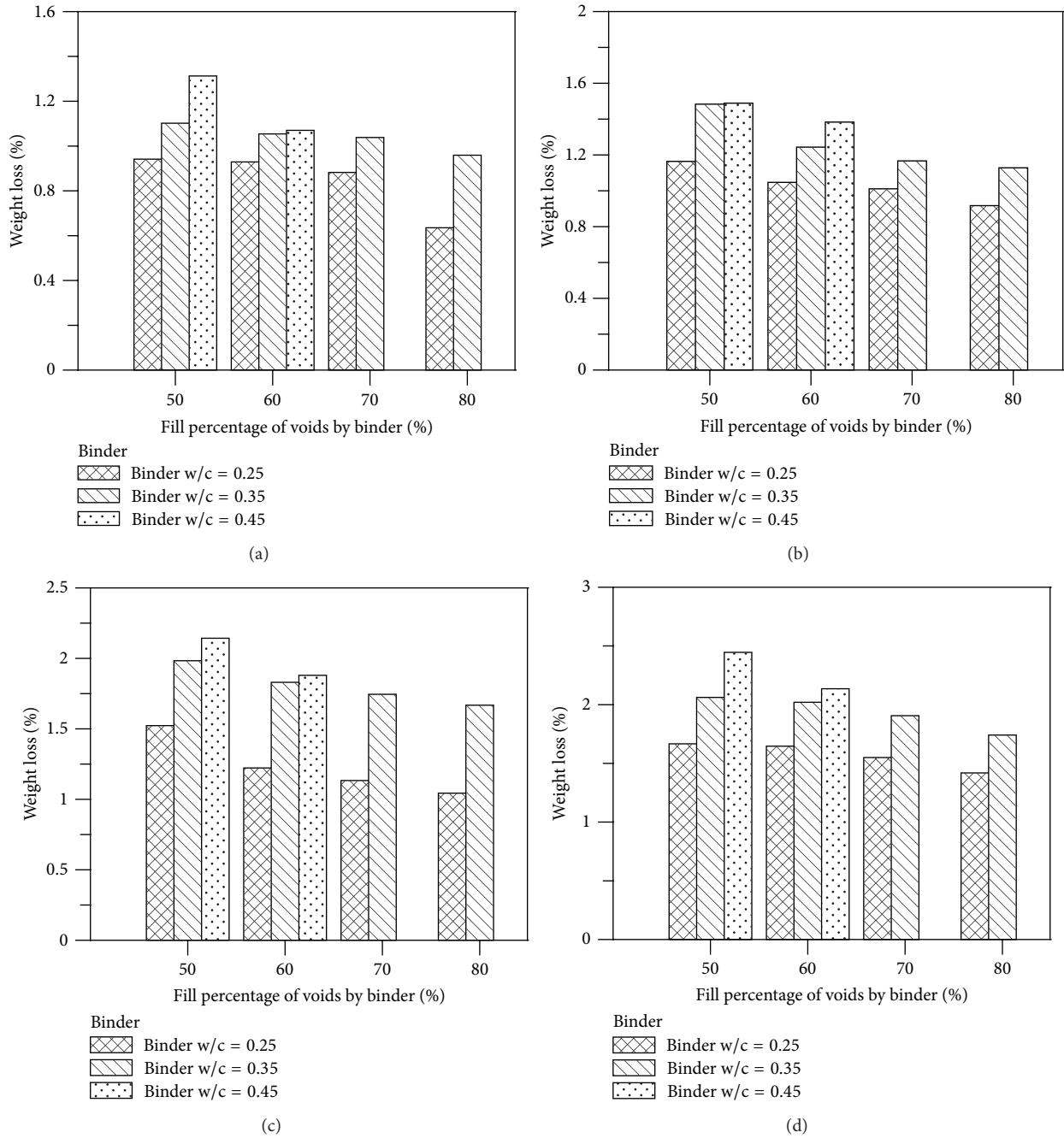


FIGURE 7: The weight loss percentages of specimens comprising different aggregate sizes that were immersed in a sulfate solution: (a) aggregate A; (b) aggregate B; (c) aggregate C; (d) aggregate D.

(6) The mechanical strengths of the specimens with binders containing 10% silica fume were superior to those of the control specimen. This result was not observed for specimens with binders containing 20% or 30% silica fume, possibly because excessive amount of silica fume was added. Therefore, adding a suitable amount of silica fume to binders can enhance the overall mechanical strength of pervious concrete.

(7) The weight-loss percentage of pervious concrete experiencing sulfate attacks increased as the w/c ratio of cement pastes increased. Weight loss percentage was directly proportional to aggregate size, because the stacking of large aggregates generated large voids, which created paths for sulfate to infiltrate specimens and induce reactions. Specimens composed of alkali-activated slag binder exhibited a superior resistance to

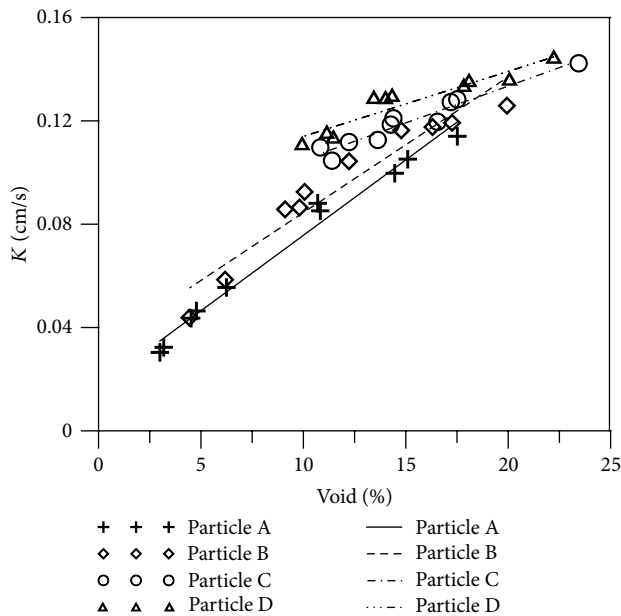


FIGURE 8: The relationship between porosity and water permeability coefficient.

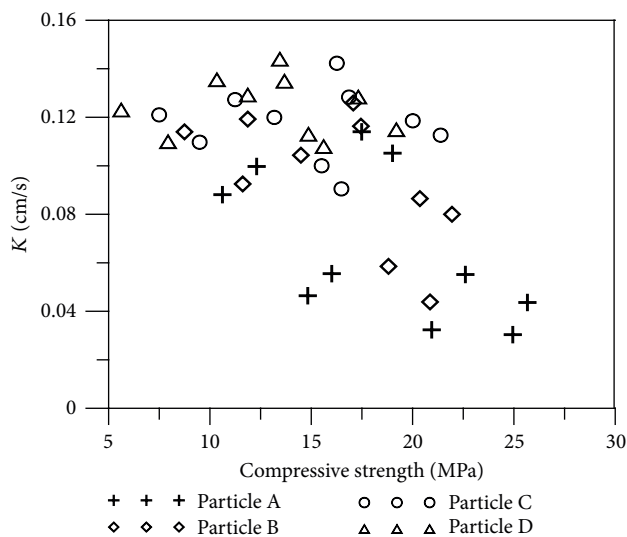


FIGURE 9: Correlation between compressive strength and water permeability coefficient.

sulfate attacks than those consisting of cement pastes did.

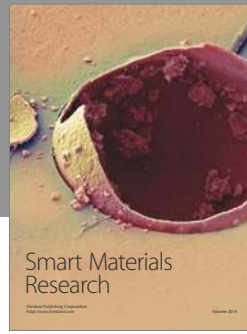
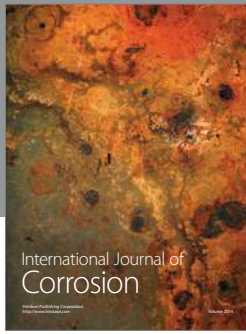
Conflict of Interests

The authors declare that there is no conflict of interests regarding the publication of this paper.

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