

1 Total recycling of low-quality urban-fringe construction and
2 demolition waste towards the development of sustainable
3 non-cement pervious concrete: The proof of concept

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8 **Abstract**

9 This work provided a proof-of-concept study of recycling low-quality urban-fringe con-
10 struction and demolition waste (CDW) for the development of sustainable non-cement
11 pervious concrete. CDW in-situ collected from a local construction site of urban-fringe
12 in Hangzhou, China, was elaborately analyzed for quality assessment. Ground brick
13 powder was activated by alkali activators (NaOH and Na₂SiO₄) to fabricate the binding
14 material, and the optimal mixing ratio was investigated. Macro CDW solids and alkali-
15 activated recycled brick powder (AARBP) were used as the aggregate and binding
16 material, respectively, to achieve the CDW's total recycling. The effects of binder-
17 to-aggregate ratio on the engineering performances and multi-scale structures of the
18 non-cement recycled CDW pervious concrete were explored. Results showed that the
19 AARBP paste had the strength up to 50 MPa at the Al+Si/Na (AS/N) ratio of 6.3.
20 The non-cement recycled CDW previous concrete showed relatively low compressive
21 strength but high water permeability. Microstructural mechanisms in the aspects of
22 pores, skeletons, and matrix-aggregate interactions were explored. The findings of this

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23 work provide a promising route towards solving the large-scale CDW in China.

24 *Keywords:* Recycling; Pervious concrete; Strength; Microstructure.

25 **1. Introduction**

26 The continual urbanization currently occurring in China triggers the large-scale
27 constructions of infrastructures and buildings, which, however, simultaneously produce
28 massive construction and demolition waste (CDW) that increases the landfill, and pol-
29 lutes the air and water in local cities (Li et al., 2020c). The rapid annual increment of
30 CDW and the low recycling rate (around 5%) in China jointly put high economic and
31 environmental stresses (Duan et al., 2019). It was estimated that the annual accumula-
32 tion of CDW was raised up to over 2.4 billion tones (Yazdani et al., 2021), and around
33 three-quarter of Chinese cities suffered excessive CDW (Ma et al., 2020). While policies
34 issued by Chinese government attempt to encourage the reuse and recycling of CDW,
35 the recycling efficiency of CDW faces difficulties due to different reasons, such as, the
36 huge variances in CDW source and quality, and the lack of tracing system of resources,
37 products and wastes (Ma et al., 2020). Challenges rise for the high efficient uses of
38 low-quality CDW that doesn't meet the requirements of engineering applications.

39 At present, a common way of CDW recycling is to reuse the inert phases of CDW
40 with relatively high mechanical properties and volume stability (e.g., concrete, brick,
41 ceramic and mortar) after complete or partial processes of screening, crushing, sieving,
42 separating and washing. Generally, those inert wastes can be directly used as secondary
43 construction material to totally or partially replace natural aggregate and/or filler in the

44 engineering scenarios of pavement subgrade (Zhang et al., 2021), and recycled aggregate
45 for concrete construction (Robalo et al., 2021; Olofinnade and Ogara, 2021). Xiao and
46 coworkers have systematically investigated the engineering properties of concrete with
47 CDW aggregate and filler (Duan et al., 2020a,b; Sun et al., 2021). Moreover, CDW may
48 act as the source materials for geopolymer fabrication (Ulugol et al., 2021a,b), composite
49 manufacturing (Sormunen and Karki, 2019; Clark et al., 2020), and functional materials
50 development for control of environmental noise (Amarilla et al., 2021).

51 Compared with natural aggregate, recycled CDW aggregate often possesses lower
52 crushing index, but higher water absorption due to the existence of porous phases like re-
53 cycled mortar and brick. Therefore, the direct substitution of natural aggregate by recy-
54 cled CDW aggregate for concrete manufacture generally causes the substantial decrease
55 of workability, strength and permeability resistance (Duan et al., 2020a; Olofinnade and
56 Ogara, 2021; Meng et al., 2021). For example, Liu et al. (2020) evidenced the negatively
57 effects of recycled brick particles with low material strength, rough particle surfaces,
58 and high porous microstructure on the workability and strength of concrete and mor-
59 tar. Strengthening techniques were therefore developed for the proper uses of CDW in
60 construction materials manufactures. Xiao et al. (2018) reported that the milling of
61 CDW to powder (named mechanical activation) can retain the mechanical properties
62 of concrete with CDW powder ratio up to 45%. Meng et al. (2021) suggested that,
63 compared with the concrete blended with untreated CDW, the activation of CDW with
64 nano particles can substantially increase the mechanical properties of concrete with the
65 same CDW replacement ratio.

66 Alternatively, it would be a proper solution to avoid the low engineering properties
67 of CDW for developing construction materials with relatively low strength and perme-
68 ability thresholds like pervious concrete. The uses of pervious materials in pavements,
69 roads, parking lots and riverbanks raised by the development of sponge cities (Shen et
70 al., 2020) would bring the additional benefits to mitigate the urban heat-island effect
71 (Chen et al., 2019; Liu et al., 2020; Tan et al., 2021) and waterlog disaster (Cai et al.,
72 2018; Zhou et al., 2021). Great efforts therefore have been made to develop pervious
73 concrete with high content of CDW as the aggregate (Ibrahim et al., 2020; Debnath
74 and Sarkar, 2020; Lu et al., 2019; Vieira et al., 2020). However, ordinary Portland ce-
75 ment (OPC) is generally used as the binding material in most CDW pervious concretes,
76 which, is blamed for the high energy consumption and CO₂ emissions of cement produc-
77 tion (IEA, 2020). Therefore, it provides strong incentives to develop new eco-friendly
78 cementitious material to replace OPC for CDW pervious concrete manufacture. Alkali
79 activation may be a preferable technique to develop low-carbon binder, as part of the
80 silica and alumina in CDW inerts like sintered clay brick, ceramics and glasses show
81 active potentials (Li et al., 2020a,b; Collivignarelli et al., 2021; Ulugol et al., 2021a,b).
82 For instance, alkali-activated CDW was used to construct road sudgrade (Bassani et al.,
83 2019a,b; Tefa et al., 2021) and masonry unit (Zhang et al., 2021). Inspired by the high
84 sustainability of alkali-activated CDW binder, great significance will rise for pervious
85 concrete manufactured by both CDW binder and aggregate, which may be termed as
86 non-cement pervious concrete with total CDW recycling.

87 The main goal of this work is developing a feasible technique for the total recycling of

88 low-quality CDW for non-cement pervious concrete manufacture. To this end, alkali-
89 activated recycled brick powder (AARBP) alternative to cement was used as binder
90 to replace OPC on one hand, a low-quality CDW was recycled as aggregate on the
91 other hand. The low-quality CDW used in this work was in-situ collected from a local
92 urban-fringe of Hangzhou, China, and their physical properties were comprehensively
93 characterized. Specific experimental schemes were designed to 1) optimize the mix
94 proportions of AARBP paste, and 2) fabricate non-cement CDW pervious concrete.
95 Structures of pores, skeletons and matrix-aggregate interfacial transition zones (ITZs)
96 were characterized by X-ray computed tomography (XCT) and scanning electron mi-
97 croscopy (SEM). Profound discussions were performed to explore the microstructural
98 mechanisms of engineering performances of the non-cement CDW pervious concrete.
99 The technique developed in this work may broaden the ways to relax the environmental
100 stresses raised by the massive pileup of CDW and CO₂ emissions by cement uses.

101 **2. Characterization of low-quality CDW**

102 The ongoing constructions and demolitions in local urban-fringe of Hangzhou, China,
103 stimulated by the 2022 Asian Games simultaneously raised new buildings and piled up
104 massive CDW. As an example, Fig. 1a shows the mountainous pile of CDW in a con-
105 struction site, Gongshu district of Hangzhou, Zhejiang. A recycling factory (Hangzhou
106 Qianjiangxincheng Municipal Garden Construction Co., Ltd) has been established near
107 the construction site for coarse materials recycling. CDW was crushed, sieved and par-
108 tially recycled, where the inerts with particle size between 10 and 45 mm were recycled

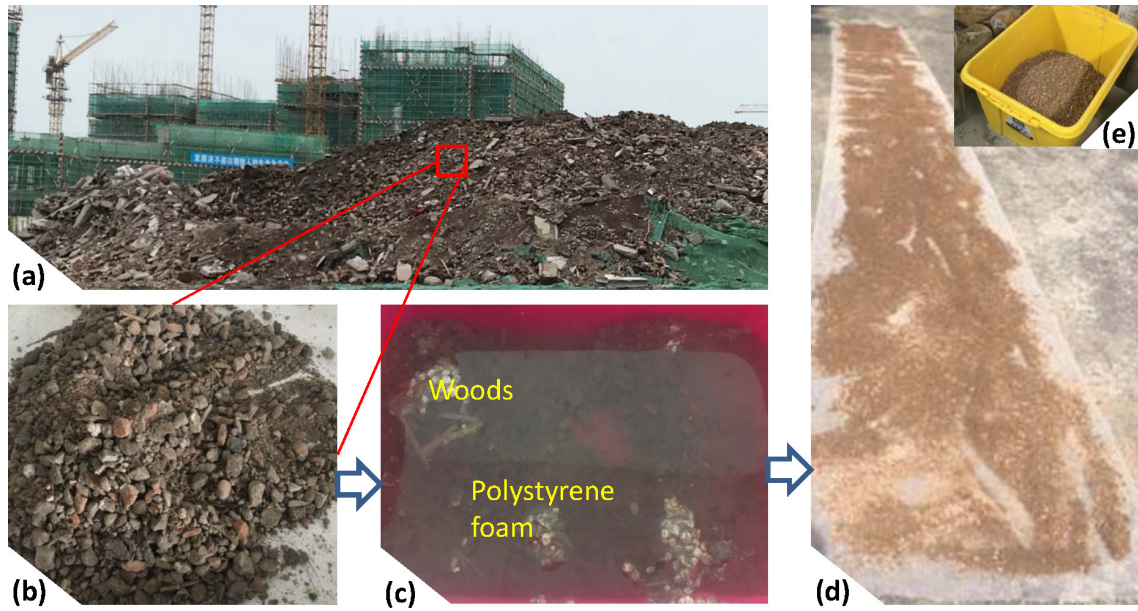


Figure 1: (a) Pileup of CDW in a local area of Hangzhou (Zhejiang province, China. Photographed by the corresponding author at 16, July, 2020), (b) low-quality CDW after crushing and sieving, (c) washing of the CDW to remove the light woods and plastics, (d) drying of the CDW in the sun, (e) storage of the CDW solids in a box.

109 as aggregate after washing, but the rest can not be directly recycled due to complex
 110 compounds and poor properties. Therefore, the untreated CDW particles with size
 111 below 10 mm may be classified as a type of low-quality CDW (Fig. 1b). Large scale of
 112 low-quality CDW that pileups everyday raises stresses on land circulation of Hangzhou.

113 The obtained low-quality CDW, a mixture of different compounds, was first im-
 114 mersed in water to distinguish between light materials (e.g., wood and foam plastics)
 115 and sediments (Fig. 1c). The light materials were removed, while the rest sediments
 116 were retained for further processes. After an air-drying process by sunlight exposure
 117 (Fig. 1d), the sediments were stored in a box for further tests (Fig. 1e).

118 The dried low-quality CDW was then sieved in a sieving system to analyze particle
119 size distribution (PSD) (Fig. 2a). Particles in the size ranges of below 1.18 mm, 1.18-
120 2.36 mm, 2.36-4.75 mm, 4.75-9.5 mm and over 9.5 mm occupied 22.4%, 18.%, 21.3%,
121 32.3% and 6.6% of the total low-quality CDW by mass (Fig. 2b). Typical morphology of
122 those particles at different size levels is shown in Fig. 2c. It is noteworthy that the CDW
123 with size below 1.18 mm can be sorted as dusts and soils, which thus can not directly
124 recycled as the fillers for construction materials. Therefore, the waste particles with
125 size over 1.18 mm were retained and recycled as the aggregate for further engineering
126 applications.

127 Component analysis was further performed on the cleaned and sieved CDW sample
128 (particle size over 1.18 mm). Fig. 3a-i demonstrates typical pictures of the main phases
129 classified from 1 kg low-quality CDW inert. Crushed concrete pieces (aggregates covered
130 with cement mortar), clean stones, and bricks took the most mass fractions (i.e., 59%wt,
131 26%wt, and 10%wt, respectively), while the remainders, i.e., ceramic, glass, wood, slag,
132 gypsum and metal, only occupied the rest 5%wt (Fig. 3j). The ceramic, glass, wood and
133 gypsum may come from decoration materials in the urban-fringe building demolitions.
134 Nails in different specifications were observed (Fig. 3i). Component complexity of the
135 low-quality CDW increased recycling difficulties.

136 The sieved CDW aggregates then experienced the tests of engineering properties.
137 The obtained bulk density, crushing index and water absorption were 1350 kg/m³,
138 15.9% and 8.5%. According to a Chinese standard (GB/T 25177-2010), the CDW
139 inert cannot be directly recycled as the aggregate for construction concrete production,

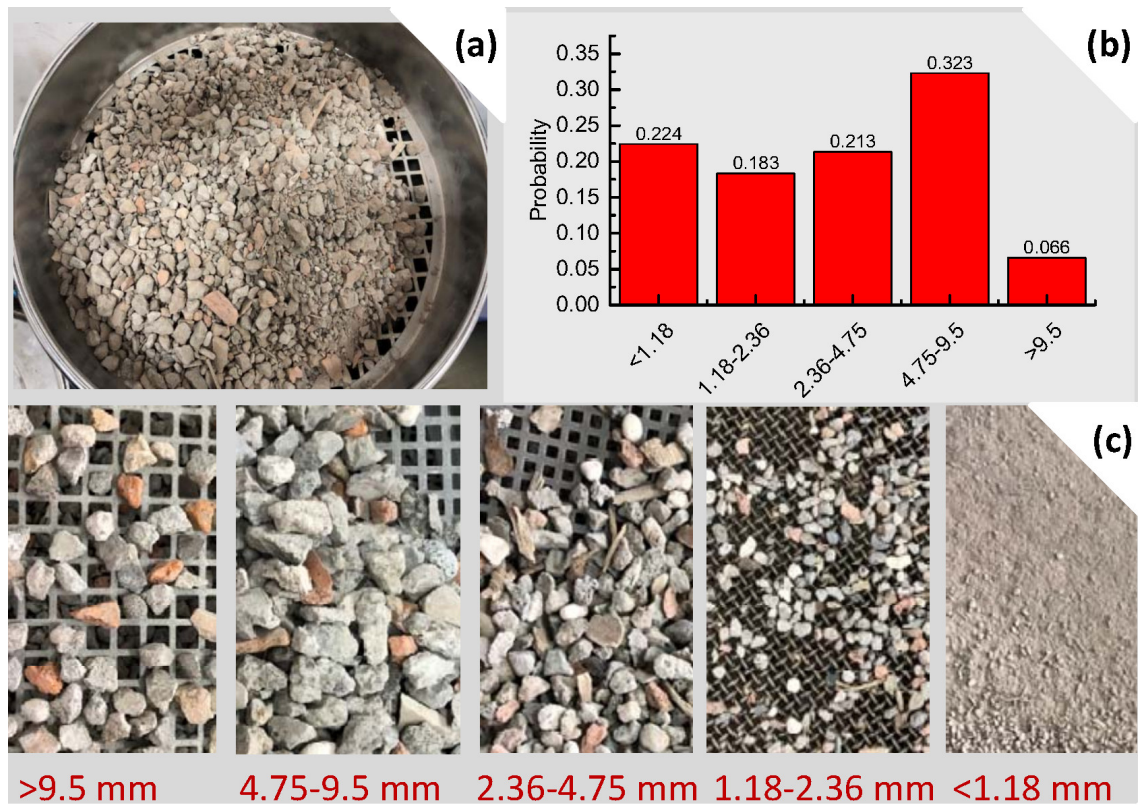


Figure 2: (a) Sieving of the cleaned and dried CDW, (b) particle size distribution of the CDW, (c) pictures of the sieving residue of different size ranges.

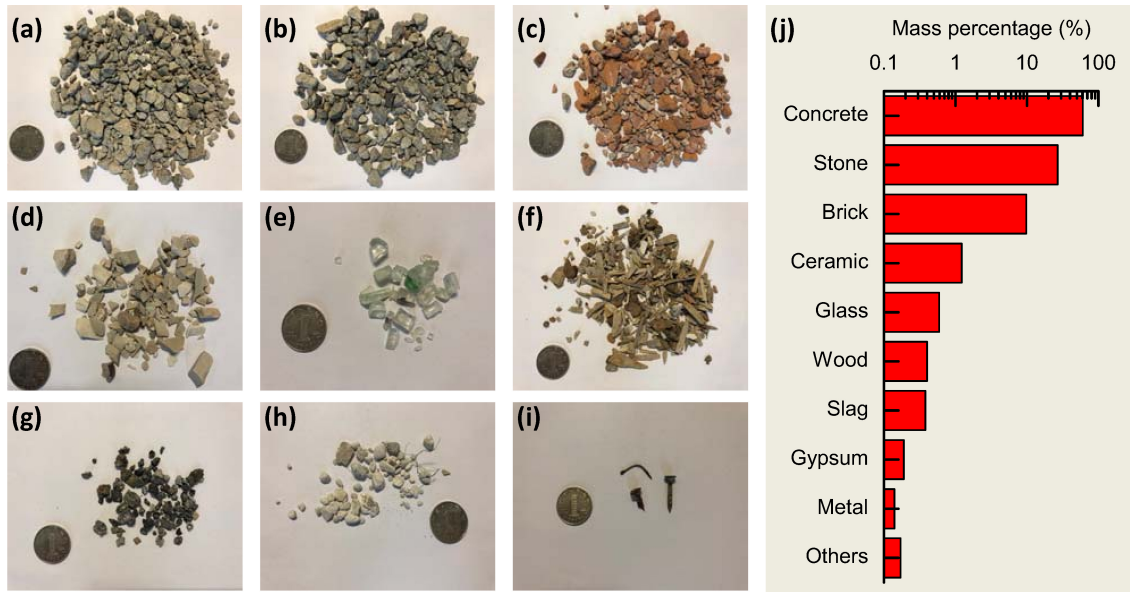


Figure 3: Classification of the CDW sample with size over 1.18 mm: (a) recycled concrete, (b) stone, (c) brick, (d) ceramics, (e) glass, (f) wood, (g) slag, (h) gypsum, (i) metals, and (j) their mass percentages.

140 because the water absorption was higher than the threshold index ($<8\%$). Therefore,
 141 new techniques to use those low-quality CDW should be developed.

142 3. Experiments and methods

143 3.1. Experimental design

144 In this work, in addition to the ordinary recycling of CDW as aggregate, the alkali
 145 activation technique was applied to activate brick powder as the binding material al-
 146 ternative to cement for achieving the maximum sustainability (Bassani et al., 2019a,b;
 147 Tefa et al., 2021; Collivignarelli et al., 2021; Ulugol et al., 2021a,b). The sketch of ex-
 148 perimental design is displayed in Fig. 4. The experimental work consisted of two main
 149 parts.

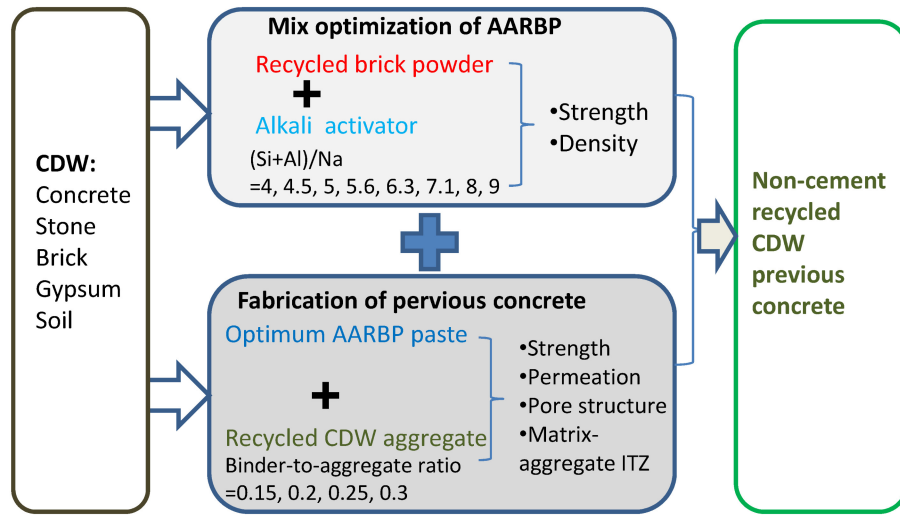


Figure 4: Experimental scheme for the development of non-cement recycled CDW pervious concrete.

- 150 • Optimization of AARBP mixes. Finding out the appropriate mix proportions

151 between the brick powder and alkali activators was the premise to use AARBP as

152 the binding material. Different molar ratios of $(Al_2O_3 + SiO_2)/Na_2O$ (abbreviated

153 as AS/N ratios) were designed according to (Reig et al., 2013; Robayo et al., 2016;

154 Tuyan et al., 2018). Mix optimization was performed based on strength data

155 (Fig. 4).
- 156 • Fabrication of non-cement CDW pervious concrete. The optimum AARBP mix

157 was adopted to prepare the paste that acts as the binding material to bond to-

158 gether the low-quality CDW aggregates. Because the content of binding material

159 can greatly impact the properties of pervious concrete, four different binder-to-

160 aggregate (B/A) ratios were designed (B/A=0.15, 0.2, 0.25, and 0.3 by mass).

161 The properties of compressive strength and water permeability coefficient were

162 tested, and the material features of pore structure, skeleton and matrix-aggregate

163 ITZ were characterized with multi-scale tests (Fig. 4).

164 3.2. Sample preparation

165 3.2.1. Preparation of AARBP

166 Waste sintered clay bricks were collected from the same demolitions, and crushed
167 into macro pieces by a jaw crushing machine (Fig. 5a). The macro brick pieces were
168 then ground to powder by a ball milling machine for 2 h (Fig. 5b). PSD of the brick
169 powder was tested via a laser particle size analyzer (Beckman Coulter LS 230). The
170 most probably particle size at the peak of differential PSD curve was $60.3 \mu\text{m}$, and
171 D50 was $30.2 \mu\text{m}$ (Fig. 5c). The brick powder showed the similar fineness to a type of
172 recycled brick dust (Li et al., 2020d), but coarser than some highly milled brick powders
173 (Tang et al., 2020) and Portland cement (Li et al., 2020b).

174 X-ray fluorescence (XRF) test was performed to measure the main oxides in the
175 recycled brick powder. Silica, alumina and ferric oxide occupied the ulmost mass of the
176 brick sample (over 92%wt), while potassium oxide, magnesium oxide and calcium oxide
177 occupied 6.4%wt and other oxides were all less than 0.5% (Fig. 5d). Similar chemical
178 component distribution was reported for other recycled bricks and ceramics (Tuyan et
179 al., 2018; Liu et al., 2020; Li et al., 2020a).

180 The chemical mechanisms of using brick powder as the binding material are that
181 part of the silica and alumina in the sintered clay bricks would show potential chemical
182 activities under highly alkaline environments (Tuyan et al., 2018; Bassani et al., 2019a,b;
183 Tefa et al., 2021; Ulugol et al., 2021a,b). Here sodium silicate solution (Na_2SiO_4 with

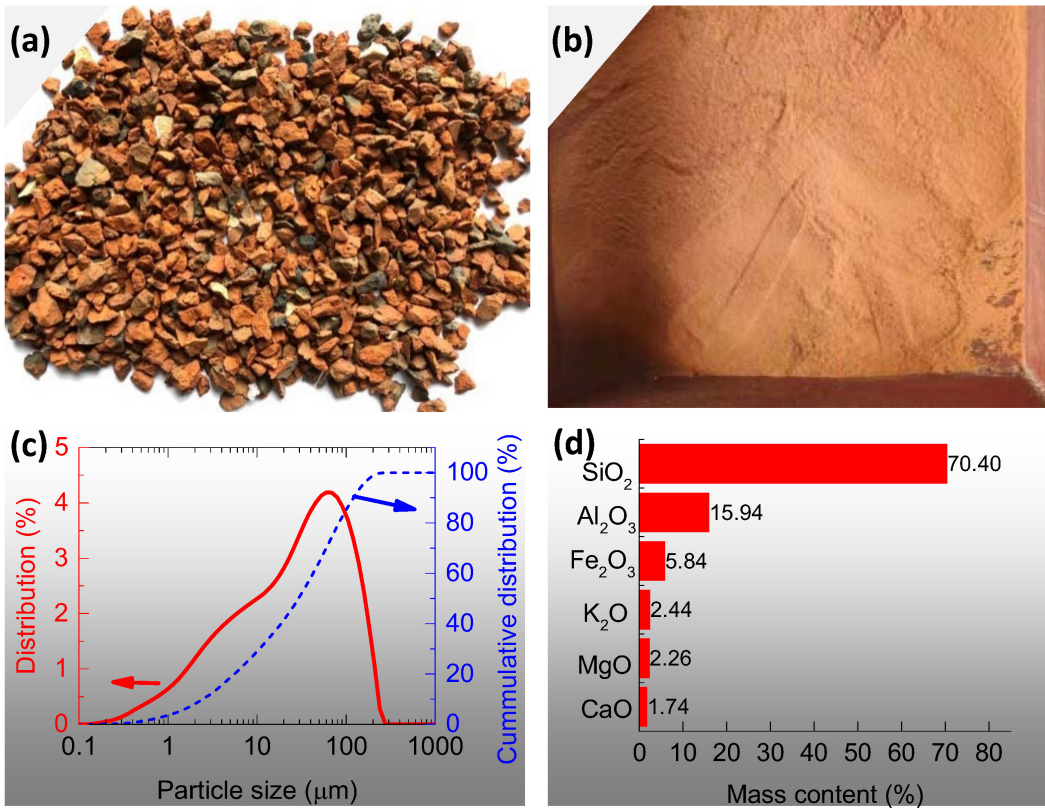


Figure 5: (a) Crushed brick particles and (b) brick powder after ball milling, (c) PSD of the brick powder, and (d) the main oxides in the brick powder tested by XRF.

184 $\text{SiO}_2 = 27.35\%$, $\text{Na}_2\text{O} = 8.42\%$, and $\text{H}_2\text{O} = 64.23\%$) and sodium hydroxide ($\text{Na}(\text{OH})_2$
185 with $\text{Na}_2\text{O} = 77.4\%$, and $\text{H}_2\text{O} = 22.5\%$, impurity =0.1%) were used as the activators.
186 Both the sodium silicate solution and sodium hydroxide were purchased from Sinopharm
187 Chemical Reagent Co., Ltd. The sodium silicate solution, sodium hydroxide and solvent
188 water were first mixed to prepare the alkali activator solutions. The solutions first
189 settled for 4 hours before mixing to eliminate the solutions' instability (Yan et al.,
190 2016).

Table 1: Mix proportions of AARBP paste

Sample ID	Brick powder (g)	Na_2SiO_4 (g)	NaOH (g)	AS/N ratio	W/B ratio
BP-AS/N-4.0	1000	456	120.03	4.0	0.3
BP-AS/N-4.5	1000	456	101.72	4.5	0.3
BP-AS/N-5.0	1000	456	85.56	5.0	0.3
BP-AS/N-5.6	1000	456	71.20	5.6	0.3
BP-AS/N-6.3	1000	456	58.35	6.3	0.3
BP-AS/N-7.1	1000	456	46.78	7.1	0.3
BP-AS/N-8.0	1000	456	36.32	8.0	0.3
BP-AS/N-9.0	1000	456	26.81	9.0	0.3

191 The AS/N ratios of 4, 4.5, 5, 5.6, 6.3, 7.1, 8, and 9 were designed to screen the
192 optimum mixes under a constant water-to-binder (W/B) ratio of 0.3. The specific mix
193 proportions of each AARBP paste are shown in Table 1. The precisely weighed brick
194 powder and alkali activator solutions were mixed together in a mixing bowl with high-

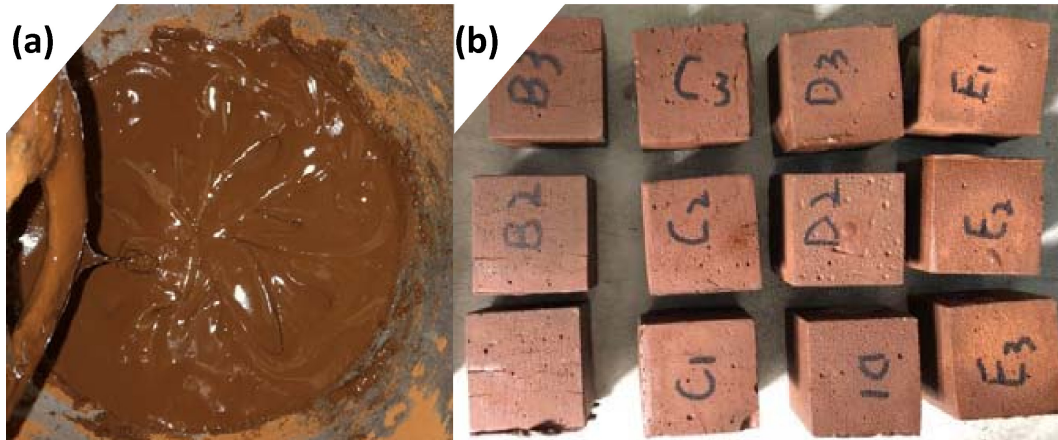


Figure 6: (a) Slurries of the AARBP paste, and (b) the hardened paste cubes.

195 speed stirrings for 3 min to obtain homogeneous slurries (Fig. 6a). The AARBP paste
196 slurries were then cast into cubic moulds ($40 \times 40 \times 40 \text{ mm}^3$). High-frequent vibrations
197 were operated to the slurries-filled cubic moulds to remove the air bubbles entrapped
198 in the AARBP pastes. After a surface finishing process, all the open surfaces were
199 covered with a layer of plastic film to avoid the possible drying shrinkage caused by
200 water loss. The moulds were then stored in a sealed chamber with the temperature of
201 $65 \pm 2 \text{ }^\circ\text{C}$ to accelerate the chemical interactions between the brick powder and alkaline
202 activators (Reig et al., 2013; Ulugol et al., 2021a). After 24 hours' primary curing, the
203 paste specimens were demoulded and again sealed with plastic films for further curing
204 in the same chamber. At set ages, all AARBP paste specimens were removed from the
205 curing chamber and cooled in ambient temperature for the designed tests.

206 Fig. 6b shows a typical picture of the hardened AARBP paste cubes. Apparently,
207 after the alkaline activating processes, the AARBP paste specimens kept the same red
208 color with the sintered clay brick. This aesthetic characteristic of AARBP materials

Table 2: Mix proportions of the non-cement CDW pervious concrete

Sample ID	Brick powder	AS/N ratio	W/B ratio	Aggregate (g)	B/A ratio
CDPC-B/A-0.15	500	6.3	0.3	3333	0.15
CDPC-B/A-0.20	500	6.3	0.3	2500	0.20
CDPC-B/A-0.25	500	6.3	0.3	2000	0.25
CDPC-B/A-0.30	500	6.3	0.3	1667	0.30

209 may broaden their application scenarios for decorations and repairs other than con-
 210 structions (Sassoni et al., 2016).

211 3.2.2. Fabrication of non-cement CDW pervious concrete

212 The cementitious AARBP material alternative to OPC was used as the binder
 213 to fabricate non-cement CDW pervious concretes. The optimum AARBP paste mix
 214 (AS/N=6.3) was adopted according to the strength results (section 4.1). The binder-
 215 to-aggregate ratio (B/A), as the unique variable to impact the engineering properties of
 216 the pervious concrete, was set as 0.15, 0.2, 0.25 and 0.3. The detailed pervious concrete
 217 mix proportions are shown in Table 2.

218 The AARBP paste slurries at the optimum mix were first prepared according to the
 219 procedures presented in section 3.2.1. Then the low-quality CDW aggregate (size over
 220 1.18 mm) was poured into the mixing bowl with stirrings at the speed of 120 rpm for
 221 3 min. Due to the relatively low W/B ratio and the discontinuous aggregate grading,
 222 the obtained fresh concrete showed almost no fluidity (Fig. 7a). The fresh concrete was

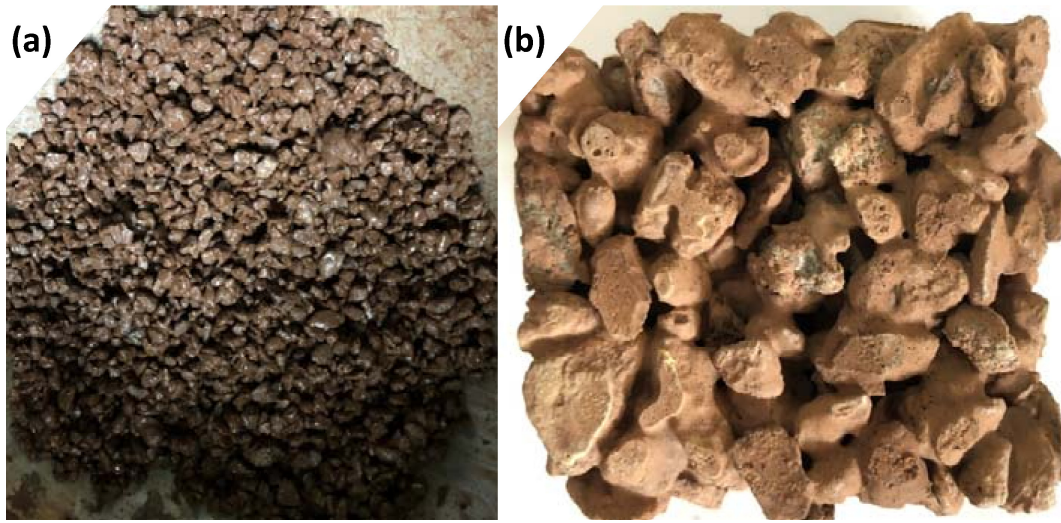


Figure 7: (a) In-situ mixing of pervious concrete, and (b) the hardened cubic specimens.

223 then cast into cubic moulds ($50 \times 50 \times 50 \text{ mm}^3$) for strength tests, and cylinder moulds
224 (diameter=63 mm and height=75 mm) for water permeability tests. After the moulds
225 were fully filled with the non-cement CDW concrete, manual press on the specimens was
226 operated to enhance the compactness. No vibrations were used to avoid the possible
227 pore clogging at the bottom of pervious concrete specimens (Cui et al., 2020). The
228 non-cement CDW pervious concrete specimens followed the same curing scheme of the
229 AARBP paste (i.e., sealed curing at $65 \pm 2 \text{ }^\circ\text{C}$).

230 Fig. 7b displays a typical picture of a demoulded pervious concrete cube. The spec-
231 imen surfaces were rough due to the insufficient fillings of the large space between dis-
232 continuously graded CDW aggregates. The insufficient space fillings indeed accounted
233 for the high water permeability of pervious concrete in design (AlShareedah and Nassiri,
234 2021; Xie et al., 2020).

235 *3.3. Methods*

236 *3.3.1. Mechanical tests*

237 Compression tests were performed to cubic specimens in an Instron 8802 full func-
238 tional test machine. The loading speed was maintained at 0.35 kN/s controlled by its
239 electro-hydraulic servo system. As the test proceeded, the forces accumulated, where
240 the maximum values were recorded to calculate compressive strength.

241 Three independent tests were conducted for each AARBP paste, and six for each
242 CDW pervious concrete due to its large data variance. The statistical results of com-
243 pression strength for each mix and curing age were analyzed and plotted with the
244 software of Origin (version 9.1).

245 *3.3.2. Water permeability test*

246 Water permability tests for the non-cement CDW pervious concrete were conducted
247 according to a Chinese standard of permeable paving bricks & permeable paving flags
248 (GB/T 25993-2010). A home-made testing setup in a constant-head regime was used
249 to measure the water permeability. As shown in Fig. 8, a cylindrical pervious concrete
250 specimen was first fixed in a plastic tube, where the side surface of the specimen was
251 sealed with an epoxy resin. The tube space above the specimen provided a water
252 reservoir before permeation (named as the top reservoir). The plastic tube, together
253 with the fixed concrete specimen, was put into a bigger plastic bucket that acted as
254 another reservoir (named as the bottom reservoir) to store the permeated water. Two
255 drain pipes were connected to the top and bottom reservoirs respectively to control the

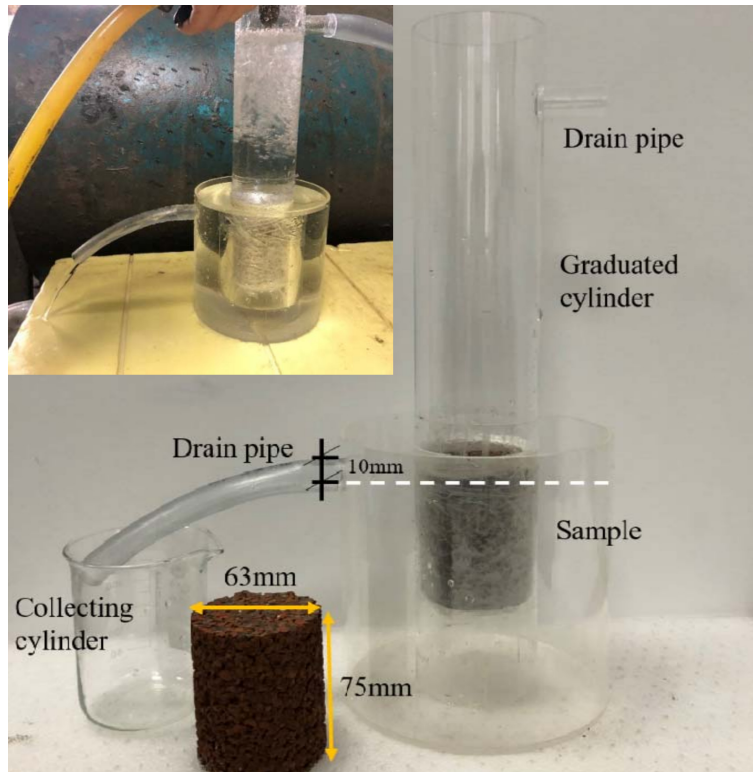


Figure 8: Setup for water permeability test (Inset panel: picture of an in-situ water permeability test).

256 water head.

257 During the test, the steady water flows Q in a fixed period t was recorded (see the
 258 inset panel of Fig. 8 for a snapshot of in-situ water permeability test), so the water
 259 permeability K can be expressed as,

$$K = \frac{QL}{AHt} \quad (1)$$

260 where L and A are the height and cross sectional area of the specimen, H is the water
 261 head. In this work, $L = 7.5$ cm, $A = 31.17$ cm² and $H = 18$ cm, so $K = 0.01337Q/t$
 262 (cm/s).

263 *3.3.3. X-ray diffraction*

264 X-ray diffraction (XRD) tests were conducted to characterize the minerals in dif-
265 ferent AARBP mixes. Each AARBP paste was milled to powder with the 200-mesh
266 passing ratio of 95% for XRD tests. Isopropanol was used as the protecting liquid
267 during milling to avoid the interactions between AARBP hydrates and the air. XRD
268 scans were conducted in a Bruker D8 Advance diffractometer. $\text{CuK}\alpha$ radiations with
269 the wavelength of $\lambda=0.15419$ nm were applied in the scanning angles between 5 and 90
270 °with the step length of 0.02 °. A software of MDI Jade 6 was used to identify main
271 mineral phases in the tested samples.

272 *3.3.4. X-ray computed tomography*

273 X-ray computed tomographic (XCT) tests were used to non-destructively measure
274 the macro porosity, pore size/spatial distribution and skeleton of the non-cement CDW
275 pervious concretes, because X-ray attenuation is generally sensitive to material density
276 (Zeng et al., 2019; Brisard et al., 2020). Each concrete cylinder was fixed on the sample
277 frame, and rotated evenly by 360 °in 1800 s during the penetrations of X-ray beams
278 in a device of XTH255/320 LC (Nikon, Japan). The accelerating voltage of 180 kV
279 and the beam current of 160 μA were used. A high-resolution detector (2000 \times 2000
280 pixels) at the back of the specimen continually recorded the attenuated X-ray beams,
281 and transferred the X-ray attenuation signals to massive transmission X-ray projections
282 at different angles. For each sample, 2500 transmission X-ray projections were recorded
283 and loaded into a CTPro software to generate numerous 8-bit gray images.

284 A software of VG Studio MAX 3.1 was used for further data processing including
285 region-of-interest (ROI) selection, threshold for phase segmentation, and microstructure
286 reconstruction (Zeng et al., 2020; Qi et al., 2021). The pixel size of the images was 55
287 μm .

288 *3.3.5. Scanning electron microscopy*

289 Microstructure analysis was performed via a field emission environmental SEM in
290 type of Quanta FEG650. Back-scattered electrons (BSE) mode was applied to obtain
291 high quality BSE images for phase analysis. Small concrete blocks (around 10 mm)
292 including both recycled C&D aggregates and AARBP pastes collected from the central
293 part of pervious concretes were impregnated in cylindrical molds with epoxy resin for
294 sample encasement. After the epoxy resin was hardened, the samples were demoulded
295 and polished in a Buehler semi-automatic polishing machine. Diamond papers in the
296 grade grits of 400, 800, 1200, 2000, and 4000# were used for sample surface polishing
297 with 1 min for each grit. An oven-drying at 40 °C for 24 h was performed to remove
298 the capillary water confined in pores of the pervious concrete samples.

299 During the SEM tests, the accelerating voltage and spot size were set as 20 keV
300 and 5.0 nm, respectively. Images with different magnifications (50 \times to 1000 \times) were
301 acquired for the analysis of skeletons, pores, paste microstructure and matrix-aggregate
302 interfaces.

303 4. Results and discussion

304 4.1. Optimum AARBP mix

305 Fig. 9 shows the compressive strengths and densities of the AARBP pastes with 8
306 different AS/N ratios cured for 7 d and 28 d. Apparently, increasing the curing age
307 systematically raised the compressive strength. Compared with the strength data of the
308 AARBP samples at 7 d (15 to 25 MPa), those at 28 d were greatly increased by between
309 30% and 93% (22 to 50 MPa) (Fig. 9a). The great strength promotions were caused
310 by the enhanced chemical reactions between the brick powder and alkaline activators
311 under the longer curing periods (Zhang et al., 2021).

312 Fig. 9a also displays the effects of AS/N ratio on AARBPs' strength. As the AS/N
313 ratio increased from 4 to 9, the compressive strength rose slowly to peak values followed
314 by a rapid fall to nearly constant values. Peak strengths were observed for the mixes
315 with the AS/N ratios of 5.6 and 6.3 (around 25 MPa at 7 d and 50 MPa at 28 d; see the
316 shadowed area shown in Fig. 9a). Too high or too low NaOH content would adversely
317 impact the strength of AARBP, similar trends were reported elsewhere (Tuyan et al.,
318 2018).

319 Densities of the AARBP pastes at 28 d are illustrated in Fig. 9b. Unlike the rise and
320 fall of compressive strength with AS/N ratio, the samples' density showed an almost
321 monotonously increasing trend from 1600s to 1700s kg/m³ as AS/N ratio increased.
322 This trend was reasonably caused by the higher density of the activator with higher
323 modulus.

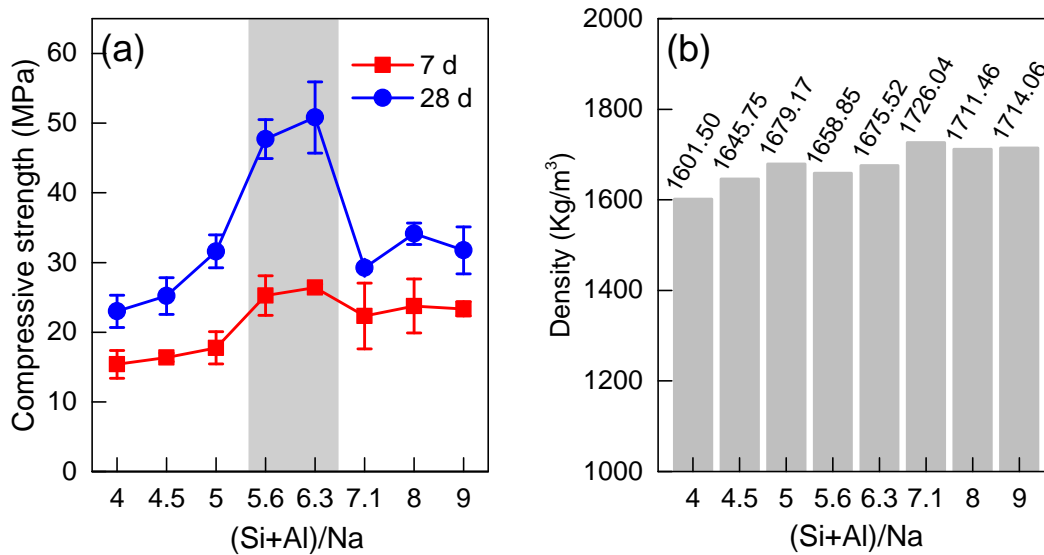


Figure 9: (a) Compressive strength and (b) density of AARBP pastes at different mixes.

324 It was suspected that AS/N ratios may alter the chemical reaction products, how-
 325 ever, the XRD patterns shown in Fig. 10 suggested no significant differences in the
 326 hydration products between the AARBP pastes. Characteristic spectra of silica from
 327 the sintered clay bricks (quartz) and water glass dominated the XRD patterns with the
 328 similar intensities (Fig. 10). Signals of albite ($\text{Na}_2\text{O} \cdot \text{Al}_2\text{O}_3 \cdot 6\text{SiO}_2$) played the secondly
 329 important role on the XRD patterns, implying that the active silica and alumina in
 330 brick powder had partially reacted with the alkali activators. However, AS/N ratio
 331 seemed to have no impacts on the intensities of albite's XRD spectra (Fig. 10). Con-
 332 sidering the similar minerals in the AARBP pastes, the mix of BP-AS/N-6.3 with the
 333 highest strength (Fig. 9a) was selected as the basic binder mix for pervious concrete
 334 fabrication.

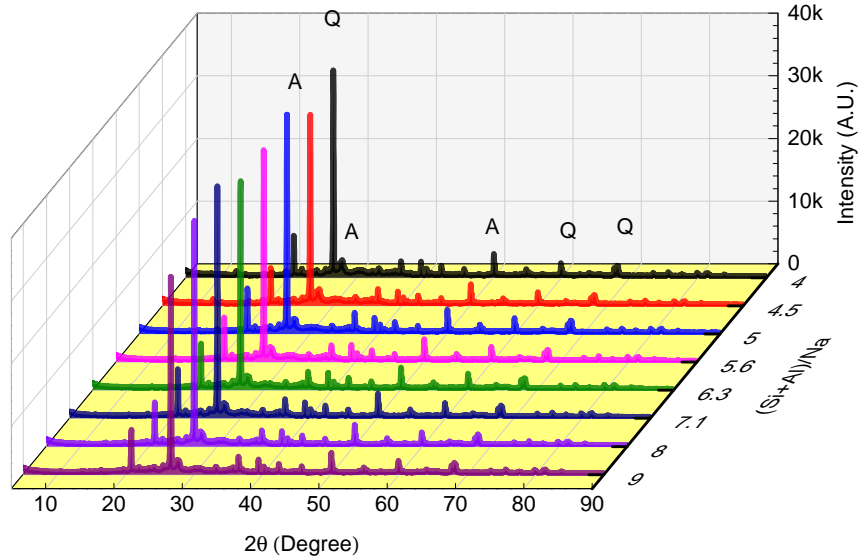


Figure 10: XRD patterns of the alkaline-activated recycled brick powder pastes (Q: Quartz, A: Albite).

335 *4.2. Engineering performances of pervious concrete*

336 Physical and mechanical properties (i.e., compressive strength, density and water
 337 permeability coefficient) of the non-cement recycled CDW pervious concrete are il-
 338 lustrated in Figs. 11 and 12. As shown in Fig. 11a, the pervious concrete displayed
 339 relatively low compressive strengths (less than 5 MPa for all concrete mixes). At the
 340 B/A ratio of 0.15, the concrete blocks only had the strength of 0.6 MPa regardless
 341 of curing age, and the density of 1256 kg/m³ (Fig. 11b), which was even lower than
 342 the bulk density of the recycled CDW aggregate (1350 kg/m³). The low compressive
 343 strength and density of CDPC-BA-0.15 were caused by two main reasons: 1), the recy-
 344 cled CDW aggregates were loosely compacted during the concrete fabrication processes
 345 because no vibrations were performed to avoid the possible pore clogging; and 2) the
 346 limited AARBP paste had poor binding effect on the recycled CDW aggregates (see

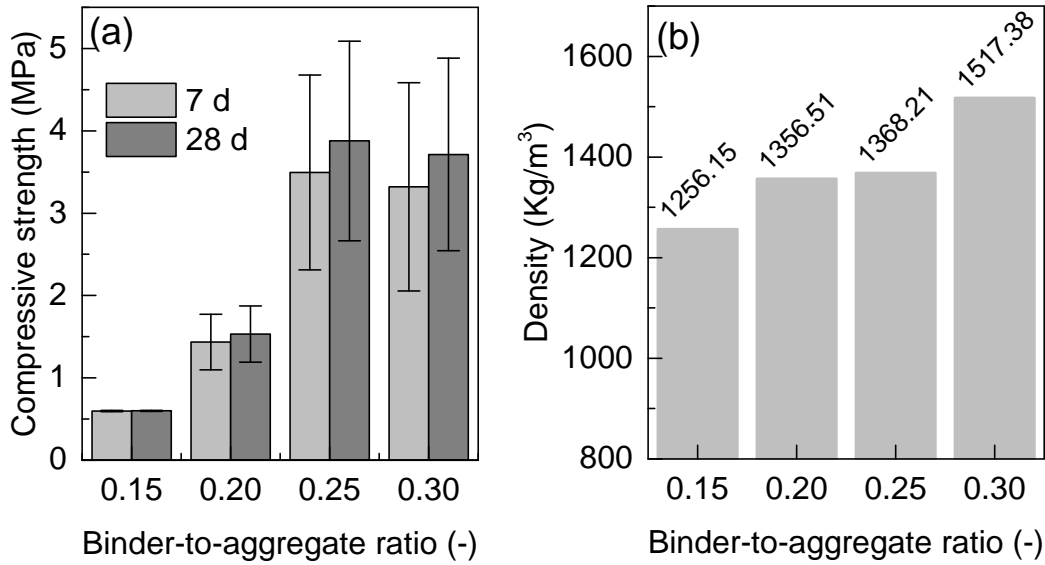


Figure 11: Compressive strength (a) and density (b) of the non-cement pervious concrete with different binder-to-aggregate ratios.

347 section 4.3.2 for more information).

348 As the binding phase content (or B/A ratio) increased, the pores were partially filled
 349 by the AARBP paste and the bonds between two neighbored CDW aggregates were
 350 enhanced, so the compressive strength increased (Fig. 11a). When the B/A ratio was
 351 higher than 0.25, the compressive strengths were over 3 MPa. Within the same regime,
 352 densities of the non-cement pervious concrete specimens increased with increasing B/A
 353 ratio (Fig. 11b). This followed the general concept of pervious concrete design, where
 354 increases of the coating paste around aggregates can enhance the mechanical properties
 355 of pervious concrete (Shen et al., 2020).

356 Fig. 12 shows the water permeability coefficients and their relationship with the
 357 compressive strengths for the non-cement recycled CDW pervious concrete with dif-

358 ferent B/A ratios. The water permeability coefficients were greatly higher than the
359 thresholds of pervious concrete pavements and bricks suggested by the code of GB/T
360 25993-2010 ($K \geq 0.02$ cm/s for level A and ≥ 0.01 cm/s for level B). The high wa-
361 ter permeability coefficients were caused by the high contents of connected pores (see
362 section 4.3 for pore structure analysis).

363 As the B/A ratio increased from 0.15 to 0.3, the water permeability coefficient
364 decreased from 0.48 cm/s to 0.26 cm/s (Fig. 12a). This was certainly induced by the
365 reduction of connected channels for water permeation when more binders were used to
366 fill the gaps between recycled CDW aggregates (see section 4.3.1 for specific evidences).
367 Analysis showed that the water permeability coefficient almost linearly decreased with
368 the increase of compressive strength (Fig. 12b). The results evidenced the opposite
369 roles of the binding phase on the water permeation and strength of pervious concrete
370 (Huang et al., 2021; AlShareedah and Nassiri, 2021).

371 *4.3. Microstructure and mechanisms*

372 Our focuses were then shifted onto the multi-scale structures of the non-cement
373 recycled CDW pervious concrete, which helped explore the pore structure mechanisms
374 of water permeation and strength changes. 3D pore structures resolved by XCT were
375 used to address the connected and isolated pores, while 2D multi-scale measurements
376 by SEM/BSE were applied to address the structure of skeleton, matrix-aggregate ITZ,
377 and AARBP paste.

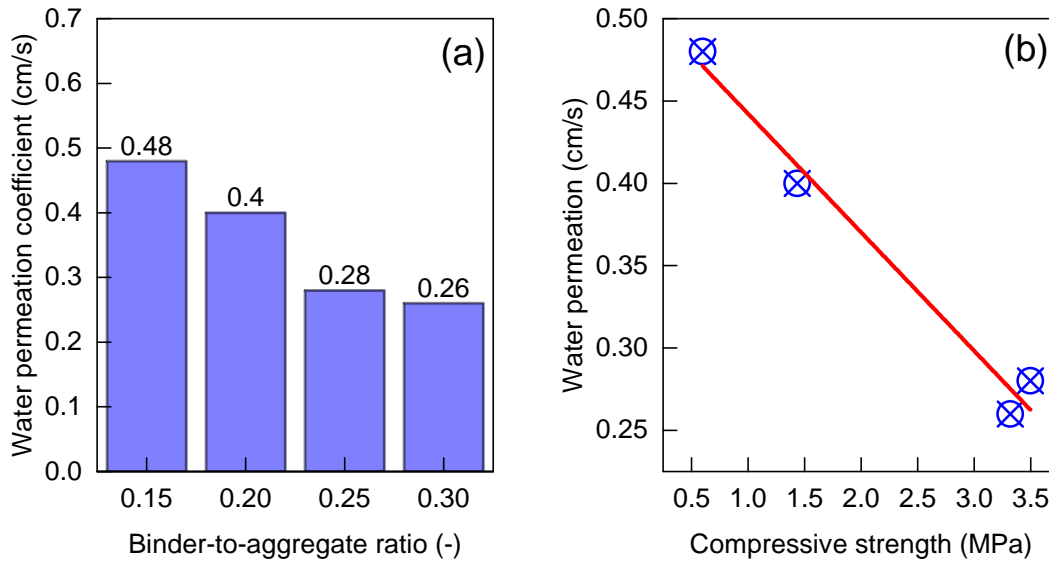


Figure 12: Water permeation coefficient (a) and its relationship with compressive strength (b) of the non-cement recycled C&D waste pervious concrete with different B/A ratios.

378 4.3.1. Pore structure

379 Fig. 13 shows 3D structures of the selected pervious concrete cylinders with different
 380 B/A ratios, where the connected and isolated pores were separately illustrated in red
 381 and blue, respectively. At a first glance, the reconstructed pervious concrete cylinders
 382 before pore segmentation showed heterogeneous color distributions due to the different
 383 attenuation values of different phases (Brisard et al., 2020; Zeng et al., 2019). For
 384 the pervious concrete specimens, the lighter color represented the phases with higher
 385 X-ray attenuation (e.g., stones and metals), while the darker color denotes the phases
 386 with lower X-ray attenuation (e.g., pores and wood pieces); see the bottom half of each
 387 overview cylinder shown in Fig. 13. Pore segmentation was then performed on the top
 388 half of each cylinder. Clearly, pores were densely distributed in the cylinders, suggesting

389 the highly porous structure of the pervious concrete.

390 Huge connected pores were identified for each cylinder (middle column of Fig. 13),
391 accounting for the large water permeability coefficients (Fig. 12a). Only small amount
392 of pores were entrapped in the AARBP paste (right column of Fig. 13). Those pores
393 were diagnosed as the isolated or closed pores in the present XCT resolution ($55 \mu\text{m}$),
394 which would have no significant contribution to water permeation.

395 The specific cumulative porosity distributions from the XCT data were also analyzed
396 (Fig. 14), as pore size distribution can decisively dominate the water permeability of
397 pervious concrete (Huang et al., 2021). The sharp porosity accumulation at a large
398 size for each pervious concrete was identical to the connected pore shown in Fig. 13.
399 After the pore size decreased by around one order of magnitude, porosity began to
400 rise, suggesting that the isolated pores in different sizes appeared in the AARBP pastes
401 (Fig. 13).

402 The XCT characterized pore structure was greatly affected by the B/A ratio. First,
403 the B/A ratio impacted the total porosity (Fig. 15). As the designed B/A ratio increased
404 from 0.15 to 0.2, 0.25 and 0.3, the total porosity decreased from 0.43 to 0.33 (by 12%),
405 0.28 (by 34%) and 0.25 (by 41%), respectively, due to the pore space fillings by the
406 AARBP paste. Consequently, the isolated porosity in the AARBP paste increased
407 from 0.002 (B/A=0.15) to 0.009 (B/A=0.25). The connected porosity showed the
408 similar trend with the total porosity, due to its dominative occupation of the pores
409 (over 96%) (Fig. 15). Second, the B/A ratio also affected the pore size distribution. As
410 demonstrated in Fig. 14, the porosity rising rates increased with the increase of B/A

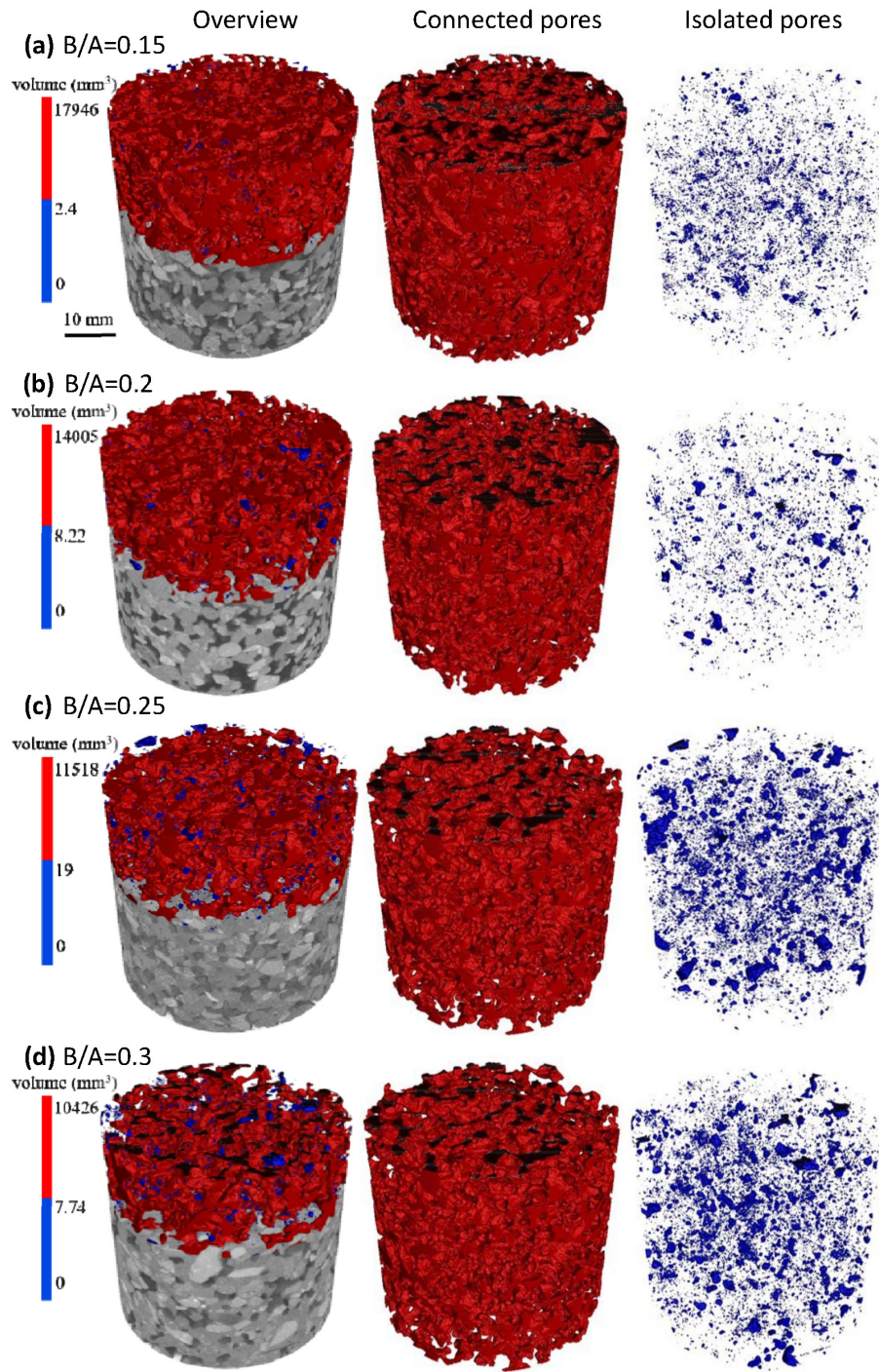


Figure 13: 3D pore structure of pervious concrete cylinders with different B/A ratios: (a) B/A=0.15, (b) B/A=0.20, (c) B/A=0.25, and (d) B/A=0.30 (left: overview of 3D pore structure; middle: connected pores; right: isolated pores).

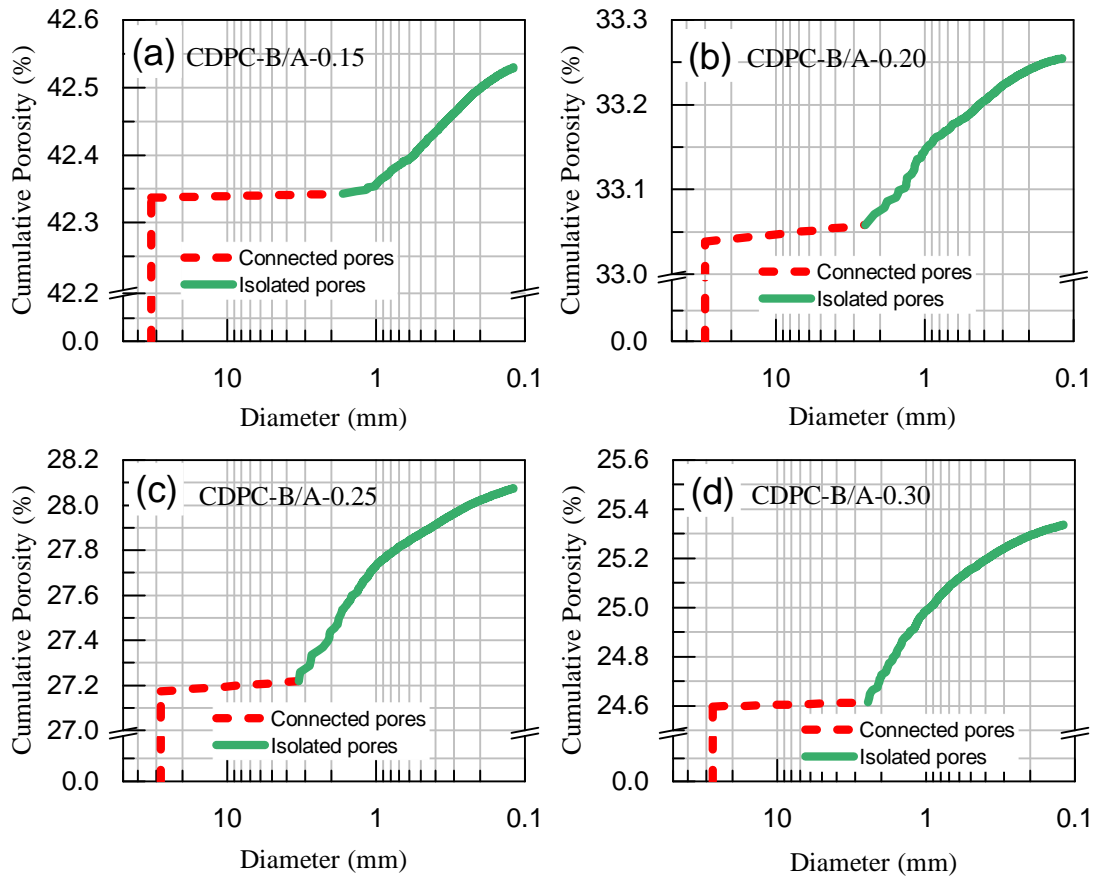


Figure 14: Cumulative connected and isolated pore distributions of (a) CDPC-B/A-0.15, (b) CDPC-B/A-0.20, (c) CDPC-B/A-0.25, and (d) CDPC-B/A-0.30..

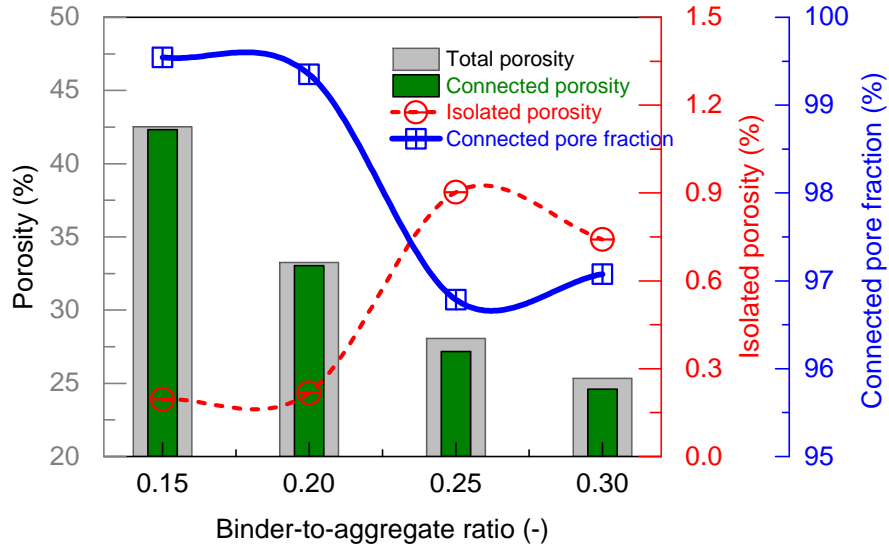


Figure 15: Total, connected and isolated (closed) porosities and the connected pore fraction of the pervious concrete with different binder-to-aggregate ratios.

ratio, and the plot of cumulative porosity versus logarithmic pore diameter progressively
 changed from linear to non-linear. The accelerated porosity rises were mainly observed
 at the pore size interval between 1 and 3 mm. Indeed, if those pores were excluded, the
 porosities with the size below 1 mm showed limited changes with B/A ratio (Fig. 14).
 The increases of closed macro pores with B/A ratio were directly evidenced by the XCT
 images (Fig. 13).

4.3.2. Skeleton, paste and ITZ

Skeletons of the pervious concrete samples were observed via BSE images at low
 magnifications. Figs. 16a-d show the selected 2D BSE images for representative demon-
 strations. Recycled CDW aggregates were covered with a layer of AARBP paste, and
 bonded together to form the skeletons, and the rest spaces formed the connected pores

422 (Fig. 13). For the CDPC-BA-0.15 sample, merely small amount of AARBP paste was
423 attached on the recycled CDW aggregates, forming rough and tortuous skeletons. As
424 shown in Fig. 16a, a needle-like aggregate was insert between two aggregates with the
425 limited bonding areas and paste thickness (around 0.1 mm), leaving the rest spaces to
426 form large, connected pores. This skeleton structure would be certainly not able to
427 sustain high external loads, so the compressive strength was low (Fig. 9). The adverse
428 effect of tortuous skeletons on strength of porous materials was also evidenced from a
429 foamed concrete system (Jin et al., 2021).

430 As the AARBP paste content increased, the paste layers were thickened and the
431 contacted areas were increased progressively; several recycled CDW aggregates were
432 bonded together to form larger solid clusters (Fig. 16c-d). The average paste thickness
433 was increased to 0.15, 0.22 and 0.36 mm for the samples of CDPC-BA-0.20, CDPC-BA-
434 0.25 and CDPC-BA-0.30, respectively. The increase of skeletons' volume facilitated to
435 build a strengthened structure (AlShareedah and Nassiri, 2021; Shen et al., 2020). As
436 a consequence, both the densities and strengths were substantially raised as the B/A
437 ratio increased (Fig. 9).

438 It is also interesting to explore the AARBP paste's microstructure and the paste-
439 aggregate ITZ. Fig. 16e shows the local ITZ between AARBP paste and a recycled CDW
440 aggregate for the CDPC-BA-0.25 sample. The relatively large contrast between the
441 paste (darker) and aggregate (brighter) allowed us to easily distinguish between the two
442 phases. No cracks were observed in the ITZ, suggesting the relatively good compatibility
443 and bonds between the paste and recycled aggregate. The rough surfaces of the recycled

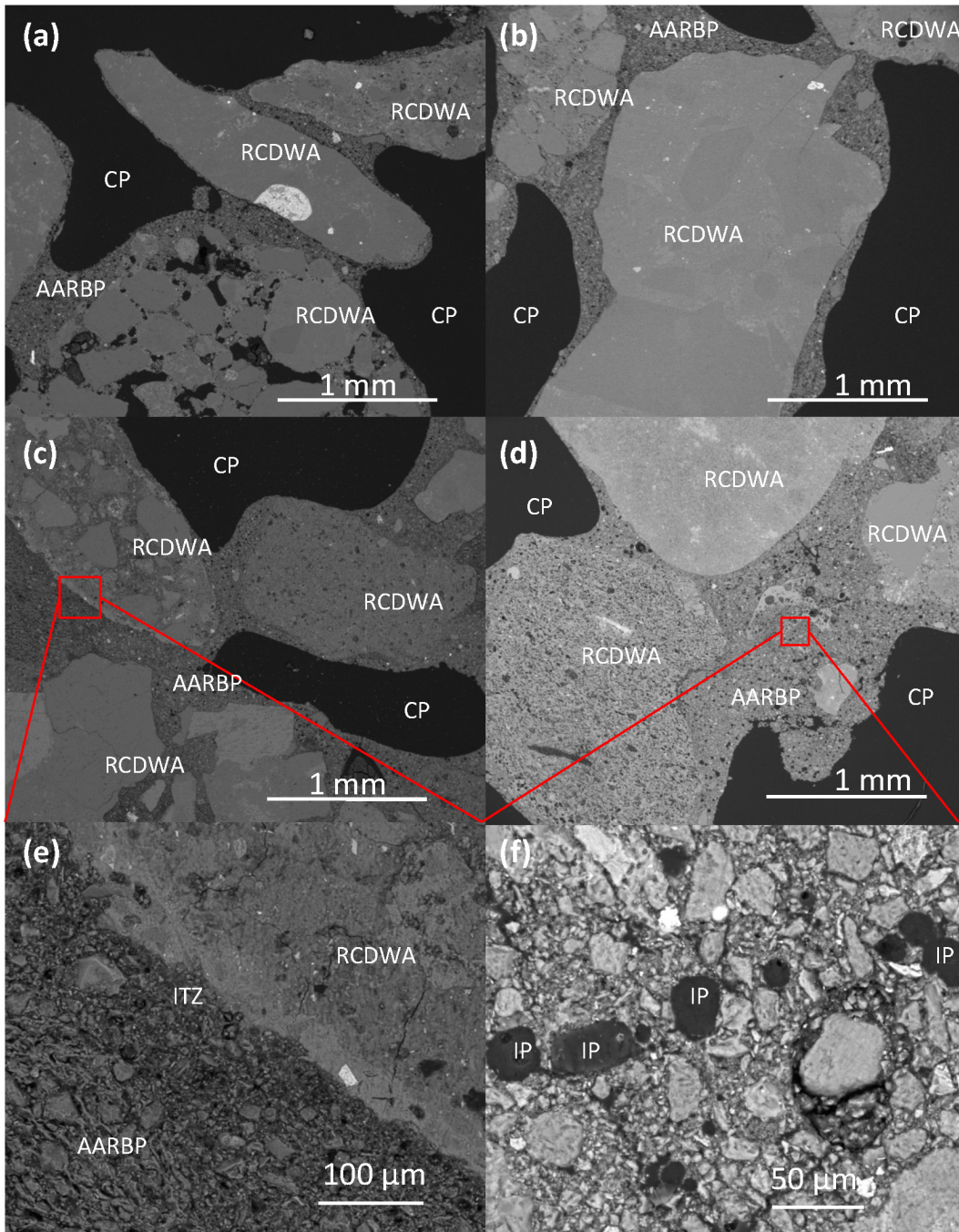


Figure 16: BSE images of (a) CDPC-B/A-0.15, (b) CDPC-B/A-0.2, (c) CDPC-B/A-0.25, (d) CDPC-B/A-0.3, (e) local interfacial area of CDPC-B/A-0.25, and (f) local paste matrix of CDPC-B/A-0.3 (AARBP: alkaline-activated recycled brick paste, RCDWA: recycled CDW aggregate, and P: pore) .

444 aggregates may account for this observations shown in Fig. 16e. First, cavities on the
445 recycled CDW aggregate surfaces may accommodate fine AARBP particles to enhance
446 the fillings of the ITZ spaces. Second, rough surfaces on the recycled CDW aggregates
447 may provide more sites for the nucleation and growth of the hydration products like
448 CSAH. Both the space filling and nucleation effects may substantially decrease the 'wall
449 effect' that raises the porous structure of ITZ in concrete with natural aggregates (Zhan
450 et al., 2020).

451 As to the AARBP paste, micro brick particles were compacted together in certain
452 patterns to form the paste matrix that acts as the binding phase to bond the recycled
453 CDW aggregates together. Some large milled brick particles can be clearly observed
454 from the BSE images (Fig. 16f), suggesting the limited reaction extent of the recycled
455 brick powder. However, due to the lack of vibrations, air voids and/or flaws in the
456 thick paste can be not eliminated, so large isolated pores can be seen in Fig. 16f. The
457 entrapped air voids and/or flaws also accounted for the increased pores detected by
458 XCT (Fig. 14d).

459 *4.3.3. Discussion of mechanisms*

460 1, Why is the AARBP strength much higher than that of the non-cement recycled
461 CDW pervious concrete?

462 The compressive strength of the selected AARBP paste (AS/N=6.3) was as high as
463 50 MPa (Fig. 9a), higher than that of the non-cement recycled CDW pervious concrete
464 with the same binding material over one order of magnitude (less than 5 MPa; see

465 Fig. 11a). The most decisive factor is the loose compactness of the CDW aggregates
466 in the pervious concrete system (Pieralisi et al., 2021; Yang et al., 2021). Unlike ordi-
467 nary concrete that must experience vibrations during specimen casting to enhance the
468 compactness and expel the air voids out of the cement matrix, pervious concrete with
469 severe vibrations was not recommended in order to prevent pore clogging caused by
470 the sinking of paste (Cui et al., 2020). Furthermore, compared with natural aggregates
471 with relatively smooth surfaces, the recycled CDW aggregates have rougher surfaces,
472 leading to the higher frictions between two neighbored aggregates (Li et al., 2019).
473 Consequently, the spatial compactness of rough particles without vibrations would be
474 much looser than that of smooth particles with sufficient vibrations. The attachment
475 of a thin layer of AARBP paste onto the CDW aggregates still can not compensate
476 for the loose compactness, so the density of CDPC-BA-0.15 (Fig. 9b) was even lower
477 than that of the pure CDW aggregates after sufficient vibrations. In the regimes, the
478 non-cement recycled CDW pervious concrete showed relatively low strength.

479 2, What are the roles that different pores played on the strength and water perme-
480 ability?

481 Two different types of pores were diagnosed by XCT according to the pores' connec-
482 tion (Fig. 14), and they would play different roles on water permeability and strength.
483 The connected pores provided open channels for water permeation, while the isolated
484 pores, together with the connected pores, jointly impacted the strength of materials
485 (AlShareedah and Nassiri, 2021; Deo and Neithalath, 2010; Zhang and Wille, 2016).
486 As shown in Fig. 15, all the pervious concrete specimens showed high contents of con-

487 nected pores, so high water permeabilities were measured (Fig. 12a). As the B/A
488 ratio increased, the skeletons became thicker and the total porosity decreased, so the
489 strength increased (Fig. 9a). However, the method of strength promotion by only in-
490 creasing paste thickness may be not so effective, as the AARBP pastes were porous
491 (Fig. 13 and 16f). Elimination of the voids and/or flaws in the binding paste may be
492 another way to improve the strength of skeletons, which deserves the further rigorous
493 investigations in the future.

494 **5. Conclusion remarks and perspectives**

495 In this work, low-quality CDW in-situ collected from a local urban-fringe of Hangzhou,
496 China was analyzed; non-cement pervious concrete was fabricated using the recycled
497 CDW aggregates and AARBP paste; and the engineering performances and multi-scale
498 structure of the pervious concrete were systematically investigated. Conclusions can be
499 drawn as follows.

- 500 1. The low-quality CDW contained 22.4% dusts and soils with the size below 1.18
501 mm, and 67.6% macro particles with the size between 1.18 and 10 mm. Concrete
502 (59%), stone (26%), brick (10%) and other solids (5%) in the CDW were recycled
503 as aggregates for the fabrication of pervious concrete.
- 504 2. Concept of developing non-cement recycled CDW pervious concrete was proposed.
505 First, sintered clay bricks recycled from the same urban-fringe area were milled
506 to powder and activated by alkaline activators in different AS/N ratios to act
507 as the cementing material. Second, the optimal AARBP paste was mixed with

508 the recycled CDW aggregates for fabricating non-cement pervious concrete with
509 different B/A ratios.

- 510 3. Increasing the curing age systematically promoted the compressive strength of
511 AARBP paste by 30% to 93%. The AARBP paste with the AS/N ratio of 6.3
512 showed the peak strength of 50 MPa at 28 d. Increasing the AS/N ratio enhanced
513 the AARBP pastes' density, but had no significant impacts on the XRD patterns.
- 514 4. The non-cement recycled CDW pervious concrete specimens showed relatively
515 low compressive strength, but high water permeability. Two pore classes (i.e.,
516 connected pores and isolated pores) were resolved by XCT. Increasing the B/A
517 ratio decreased the connected porosity, but increased the isolated porosity.
- 518 5. The recycled CDW aggregates and the AARBP paste formed tortuous skeletons
519 with limited contribution to strength development. Tight paste-aggregate inter-
520 actions were observed between the AARBP paste and recycled CDW aggregate.
521 The AARBP paste showed porous microstructure owing to the lack of vibrations
522 during the casting procedures.

523 Overall, the non-cement recycled CDW pervious concrete fabricated in this work
524 proofed the proposed concept of total recycling of low quality CDW without using
525 external cementing materials. Within the regime, the combined uses of recycled CDW
526 and AARBP paste as the aggregate and binder provide an sustainable solution for
527 the concrete industry. Going beyond this, the ecological benefits of total recycling of
528 CDW (reduction of CO₂ emissions without cement uses) provide promising engineering

529 solutions to the large-scale ongoing buildings and demolitions in China.

530 **Declarations of interest**

531 None.

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