



Waves off Gopalpur, northern Bay of Bengal during Cyclone Phailin

M. M. Amrutha¹, V. Sanil Kumar¹, T. R. Anoop¹, T. M. Balakrishnan Nair², A. Nherakkol², and C. Jeyakumar²

¹Council of Scientific and Industrial Research – National Institute of Oceanography, Dona Paula 403004, Goa, India

²Indian National Centre for Ocean Information Services (Ministry of Earth Sciences), “Ocean Valley”, Pragathi Nagar (BO), Nizampet (SO) 500090, Hyderabad, India

Correspondence to: V. Sanil Kumar (sanil@nio.org)

Received: 6 March 2014 – Revised: 19 May 2014 – Accepted: 31 July 2014 – Published: 2 September 2014

Abstract. The wave statistical parameters during Cyclone Phailin which crossed the northern Bay of Bengal are described based on the Directional Waverider buoy-measured wave data from 8 to 13 October 2013. On 12 October 2013, the cyclone passed within 70 km of the Waverider buoy location with a wind speed of 59.2 m s^{-1} (115 knots), and during this period, a maximum significant wave height of 7.3 m and a maximum wave height of 13.5 m were measured at 50 m water depth. Eight freak wave events are observed during the study period. The ratio of the maximum wave height to significant wave height recorded is found to be higher than the theoretical value and the ratio of the crest height to wave height during the cyclone was 0.6 to 0.7. The characteristics of the wave spectra before and after the cyclone is studied and found that the high-frequency face of the wave spectrum is proportional to f^{-3} before the cyclone and is between f^{-4} and f^{-5} during the cyclone period.

Keywords. History of geophysics (ocean sciences)

1 Introduction

In the tropical oceans, cyclones are one of the most commonly occurring natural hazards and can have significant impact on the coast. The waves generated during cyclones govern the design of marine facilities, and hence the characteristics of the waves during cyclones are required for planning and design of offshore facilities and for planning mitigation measures. A number of studies were carried out to understand wave generation and wave growth during hurricane (Young, 2006; PrasadKumar and Stone, 2007; Xu et al., 2007; Chu and Cheng, 2008; Soomere et al., 2008; Fan et al.,

2009; Babanin et al., 2011). As of today, the largest significant wave height (H_s) measured was 24 m during Typhoon Krosa in 32 m water depth (Babanin et al., 2011). In the Bay of Bengal, the largest H_s measured is 8.4 m on 28 October 1999 during the passage of the Orissa super cyclone (Rajesh et al., 2005). A number of oil and gas platforms are planned along the east coast of India and the crest height of the wave during the extreme wave condition decides the air gap of the offshore platforms (Kumar et al., 2013).

Considering the importance of the knowledge of wave statistical parameters during cyclones, a study is carried out by analysing the wave data measured in the nearshore waters of northern Bay of Bengal during the passage of Cyclone Phailin. The objective of this paper is to study the wave spectral characteristics and the wave statistical parameters of the cyclone-generated high waves.

2 Materials and methods

In connection with the real-time wave data collection program, a Datawell Directional Waverider buoy was moored at 15 m water depth ($19^{\circ}16.87' \text{ N}$, $84^{\circ}57.76' \text{ E}$) off Gopalpur (Fig. 1) and the data collection was ongoing since February 2013. The deployed buoy drifted from its moored location on 12 October 00:00 UTC due to the impact of the cyclone and the GPS data show that the buoy remained within 100 km from the cyclone track on 12 October 2013 (Fig. 2). The cyclone track and the position of the buoy are presented in Table 1. The time referred to in the paper is Coordinated Universal Time (UTC) and the local time is Indian Standard Time (IST), which is UTC +5:30. The data measured during 8–13 October 2013 are analysed to study the wave

Table 1. Cyclone position, wind speed, buoy location and wave parameters during 9–13 October 2013.

Date	Time (GMT)	Cyclone position		Central pressure (mbar)	Wind speed (m s ⁻¹)	Buoy location		Distance of buoy from cyclone (km)	H_s (m)	T_p (s)	D_p (deg)
		Latitude (°N)	Longitude (°E)			Latitude (°N)	Longitude (°E)				
9	09:00	13.5	92.5	1000	15.43	19.2812	84.9635	1055.5	1.1	13.3	165
9	12:00	13.5	92.5	999	18.01	19.2812	84.9635	1055.5	1.0	20.0	152
9	15:00	13.6	92.5	999	18.01	19.2812	84.9635	1048.7	0.9	20.0	162
9	18:00	14.0	92.0	998	20.58	19.2812	84.9635	977.6	1.1	20.0	146
9	21:00	14.0	92.0	998	20.58	19.2812	84.9635	977.6	1.2	13.3	172
10	00:00	14.5	91.5	996	23.15	19.2812	84.9635	899.9	1.1	20.0	153
10	03:00	14.5	91.0	990	28.29	19.2812	84.9635	855.7	1.1	18.2	160
10	06:00	15.0	90.5	984	33.44	19.2812	84.9635	777.7	1.2	18.2	153
10	09:00	15.0	90.5	982	36.01	19.2812	84.9635	777.7	1.4	18.2	155
10	12:00	15.5	90.0	976	38.58	19.2812	84.9635	699.8	1.7	18.2	155
10	15:00	15.5	90.0	966	46.30	19.2812	84.9635	699.8	1.7	18.2	158
10	18:00	15.5	89.5	960	48.87	19.2812	84.9635	656.2	2.3	18.2	158
10	21:00	15.5	89.0	954	51.44	19.2812	84.9635	614.6	2.2	18.2	155
11	00:00	16.0	88.5	946	56.59	19.2812	84.9635	536.1	2.5	16.7	162
11	03:00	16.0	88.5	940	59.16	19.2812	84.9635	536.1	2.5	16.7	156
11	06:00	16.2	88.3	940	59.16	19.2812	84.9635	504.7	2.6	20.0	155
11	09:00	16.5	88.0	940	59.16	19.2812	84.9635	457.6	2.8	10.5	131
11	12:00	16.8	87.7	940	59.16	19.2812	84.9635	410.5	3.4	11.1	132
11	15:00	16.9	87.2	940	59.16	19.2812	84.9635	363.0	3.9	11.8	132
11	18:00	17.0	87.0	940	59.16	19.2812	84.9635	339.8	4.1	11.8	134
11	21:00	17.1	86.8	940	59.16	19.2812	84.9635	316.8	3.9	12.5	142
12	00:00	17.5	86.5	940	59.16	19.2812	84.9635	261.4	4.3	11.8	135
12	03:00	17.8	86.0	940	59.16	19.2687	84.9494	200.7	5.5	12.5	141
12	06:00	18.1	85.7	940	59.16	19.2096	84.8967	152.2	6.7	11.8	118
12	09:00	18.6	85.4	940	59.16	19.0906	84.8352	83.1	7.0	11.1	98
12	12:00	18.7	85.2	940	59.16	18.7963	84.7293	53.4	7.3	11.1	103
12	15:00	19.1	85.2	940	59.16	18.5021	84.5786	95.8	5.8	11.8	56
12	18:00	19.5	84.8	956	51.44	18.2863	84.5187	138.4	5.2	9.1	212
12	21:00	20.0	84.5	966	46.30	18.1281	84.4606	208.1	4.4	9.1	208
13	00:00	20.5	84.5	976	38.58	17.9914	84.4150	278.9	4.4	9.1	214
13	03:00	21.0	84.0	990	28.29	17.8768	84.3647	349.4	4.4	9.1	196
13	06:00	21.5	84.0	996	20.58	17.7795	84.3228	415.0	4.3	9.1	191
13	09:00	21.8	83.8	998	18.01	17.7119	84.2892	457.5	3.6	8.3	204
13	12:00	22.5	83.8	998	18.01	17.6460	84.2574	541.8	3.5	7.1	207
13	18:00	23.0	83.5	1002	15.43	17.4855	84.1614	617.2	2.9	7.7	172

characteristics during Cyclone Phailin. Technical details of the Directional Waverider buoy and the accuracy of the measurement are presented by Barstow and Kollstad (1991). Wave data are recorded continuously at 1.28 Hz, and heave is measured in the range of -20 to 20 m with a resolution of 1 cm and an accuracy of 3 %. Wave spectrum is obtained through fast Fourier transform (FFT) with high-frequency cut-off at 0.58 Hz and resolution of 0.005 Hz. Significant wave height and mean wave period (T_{m02}) are estimated from the spectral moment. Spectral peak period (T_p) is estimated at the spectral peak. Zero-crossing analysis of the surface elevation time series is used to estimate maximum wave height (H_{\max}) and $H_{1/3}$. Other parameters studied are spectral

peakedness parameter $\left(Q_p = \frac{2}{m_0} \int_0^\infty f S^2(f) df\right)$ and spectral width parameter $\left(v = \sqrt{(m_0 m_2)/m_1^2 - 1}\right)$ based on spectral analysis (Goda, 1970) and maximum spectral energy density, where m_n is the n th order spectral moment and $S(f)$ is the spectral energy density at frequency f . Mean wave direction corresponding to the spectral peak (θ_p) and the directional width (σ) is estimated based on circular moments (Kuik et al., 1988). The Joint North Sea Wave Project (JONSWAP) theoretical spectrum (Hasselmann et al., 1973) is used for comparison of the measured data and is given below.

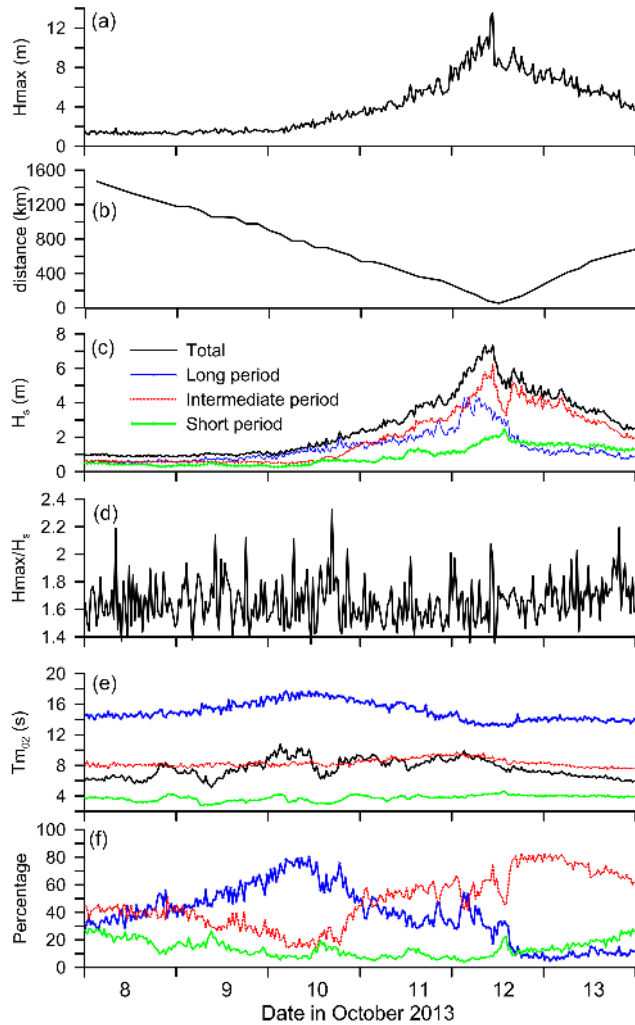


Figure 3. Variation of (a) maximum wave height, (b) distance of the cyclone from the buoy location, (c) significant wave height of long, intermediate and short period waves, (d) ratio of maximum wave height to significant wave height, (e) mean wave period of long, intermediate and short period waves and (f) percentage of long, intermediate and short period waves during 8–13 October 2013.

In the past decade, significant advances have been made in numerical modelling of hurricane-generated waves in deep water due to the availability of measured waves, mainly in the Gulf of Mexico. Fan et al. (2009) made numerical simulations of waves under Hurricane Ivan in 2004 and compared them with buoy observations and found that the original WAVEWATCH III model drag parameterisation tends to overestimate the significant wave height. Kumar et al. (2013) found that during Cyclone Thane, the maximum value of H_s estimated using a parametric wave model (USACE, 1984) for deep water conditions is 6.4 m whereas the measured value is 6 m. In the present case, the value of H_s estimated using a parametric wave model (USACE, 1984) when the cyclone was close to the measurement location (< 100 km) is 6.6 to

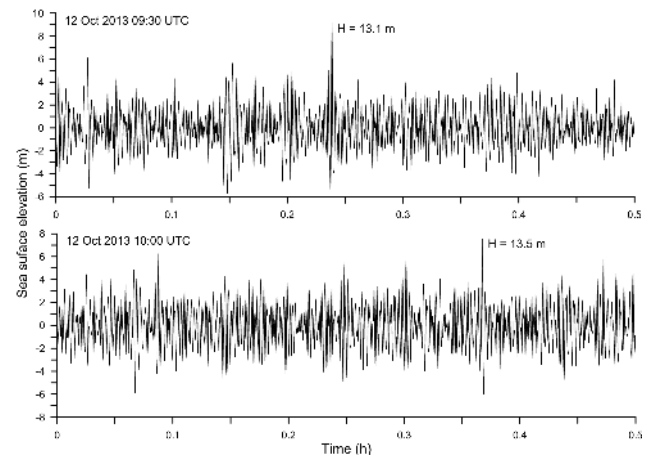


Figure 4. The time series plot of the sea surface elevation recorded at 09:30 and 10:00 UTC on 12 October 2013. The maximum wave height recorded was 13.1 m and 13.5 m and during this period, the buoy was at 50 m water depth.

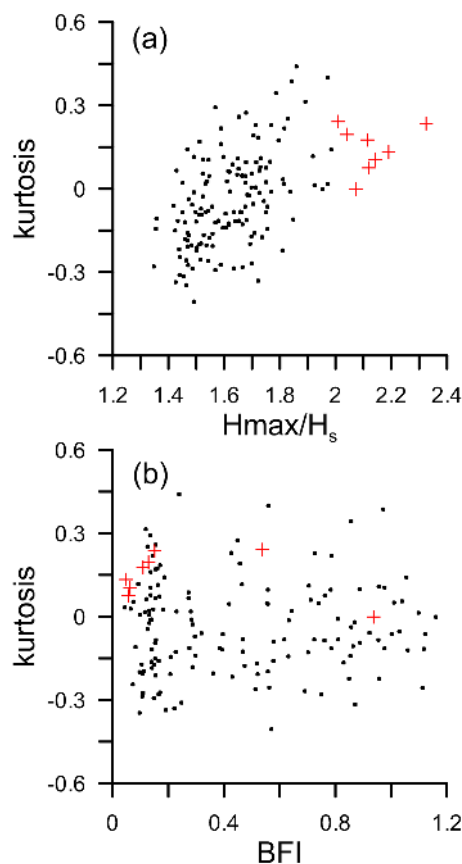
6.8 m, whereas the measured value is 7.0 to 7.3 m, indicating that the parametric wave model (USACE, 1984) estimates the wave height within 10 % of the measured value during the cyclone period.

The time series plots containing the highest waves (13.5 and 13.1 m) recorded are presented in Fig. 4. The height of the waves before and after the maximum wave height was 45 to 58 % of the maximum value. H_s for the 30 min recordings containing the highest waves are 7.3 and 7.0 m and the ratio of H_{\max} to H_s is ~ 1.85 and is higher than the theoretical value of 1.62 to 1.64 for 190 to 220 waves (Longuet-Higgins, 1952). During the cyclone, the ratio between H_{\max} and H_s for the 30 min recordings varied from 1.4 to 2.3, with an average value of 1.66 (Fig. 3d), and the average value measured is close to the theoretical value.

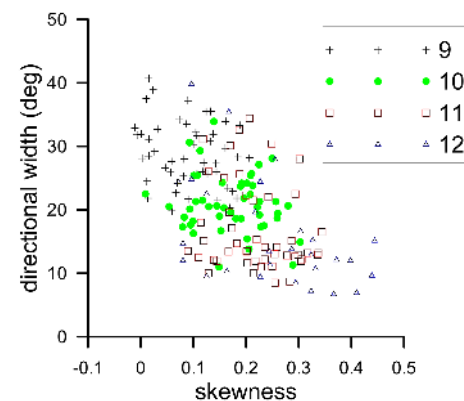
From 8 to 12 October 2013, freak waves, waves with abnormality index ($H_{\max}/H_s > 2$), were observed eight times (Fig. 5). The number of freak waves occurring during a storm is an important parameter for marine engineers, marine workers and sailors. Freak waves sometimes feature a single steep crest, causing severe damage to offshore structures and ships (Mori et al., 2011). The abnormality index is found to increase with an increase in the kurtosis of the sea surface elevation (Fig. 5a). Mori et al. (2011) showed that the kurtosis depends on the square of the Benjamin–Feir index (BFI), but the present data did not indicate such a relationship due to the presence of multi-peaked spectra in the study region (Fig. 5b). The skewness, the degree of vertical asymmetry of the surface elevation, is found to vary with the directional width (Fig. 6). On 9 October 2013, the skewness values are less than 0.2 and on 12 October 2013, the skewness increased to 0.45. The average directional width is 29 and 17° on 9 October and 12 October 2013. As the directional width decreases the skewness increases, indicating

Table 2. Daily mean wave parameters during 8–13 October 2013.

Parameters	Date in October 2013					
	8	9	10	11	12	13
Significant wave height (m)	0.9	1.0	1.6	3.3	5.6	3.6
Maximum wave height (m)	1.4	1.5	2.4	4.8	8.6	5.7
Ratio of maximum and significant wave height	1.64	1.67	1.66	1.59	1.66	1.72
Mean wave period (s)	6.6	7.1	8.6	8.6	8.2	6.6
Peak wave period (s)	14.4	16.3	18.4	13.5	11.2	8.4
Wave period of maximum wave height (s)	11.1	13.5	15.9	12.2	10.4	8.2
Maximum spectral energy density ($\text{m}^2 \text{Hz}^{-1}$)	1.2	1.7	8.4	17.9	60.7	14.5
Spectral width parameter	0.63	0.73	0.74	0.53	0.43	0.42
Spectral peakedness parameter	1.4	1.6	2.7	1.9	2.6	2.1
Long period waves (%)	38	52	67	38	25	10
Intermediate period waves (%)	41	33	23	54	65	72
Short period waves (%)	21	15	10	08	10	18

**Figure 5.** Plot of (a) kurtosis of the sea surface with abnormality index (H_{\max}/H_s) and (b) kurtosis of the sea surface elevation with Benjamin–Feir index (BFI). The data of the freak waves are indicated with cross symbols.

that at high sea states, the sea surface elevation is positively skewed and the directional spreading of spectral energy density at a minimum.

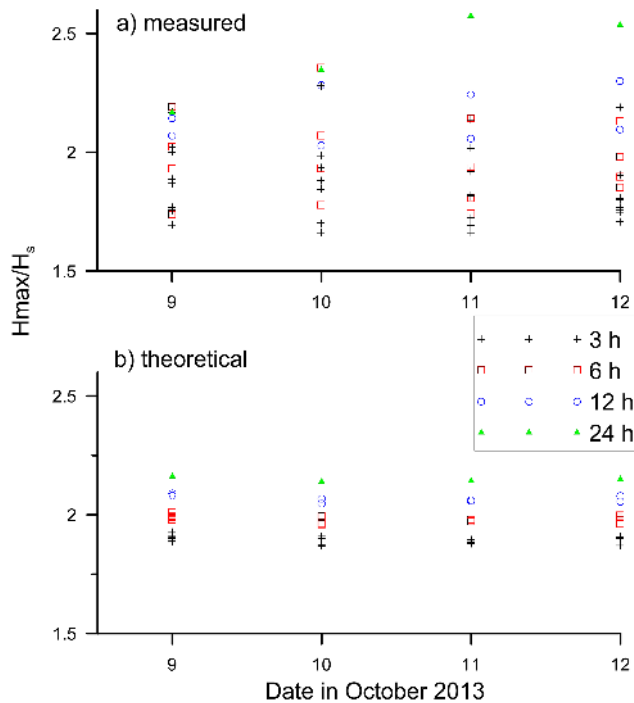
**Figure 6.** Variation of skewness of the sea surface elevation with directional width during 9–12 October 2013.

The ratio of the H_{\max} to H_s for time series data with 3, 6, 12 and 24 h duration are 1.87, 1.97, 2.16 and 2.41 and the respective theoretical values based on the number of waves (Longuet-Higgins, 1952) in the recording period are 1.89, 1.98, 2.06 and 2.15 (Fig. 7). Hence, it is important to consider the duration of the storm while estimating H_{\max} from the H_s data. For the design of offshore structures, Goda (1985) proposed using $H_{\max} = 2 H_s$, which is slightly lower than the value (2.16) observed in the present study for the 6 h duration data.

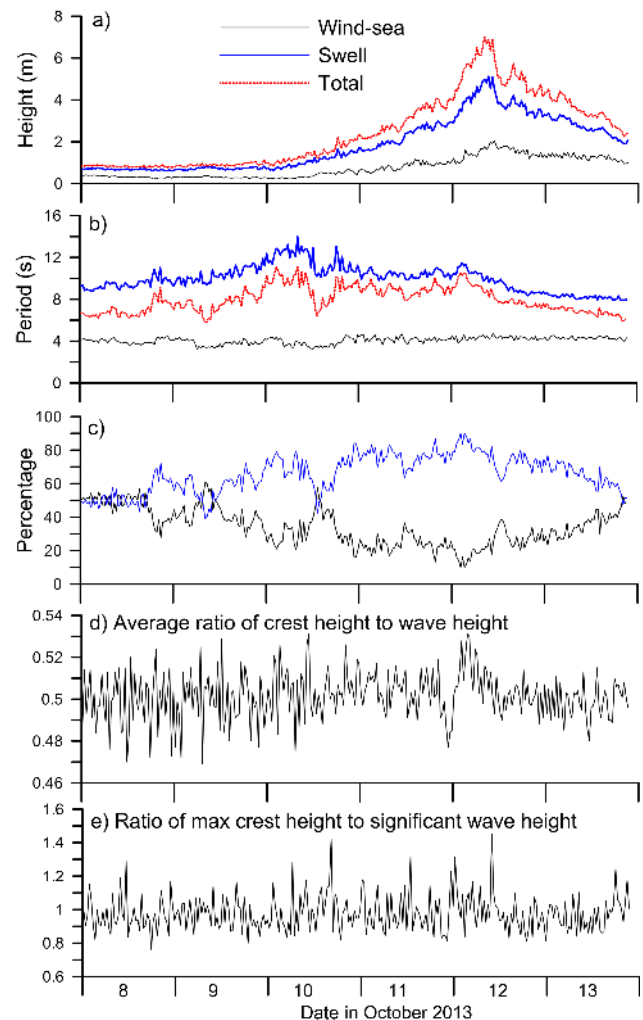
The data are recorded continuously and hence the influence of data recordings at 3, 6 and 12 h on the daily maximum H_s and the daily average H_s are studied. The study shows that data recorded with intervals of more than 3 h lead to missing the high waves, and the maximum H_s at 6 and 12 h data intervals is 8 and 23 % less than that based on 0.5 h intervals (Table 3). Mean H_s for data with 12 h intervals is 10 % less than the H_s data with 3 h intervals.

Table 3. Mean and maximum value of significant wave height for different data recording intervals during 8–12 October 2013.

Date	Data recording interval							
	0.5 h	3 h	6 h	12 h	0.5 h	3 h	6 h	12 h
	Mean significant wave height (m)				Maximum significant wave height (m)			
8	0.9	0.9	0.9	0.9	1.0	1.0	1.0	1.0
9	1.0	1.0	1.0	0.9	1.2	1.2	1.1	1.0
10	1.6	1.6	1.5	1.3	2.3	2.3	2.3	1.7
11	3.3	3.2	3.1	2.9	4.4	4.2	4.1	3.4
12	5.6	5.6	5.5	5.0	7.3	7.0	6.7	5.6

**Figure 7.** Ratio of maximum wave height to significant wave height (a) based on measured data and (b) estimated using the Rayleigh distribution for data collected for 3, 6, 12 and 24 h durations.

Individual wave heights and associated wave periods were estimated from the 30 min recordings during 8–12 October 2013 and the waves were separated into swell and wind sea considering 6 s wave period as the separation period (Fig. 8a and b). The analysis shows that the percentage of swell and wind sea is the same ($\sim 50\%$) before the cyclone, and due to the influence of cyclone, the swells increased and reached a maximum (~ 90) just before the maximum H_s (Fig. 8c). Even though high local wind is present, when the cyclone is close to the buoy location, the measured waves are predominantly swells (Fig. 8c). The reason for swell domination is that swell generated in the cyclone wind regions propagates ahead of the cyclone and dominates the whole wave

**Figure 8.** Variation of (a) average wave height of wind sea, swell and total, (b) average period of wind sea, swell and total, (c) percentage of wind sea and swell, (d) average ratio of crest height to wave height and (e) ratio of the maximum crest height to significant wave height in the 30 min recordings.

field. The cyclone took 60 h (10 October 2013 00:00 UTC to 12 October 12:00 UTC) to reach the wave measurement location whereas the swells generated by the cyclone took only 27 to 40 h (group velocities for waves with peak wave period 8 s and 12 s are 6.2 and 9.4 m s⁻¹) to travel 900 km (distance between the cyclone position at 10 October 2013 00:00 UTC and 12 October 12:00 UTC). Previous measurements of tropical cyclones (Young, 2006; Kumar et al., 2013) also showed similar results. King and Shemdin (1978) found that cyclonic storms cause generation of low-frequency waves and these waves can propagate ahead of the cyclone.

The average ratio of crest height to wave height of all waves in the 30 min recordings varied from 0.47 to 0.53 (Fig. 8d). The ratio of crest height to wave height of the high wave recorded during the cyclone was 0.6 to 0.7 (Fig. 9a

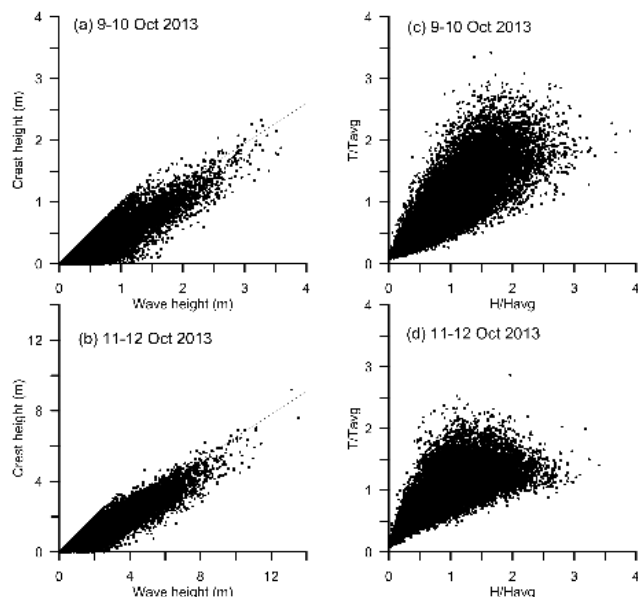


Figure 9. Relationship between crest height and wave height (a) before the cyclone and (b) during the cyclone. Joint distribution of normalised period and height (c) before the cyclone and (d) during the cyclone.

and b) and is similar to the value (0.65) observed during Cyclone Thane (Kumar et al., 2013). During Cyclone Thane, the maximum H_s measured was 6 m and occurred on 29 December 2011. In the present study, the ratio of the maximum crest height to the significant wave height in the 30 min recordings varied from 0.8 to 1.5 with an average value of 1 (Fig. 8e).

The joint distribution of wave height and period measured on 12 October 2013 09:30 and 10:00 UTC shows that high waves ($H > 8$ m) are associated with 11–13 s wave periods and waves with heights of 6 to 8 m are associated with periods of 9 to 14 s (Fig. 10). The scatter plot of normalised wave height and normalised wave period during 9–10 October and 11–12 October is presented in Fig. 9c and d. The shape of the distribution is different before the cyclone and during the cyclone. Before the cyclone, the increase in wave period was observed with an increase in wave height. The period distribution narrows as the wave height increases during the cyclone period, and the long period waves ($T > 18$ s) measured are with height less than 4 m. The steepness of the waves measured is less than the limiting steepness value during the cyclone (Fig. 11a). The curve for the greatest wave height as a function of wavelength and water depth derived by Fenton (1990) also shows that the measured waves are not fully breaking waves (Fig. 11b and c).

Following Bromirski et al. (2005), the wave spectral energy is divided into three bands, that is (1) short period (wave period, $T < 6$ s, is dominated by local seas), (2) long period ($T > 12$ s, results primarily from swell) and (3) intermediate period ($6 \text{ s} \leq T \leq 12 \text{ s}$, probably results from a mixture

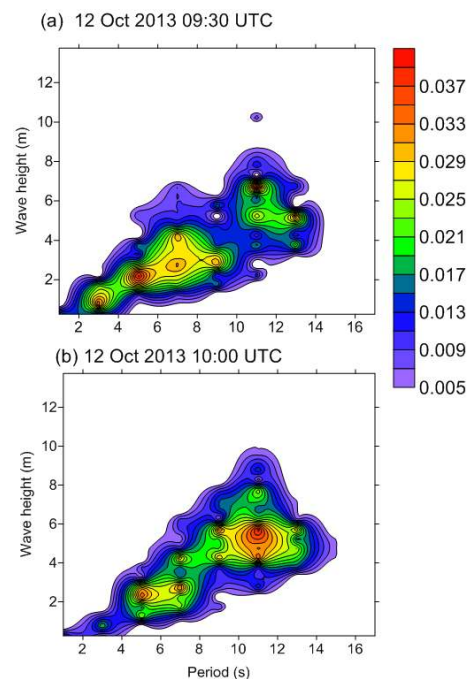


Figure 10. Joint distribution of wave height and period of the high waves. The contours indicate the probability values.

of local and regional wind forcing), and the variations were studied. Before the influence of the cyclone, the waves were a combination of 21, 41 and 38 % short period, intermediate and long period waves and during the cyclone these values changed to 10, 62 and 28 % (Fig. 3f). The contribution of long period waves to the total H_s increased (67 %) on 10 October 2013, when the cyclone was in the Andaman Sea (Fig. 1), but when the cyclone was close to the buoy location, the contribution of intermediate period waves was at a maximum (Fig. 3c). Due to the dominance of the intermediate period waves, the average period of the waves decreased during the cyclone (Fig. 3e).

4.2 Wave spectra

Since there is a large variation in spectral energy density (0.5 to $122.4 \text{ m}^2 \text{ Hz}^{-1}$) during 8–13 October (Fig. 12a), the normalised spectral energy density is plotted on a time–frequency grid to understand the contribution of waves with different frequencies during the cyclone period (Fig. 13a). On 10 October 2013, since the swells reached the measurement site before the cyclone, the wave spectra changed from broad to narrowband and the spectral energy density was within narrow range of frequencies (0.05 to 0.07 Hz). During this period, the spectral peakedness parameter (Q_p) increased (> 3) (Fig. 12b), indicating a very narrow spectrum and the peak wave period increased (> 16 s) (Fig. 12c). Q_p increased as the bandwidth became narrow (fully developed wind seas have $Q_p \approx 2$, whereas broad-banded swells have

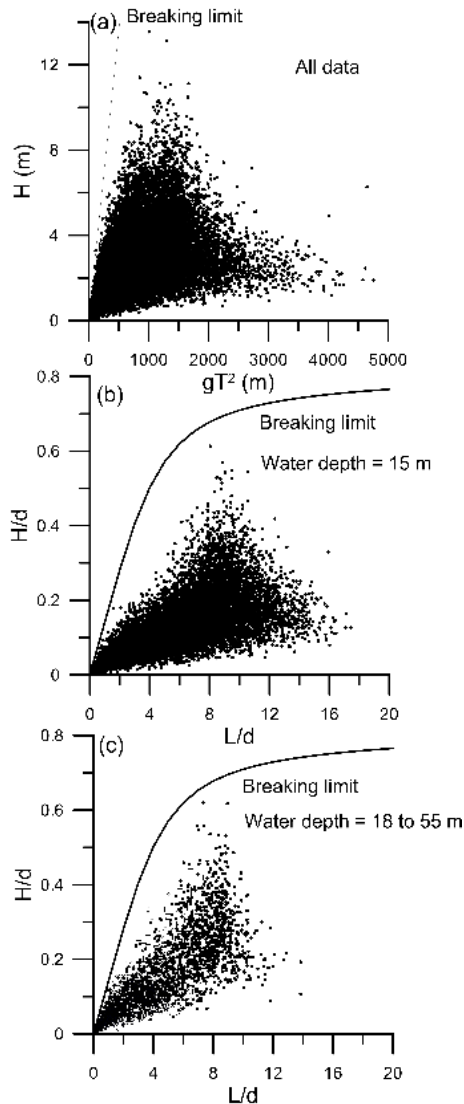


Figure 11. The graph showing the breaking limit based on (a) Ochi (1990) for all data, (b) Fenton (1990) for data at 15 m water depth and (c) Fenton (1990) for data at 18 to 55 m water depth.

values > 2). Goda (1976) reported Q_p values of 2 to 3 from single-peaked spectra recorded in the Japanese coastal waters. On 11 October 2013, due to the influence of cyclone winds, the peak frequency shifted from 0.06 to 0.09 Hz and a broad-banded wave spectrum was observed, due to the combined wind sea and swell. On 12 October 2013, during the cyclone period the mean value of the spectral peakedness parameter was around 2.6, indicating that the spectrum was narrow-banded similar to the observation on 10 October 2013. On 12 October 2013, the spectral width parameter decreased (< 0.5) also indicating narrowband wave spectra (Fig. 12d). The spectral width parameter varies from 0 to 1, and has smaller values for narrower spectra. When the cyclone was close to the buoy, due to the influence of high local

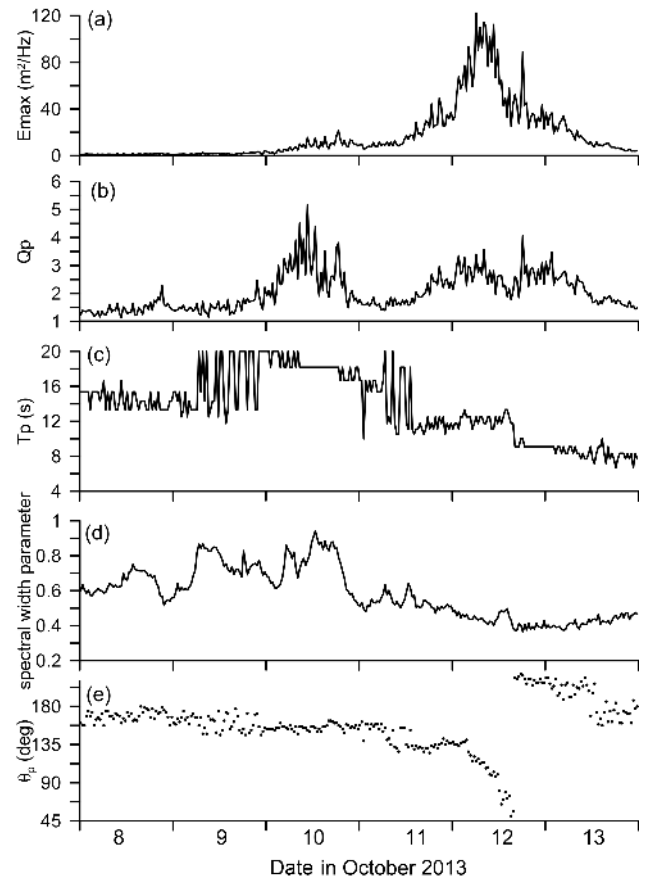


Figure 12. Variation of (a) maximum spectral energy density, (b) spectral peakedness parameter, (c) peak wave period, (d) spectral width parameter and (e) mean wave direction during 8–13 October 2013.

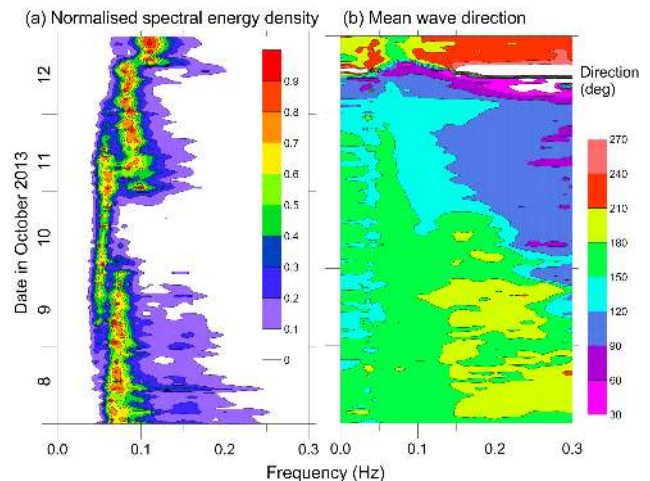


Figure 13. Time history of (a) normalised spectral energy density and (b) mean wave direction with frequency from 8 to 12 October 2013.

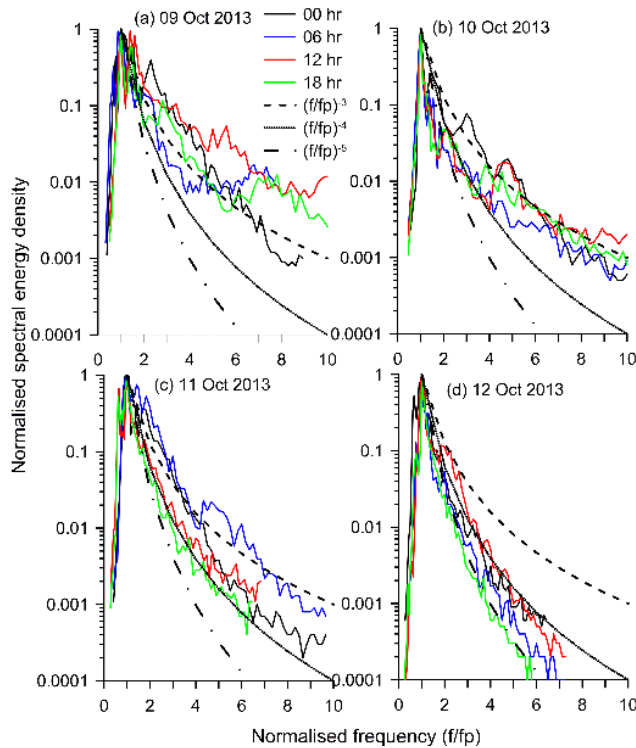


Figure 14. The normalised wave spectra during 9–12 October at different times. The slope of the high-frequency part of the spectra before the cyclone is f^{-3} and during the cyclone it is between f^{-4} and f^{-5} .

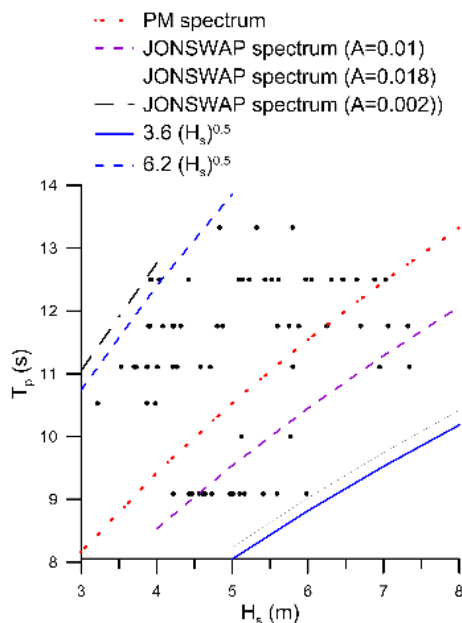


Figure 15. Variation of peak wave period (T_p) with significant wave height (H_s) during 11–12 October 2013. The various theoretical formulations are also presented in the figure.

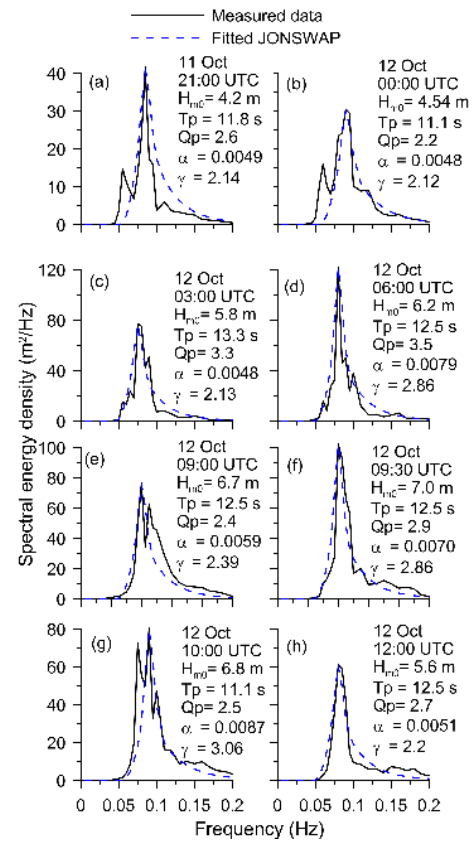


Figure 16. Wave spectra based on measurement and the fitted JONSWAP spectra during the period (11–12 October 2013) when the waves are high due to the influence of Cyclone Phailin.

winds and swells, broadband spectrum should have been observed since the wind sea spectrum was broader than the narrow swell spectrum, but the measured spectra indicate narrowband spectra, due to the dominance of intermediate period waves. Before the cyclone, the mean wave directions of the waves were $150\text{--}180^\circ$ (Fig. 12e). During the cyclone, the mean wave direction initially shifted to $120\text{--}150^\circ$ and then to $90\text{--}120^\circ$. The direction of high-frequency waves also shifted from $180\text{--}210^\circ$ to $90\text{--}120^\circ$ (Fig. 13b).

The measured spectral shape in the high-frequency region lies between the curves proportional to f^{-5} and f^{-3} (where f is the frequency), is between f^{-4} and f^{-5} when the cyclone is close to the measurement location and is close to f^{-3} before the cyclone (Fig. 14). Young and Verhagen (1996) found that the exponent of the spectral curve in the high-frequency region is approximately -5 in deep water and around -3 in finite depth.

It is important to know the value of T_p associated with the H_s for certain designs and hence the variation of T_p with H_s is studied along with the theoretical formulations. The $T_p/(H_s)^{0.5}$ varied between 3.6 and 6.2 during the cyclone period (Fig. 15) and is within the values 3.6 and 5 recommended (DNV, 2011) to use the JONSWAP spectrum

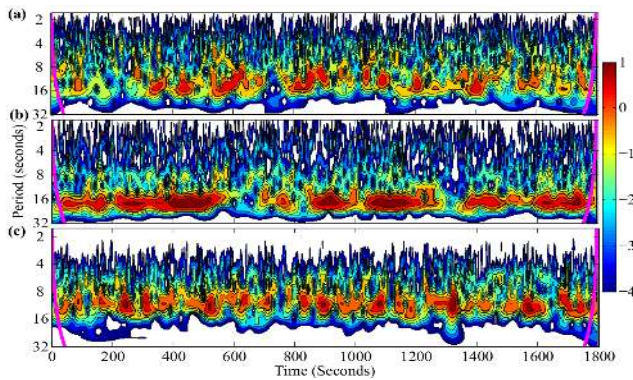


Figure 17. Wavelet power spectrum (a) before cyclone (8 October 00:00 UTC), (b) middle lifespan of cyclone (10 October 12:00 UTC) and (c) cyclone close to the buoy location (12 October 10:00 UTC).

(Hasselmann et al., 1973). Hence, the single-peaked wave spectra of high waves can be represented by the JONSWAP spectrum with modified parameter α (Phillips constant) and γ (Fig. 16). When the spectrum is double-peaked in nature, the JONSWAP spectrum will not provide good fit. The fitted JONSWAP spectrum is obtained by selecting the parameter α and γ of the JONSWAP spectrum such that the peak of the spectrum agrees. During 8–12 October, the average values of the JONSWAP parameters, α and γ , are 0.003 and 1.7 and are close to the observations (0.0027 and 1.63) of Kumar and Kumar (2008) and Kumar et al. (2013). The estimated average value of the JONSWAP parameters, α and γ , are less than the generally recommended values of 0.0081 and 3.3 respectively. Ochi (1990) found that the JONSWAP spectrum provided good approximation to the data for the uni-model spectra with the JONSWAP parameters of $\alpha = 0.023$ and $\gamma = 2.2$.

The wavelet power spectrum of 0.5 h recorded data during the cyclone period is examined to know the variation of the period of individual waves (Fig. 17): once before the cyclone, once during the middle lifespan of the cyclone and once when the cyclone is close to the buoy location. In normal conditions, the wave period varied between 8 and 16 s (Fig. 17a), but when the cyclone is 900 km away from the buoy location, the wave period shifted to 16 to 24 s (Fig. 17b). These long period waves has no significant influence on the wave height and spectral wave energy since they were low-amplitude waves. Since the dissipation of the swell decreases with decreasing wave height (Ardhuin et al., 2009), the long period waves generated by cyclones can propagate further with little dissipation. When the cyclone hit the buoy location, the predominant wave period again shifted back to the range of 8 to 16 s due to the increase of wind sea due to the cyclone's winds (Fig. 17c).

5 Concluding remarks

The maximum wave height (8.5 m) measured when the buoy was at 15 m water depth is 0.57 times the water depth and is less than the limiting wave height (0.6 times the water depth). The steepness of the measured waves during the cyclone is less than the limiting steepness value. During the cyclone period, the maximum significant wave height of 7.3 m and maximum wave height of 13.5 m were measured at 50 m water depth when the cyclone was 70 km away from the buoy location. Eight freak wave events were observed during the study period. The average ratio of the maximum wave height to significant wave height during the cyclone is 1.66 and the ratio of the crest height to wave height of the highest wave recorded is 0.6 to 0.7. When the cyclone was close to the buoy location, narrowband spectra were observed, due to the dominance of intermediate period waves, and the high-frequency face of the wave spectrum is proportional to f^{-3} before the cyclone and is between f^{-4} and f^{-5} during the cyclone.

Acknowledgements. The authors acknowledge the financial support given by the Earth System Science Organization (ESSO), Indian National Centre for Ocean Information Services (INCOIS), Ministry of Earth Sciences, government of India to conduct this research. The authors also acknowledge the director of the CSIR-National Institute of Oceanography, Goa, and of INCOIS for the encouragement provided to carry out the study. We thank Chilka Development Authority, Bhubaneswar, for help during the measurement. The authors would also like to thank Hubert Branger and the anonymous reviewer who greatly contributed to the improvement of this manuscript. This is NIO contribution 5628 and INCOIS contribution 201.

Topical Editor V. Kotroni thanks H. B. Branger and one anonymous referee for their help in evaluating this paper.

References

- Ardhuin, F., Chapron, B., and Collard, F.: Observation of swell dissipation across oceans, *Geophys. Res. Lett.*, 36, L06607, doi:10.1029/2008GL037030, 2009.
- Babanin, A. V., Hsu, T.-W., Roland, A., Ou, S.-H., Doong, D.-J., and Kao, C. C.: Spectral wave modelling of Typhoon Krosa, *Nat. Hazards Earth Syst. Sci.*, 11, 501–511, doi:10.5194/nhess-11-501-2011, 2011.
- Barstow, S. B. and Kollstad, T.: Field trials of the directional waverider, in: *Proceedings of the First International Offshore and Polar Engineering Conference*, Edinburgh, III, 55–63, 1991.
- Bromirski, P. D., Cayan, D. R., and Flick, R. E.: Wave spectral energy variability in the northeast Pacific, *J. Geophys. Res.*, 110, C03005, doi:10.1029/2004JC002398, 2005.
- Chandramohan, P., Kumar, V. S., and Nayak, B. U.: Coastal processes along the shorefront of Chilka lake, *Indian J. Mar. Sci.*, 22, 268–272, 1993.
- Chu, P. C. and Cheng, K. F.: South China Sea Wave Characteristics during Typhoon Muifa Passage in Winter 2004, *J. Oceanogr.*, 64, 1–21, 2008.

- DNV: Modelling and analysis of marine operations, Recommended Practice DNV-RP-H103, DET NORSKE VERITAS, 150 pp., 2011.
- Fan, Y., Ginis, I., Hara, T., Wright, C. Y., and Walsh, E. J.: Numerical Simulations and Observations of Surface Wave Fields under an Extreme Tropical Cyclone, *J. Phys. Oceanogr.*, 39, 2097–2116, 2009.
- Fenton, J. D.: Non-linear wave theories, in: *The Sea, Ocean Engineering Science*, Vol. 9, Part A, edited by: Le Mehaute, B. and Hanes, D. M., Pub. Wiley Interscience, 1990.
- Goda, Y.: Numerical experiments on wave statistics with spectral simulation, Report Port and Harbour Research Institute, Japan, 9, 3–57, 1970.
- Goda, Y.: On Wave Groups, *Proc. Conf. Behaviour of Offshore Structures*, Trondheim, Norway, Vol. 1, 115–128, 1976.
- Goda, Y.: Random Seas and the design of maritime structures, University of Tokyo Press, Tokyo, 1985.
- Hasselmann, K., Barnett, T. P., Bouws, E., Carlson, H., Cartwright, D. E., Enke, K., Ewing, J. A., Gienapp, H., Hasselmann, D. E., Kruseman, P., Meerburg, A., Muller, P., Olbers, D. J., Richter, K., Sell, W., and Walden, H.: Measurements of wind-wave growth and swell decay during the Joint North Sea Wave Project (JONSWAP), *Deutsche. Hydrograph., Z.*, A12, 95, 1973.
- IMD: Very Severe Cyclonic Storm, PHAILIN over the Bay of Bengal (8–14 October 2013): A Report, India meteorological department, Earth System Science Organisation, Ministry of Earth Sciences Government of India Cyclone Warning Division, New Delhi, available at: <http://www.imd.gov.in/section/nhac/dynamic/phailin.pdf> (last access: 20 December 2013), October 2013.
- King, D. B. and Shemdin, O. H.: Radar observations of hurricane wave directions, *Proc. 16th Int. Conf. Coastal Eng. Hamburg, Germany*, ASCE, 209–226, 1978.
- Kuik, A. J., Vledder, G., and Holthuijsen, L. H.: A method for the routine analysis of pitch and roll buoy wave data, *J. Phys. Oceanogr.*, 18, 1020–1034, 1988.
- Kumar, V. S. and Kumar, K. A.: Spectral representation of high shallow water waves, *Ocean. Eng.*, 35, 900–911, 2008.
- Kumar, V. S., Glejin, J., Dubhashi, K. K., and Nair, T. M. B.: Waves during THANE cyclone, Bay of Bengal, *Natural Hazards*, 69, 509–522, doi:10.1007/s11069-013-0713-z, 2013.
- Longuet-Higgins, M. S.: On Statistical Distribution of the Heights of Sea Waves, *J. Mar. Res.*, 11, 245–266, 1952.
- Massel, S. R.: On the largest wave height in water of constant depth, *Ocean. Eng.*, 23, 553–573, 1966.
- Mohanty, P. K., Patra, S. K., Bramha, S., Seth, B., Pradhan, U., Behera, B., Mishra, P., and Panda, U. S.: Impact of Groins on beach morphology: A case study near Gopalpur Port, East Coast of India, *J. Coast. Res.*, 28, 132–142, 2012.
- Mori, N., Onorato, M., and Janssen, P. A. E. M.: On the Estimation of the Kurtosis in Directional Sea States for Freak Wave Forecasting, *J. Phys. Oceanogr.*, 41, 1484–1497, 2011.
- Ochi, M. K.: Stochastic description of offshore environment, *Water Waves Kinematics*, Kluwer, Academic, 31–56, 1990.
- PrasadKumar, B. and Stone, G. W.: Numerical Simulation of Typhoon Wind Forcing in the Korean Seas Using a Spectral Wave Model, *J. Coastal Res.*, 23, 362–373, 2007.
- Rajesh, G., JossiaJoseph, K., Harikrishnan, M., and Premkumar, K.: Observations on extreme meteorological and oceanographic parameters in Indian seas, *Curr. Sci.*, 88, 1279–1282, 2005.
- Soomere, T., Behrens, A., Tuomi, L., and Nielsen, J. W.: Wave conditions in the Baltic Proper and in the Gulf of Finland during windstorm Gudrun, *Nat. Hazards Earth Syst. Sci.*, 8, 37–46, doi:10.5194/nhess-8-37-2008, 2008.
- USACE: Shore Protection Manual, Department of the Army, U.S. Corps of Engineers, Washington DC, 3.81–3.84, 1984.
- Xu, F., Perrie, W., Toulany, B., and Smith, P. C.: Wind-generated waves in Hurricane Juan, *Ocean. Model.*, 16, 188–205, 2007.
- Young, I. R.: Directional spectra of hurricane wind-waves, *J. Geophys. Res.*, 111, C08020, doi:10.1029/2006JC003540, 2006.
- Young, I. R. and Verhagen, L. A.: The growth of fetch limited waves in water of finite depth. Part I: Total energy and peak frequency, *Coast. Eng.*, 29, 47–78, 1996.